TRAFFIC IMPACT ANALYSIS

411 Michigan Avenue

411-419 Michigan Ave Miami Beach, FL 33139

Prepared For:
Bizzi & Partners Acquisitions, LLC
55 East 59th Street, 24th floor
New York, NY 10022

Prepared By:
Langan Engineering & Environmental Services, Inc.
15150 NW 79 Court
Miami Lakes, FL 33016
FL Certificate of Authorization No: 6601

John P. Kim, P.E., PTOE P.E. License No. 62400

Eric Schwarz, P.E., LEED AP Principal/Vice President

9 September 2021;11 November 2021

Revised: 3 February 2022

300277901

LANGAN

Table of Contents

EXECUTIVE SUMMARY	i
INTRODUCTION	
Project Description	
DESCRIPTION OF EXISTING CONDITIONS	
Roads	
Traffic Volumes	4
Intersection Capacity Analysis (Level of Service)	
PLANNED AND PROGRAMMED ROADWAY IMPROVEMENTS	5
FUTURE CONDITIONS	
Background Traffic	6
Site-Generated Trips	
Trip Distribution	
Build Traffic Volumes	8
Site Access & Circulation	9
Driveway Volumes	
Valet Operation and Car Elevator Queuing Analysis	11
Transportation Demand Management Strategies	13
CONCLUSIONS	14

List of Figures

Figure 1 - Site Location Map

Figure 2 - Intersection Lane Configurations

Figure 3 - 2021 Existing Peak-Hour Traffic Volumes

Figure 4 - 2023 No-Build Peak-Hour Traffic Volumes

Figure 5a - Project Traffic Distribution - AM

Figure 5b - Project Traffic Distribution - PM

Figure 6 - Project Traffic Assignment

Figure 7 - 2023 Build Peak-Hour Traffic Volumes

Figure 8 - Site Driveway Volumes

List of Tables

Table 1 - 2021 Existing Conditions Intersection Capacity Analysis Summary

Table 2 - 2023 No-Build Conditions Intersection Capacity Analysis Summary

Table 3 - Trip Generation Estimates

Table 4 - Cardinal Distribution

Table 5 - 2023 Build Conditions Intersection Capacity Analysis Summary

Table 6 - Valet Operation Queuing Analysis Summary

Table 7 - Car Elevator Queuing Analysis Summary

Table 8 - Proposed TDM Strategies

Appendices

Appendix A - Figures

Appendix B - Site Plan

Appendix C - Methodology Letter

Appendix D - Traffic, TAZ, Signal Timing Data, Census Data & FDOT Tables

Appendix E - Intersection Volume Spreadsheets

Appendix F - Intersection Capacity Reports

Appendix G - Trip Generation Data

Appendix H - ITE Excerpts & Queuing Analysis Calculations

EXECUTIVE SUMMARY

Bizzi & Partners Acquisitions, LLC retained Langan Engineering & Environmental Services, Inc. to prepare a traffic-impact analysis for the 411 Michigan mixed-use development. The 0.94-acre vacant site is at 411-419 Michigan Avenue in Miami Beach, Florida. The proposed development comprises 36,442 square feet of general office space, which will have approximately 200-employees, and 4,320 square-feet of retail uses expected to be built by 2023. We analyzed two signalized intersections and one stop-sign controlled intersection for the 2023 build conditions. The peak-hour capacity analyses with the proposed development's impacts in 2023 yielded the following results:

- o All study intersections are expected to operate within their adopted Level of Service (LOS) during the morning and afternoon peak-hours with the development's impacts.
- The proposed driveway connection to the alley abutting the development is expected to operate at LOS A during the morning and afternoon peak-hours.
- o The proposed development will not have gate-controlled access at any of the proposed site driveways.
- The proposed development will have a have a valet-operation that will control parking throughout the entire day. All patrons will be required to use the valet-operation.
- o The proposed connection to Michigan Avenue will operate as an ingress only driveway, and the proposed connection to the alley will operate as an egress only driveway during all times of the day. The alley will operate as a two-way road between 5th and 4th streets.
- All patrons will be required to drop-off their vehicles at the vehicle elevator entrance located on the alley. This will allow for efficient valet operation without impacting the public right-ofway.
- The developer plans to lease out three parking spaces abutting the property on Michigan Avenue to assist in vehicle drop-off and avoid queues on public right-of-way.
- o The valet operation will not cause entering traffic to back into the adjacent public roadways with a minimum of six parking attendants to serve expected demand.
- o The proposed car elevators will be sufficient to serve the expected demand.
- o The development site is within the Urban Infill Area (UIA).
- The development will promote the use of different modes of transportation through the implementation of several TDM strategies.

We conducted intersection-capacity analyses for the existing, no build (future without project) and build (future with project) conditions. The proposed development is expected to generate 1,250 daily, 66 morning peak-hour and 90 afternoon net-new peak-hour trips.

INTRODUCTION

Langan was retained by Bizzi & Partners Acquisitions, LLC to prepare this impact-analysis report for the 411 Michigan mixed-use development that will be built by 2023. The site will comprise approximately 0.94 acres at 411-419 Michigan Avenue in Miami Beach, Florida. The development will comprise 36,442 square feet of office space, with approximately 200-employess, and 4,320 square feet of retail uses.

We analyzed two signalized intersections and one stop-sign controlled intersection during the morning and afternoon peak hours. We found that all the study intersections are expected to operate within their adopted LOS during the morning and afternoon peak-hours with and without the proposed project's impacts. The valet operation is expected to generate a queue of four vehicles and need a minimum of six parking attendants to serve the expected demand. The development is proposing to change the operation of the abutting alley from one-way to two-way which will allow the valet-operation to operate efficiently without impacting the public right-of-way. This report presents the traffic-data and traffic-impact analysis for this proposed development.

Project Description

The proposed development will be built on two parcels (Folio Nos.: 02-4203-010-0030 & 02-4203-009-6170). **Appendix A** contains the figures of this report. **Figure 1** illustrates the site location. **Appendix B** contains a copy of the site plans showing the proposed development program and the two proposed driveway connections; one to a public road (Michigan Avenue) and one to an alley. Traffic from the proposed development will enter the site via Michigan Avenue and will exit to the alley (abutting the site). The drop-off and pick-up locations will facilitate the valet-parking operation, and will be managed by valet staff throughout the day using signs and verbal communication. The Michigan Avenue driveway will operate as an ingress only driveway, while the alley driveway will operate solely as an egress only ingress driveway. Vehicles exiting the site to the alley will be allowed to make a left-turn or a right-turn.

The development will relocate the existing historical house on the site to front Michigan Avenue and will reuse the existing foundations on site. The majority of the 85 parking spaces provided by the proposed development will comprise vehicular lifts which can stack up to three vehicles in one parking space. Nine of the 85 parking spaces will be exclusive carpool spaces. The development will also provide 25 bicycle parking spaces and 5 scooter parking spaces. All visitors and employees will have to use the valet-parking service. The maximum acceptable LOS for

roadways and intersections is LOS D for county and city roads and LOS E for State Urban Minor Arterials (SUMA) between Infill Area and Urban Development Boundary.

Scope of Study

Langan undertook the following steps to prepare this study in accordance with the methodology discussed with the city's staff. **Appendix C** contains a copy of the methodology letter.

- Collected morning (7 to 9 AM) and afternoon (4 to 6 PM) peak-hour vehicle turningmovement volumes at the following study intersections:
 - Alton Road and 4th Street (signalized)
 - o Michigan Avenue and 4th Street (unsignalized)
 - o Michigan Avenue and State Road A1A / 5th Street (signalized)
 - Collected 24-Hour bidirectional counts on SR-A1A/5th Street between Meridian and Euclid avenues
- Used Peak Season Conversion Factors (PSCF) from the Florida Department of Transportation (FDOT) to convert the traffic data into peak-season volumes.
- Developed a COVID-adjustment factor by comparing 2020 traffic data to 2021 traffic data along segments of 5th Street to scale the traffic data to account for variations from the true traffic count due to the ongoing Coronavirus pandemic. The COVID-adjustment factor was used in conjunction with the traditional Peak Season Category Factor (PSCF) to estimate the existing traffic data.
- The COVID-adjustment factor calculated for the morning and afternoon peak-hours were 1.46 and 1.62, respectively.
- Prepared trip-generation estimates for the proposed development, based on accepted tripgeneration rates developed by the Institute of Transportation Engineers (ITE).
- Calculated a growth rate for background traffic using FDOT historical data from traffic-count stations near the project.
- Developed trip-distribution estimates for the project, based on the cardinal distribution for the corresponding Traffic Analysis Zone of the Miami-Dade County 2045 Long Range Transportation Plan (LRTP). A computer program used to develop the 2045 LRTP Directional Distribution Report generates directional distributions for each TAZ for the eight secondaryintercardinal directions (NNE; ENE; ESE; SSE; SSW; WSW; WNW; NNW).
- Prepared morning and afternoon peak-hour intersection-capacity analyses for the following conditions at the study intersections: 2021 existing, 2023 future no-build, and 2023 future build.
- Calculated the morning and afternoon peak-hour LOS intersection-capacity analyses of the development's driveways for the 2023 build conditions.

DESCRIPTION OF EXISTING CONDITIONS

Langan visited the study area to collect the lane-configuration and traffic-control data shown in **Figure 2**. **Appendix D** contains the county's signal-timing data.

Roads

Alton Road

Alton Road is a four-lane, north-south, divided, city-maintained major collector roadway with a 30 MPH posted speed limit.

4th Street

4th Street is a two-lane, east-west, undivided, city-maintained local collector roadway with a 25 MPH posted speed limit.

Michigan Avenue

Michigan Avenue is a two-lane, undivided, north-south, city-maintained local roadway with a 25 MPH posted speed limit.

SR-A1A / 5th Street

SR-A1A / 5th Street is a six-lane divided, east-west, state-maintained principal arterial roadway with a posted speed limit of 35 MPH.

Traffic Volumes

Traffic-volume data was collected on Thursday, August 5, 2021 from 7:00 to 9:00 AM and 4:00 to 6:00 PM. We applied FDOT's season adjustment factor (1.05) and a COVID-factor to convert the traffic data into peak-season volumes because the data was collected during the ongoing Coronavirus pandemic. We developed peak-hour COVID-adjustment factors (1.46 morning and 1.62 afternoon) by comparing the traffic data collected on 5th Street to 2021 traffic counts collected on the same roadway segments. We compared the data of each intersection and determined that the peak hour occurred between 8:00 AM and 9:00 AM and between 4:45 PM and 5:45 PM for the study area. **Figure 3** illustrates the existing weekday morning and afternoon peak-hour traffic volumes. Appendix D contains the traffic data and seasonal-adjustment factors.

Intersection Capacity Analysis (Level of Service)

We conducted 2021 existing-conditions capacity analyses for the study intersections using Synchro software. We found that all study intersections are operating within their adopted LOS.

Table 1 summarizes the results of the existing-conditions analysis. **Appendix E** contains intersection-volume tables; **Appendix F** contains the capacity-analyses worksheets.

Capacity analyses for stop-sign controlled intersections are calculated for certain intersection approaches, not for the entire intersection. The stop-sign controlled approaches of stop-sign controlled intersections often exceed their adopted LOS during peak hours because all vehicles must stop and incur a delay before proceeding through the intersection. Capacity analysis provides an indication of the adequacy of intersection and roadway facilities to serve traffic demand. The evaluation criteria used to analyze the study intersections is based on the 6th Edition Highway Capacity Manual published by the Transportation Research Board.

Table 1 - 2021 Existing Intersection Capacity Analysis Summary

			AM Peak Hour		AM Peak Hour		PIV	l Peak Hour
Location	Traffic Control	Approach	LOS	Delay (sec/veh)	LOS	Delay (sec/veh)		
Alton Road & 4 th Street	Signalized	Overall	С	28.7	С	26.9		
	Stop-sign controlled	EB	А	7.5	А	8.4		
Michigan Avenue & 4th		WB	А	7.5	А	8.7		
Street		NB	Α	7.7	А	8.5		
		SB	А	7.6	А	8.4		
Michigan Avenue & 5 th Street	Signalized	Overall	С	22.8	В	12.7		

PLANNED AND PROGRAMMED ROADWAY IMPROVEMENTS

We reviewed the Transportation Planning Organization's 2021 Transportation Improvement Program (2021 through 2025), the county Long Range Transportation Plan (2045) and the FDOT Five Year Work Program (2021 through 2025) and found two planned roadway improvements in the TIP's program network. The proposed improvement project number DT4434321 will construct a pedestrian and bicycle path along the MacArthur Causeway from SR 5/Biscayne Boulevard to SR 907/Alton Road. The second project (No.: TA4466531) will implement the South Beach trolley service route along 5th Street. Appendix C includes excerpts from Miami-Dade TIP showing the proposed improvement information.

FUTURE CONDITIONS

This section of the report covers background traffic growth, site-generated trips, trip distribution, and future traffic volumes. The project should be completed by the end of 2023. We developed 2023 no-build traffic volumes by applying a compounded growth rate to the 2021 volumes. We added site-generated trips to the 2023 no-build traffic volumes to develop 2023 build traffic volumes.

Background Traffic

We conducted intersection capacity analyses and found that all study intersections are expected to operate within their adopted LOS. We performed a growth rate analysis for the most recent five and nine year periods (the ten year period was not available) using FDOT historical traffic volumes. We analyzed the growth rate based on linear, exponential, and decay-exponential approaches and determined that the growth rate with the strongest correlation was the nine year linear-trend. This trend yielded a negative result, and as such we used a 0.5 percent annual growth-rate factor to develop future background volumes. The growth-rate factor accounts for increased background traffic volumes and was applied to the existing volumes. **Figure 4** illustrates the 2023 no-build traffic volumes. **Table 2** summarizes the results of the 2023 no-build conditions capacity analysis. Appendix F contains the capacity-analyses worksheets.

Table 2 - 2023 No Build Intersection Capacity Analysis Summary

	Traffic		AM	Peak Hour	PM Peak Hour	
Location	Control	Approach	LOS	Delay (sec/veh)	LOS	Delay (sec/veh)
Alton Road & 4 th Street	Signalized	Overall	С	28.9	С	27.1
		EB	А	7.5	А	8.4
Michigan Avanua 8, 4th Ctract	Stop-sign controlled	WB	А	7.5	А	8.7
Michigan Avenue & 4th Street		NB	Α	7.7	Α	8.5
		SB	А	7.6	Α	8.4
Michigan Avenue & 5 th Street	Signalized	Overall	С	22.9	В	12.8

Site-Generated Trips

The proposed development is expected to generate 1,250 daily, 66 morning peak-hour, and 90 afternoon net-new peak-hour trips. We prepared daily, morning peak-hour and afternoon peakhour trip estimates for the proposed development using equations from the 10th Edition of the ITE Trip Generation Manual. We performed the office trip generation calculations based on the expected number of employees to provide a conservative analysis, based on conversations with City Staff. We used the morning rate for the retail uses because the morning peak-hour equation for retail has a 151.78 offset, which results in a minimum trip generation of 152 trips for small area retail buildings. We applied a 34% pass-by rate to retail uses trip generation estimates, based on rates of the ITE Trip Generation Handbook 3rd Edition. We also applied a non-vehicular reduction of 20% based on the parking incentives allowed by the City of Miami Beach. In addition, the Miami Beach 2019 Transportation Plan shows that approximately 26% of its population uses transit, bikes and walks. The development will provide 85 off-street vehicle parking spaces, where nine of the 85 parking spaces will be exclusive carpool spaces. In addition the development will provide 25 bicycle long-term parking spaces, five scooter parking spaces and three showers within the development to promote the use of non-vehicular transportation. Table 3 summarizes the trip-generation estimates for the proposed development. Appendix G contains the trip-generation data and includes Miami Beach mode-share data.

Table 3 - Trip Generation Estimates*

Use	Size		Size		Daily	Wee	kday Morr Hour	ning Peak	Weel	kday Afterr Hour	noon Peak
				In	Out	Total	In	Out	Total		
Proposed Uses											
General Office	200	Employees	853	66	13	79	16	62	78		
Shopping Center**	4,320	SF	710	2	2	4	16	19	35		
	Total			68	15	83	32	81	113		
Non-vehicular reduction (20%)			313	14	3	17	6	16	23		
Net New Trips			1,250	54	12	66	26	65	90		

^{*} Based on Trip Generation Manual 10th Ed.

Trip Distribution

We determined the directional distribution of site-generated trips based on the cardinal distribution data for TAZ 652 from the Miami-Dade County 2045 Transportation Model (see Appendix D) and from the development's access to the surrounding roadway network. We interpolated the 2015 and 2045 average directional-distribution values to develop percentages for 2023. **Table 4** shows the proposed development's trip distributions. **Figures 5a** and **5b** show

^{**} Shopping Center land use includes 34% afternoon pass-by trip reduction.

the proposed development's traffic distributions to the study intersections. **Figures 6a** and **6b** illustrate the morning and afternoon development-traffic assignments at the study intersections.

Table 4 - Cardinal Distribution

Year	NNE	ENE	ESE	SSE	SSW	wsw	WNW	NNW
2015	22.90%	4.10%	3.50%	2.80%	2.50%	16.70%	19.40%	28.10%
2045	18.80%	3.20%	3.20%	1.60%	2.30%	19.50%	29.70%	21.80%
2023	21.81%	3.86%	3.42%	2.48%	2.45%	17.45%	22.15%	26.42%

Build Traffic Volumes

We conducted capacity analyses for the study intersections and determined that they are expected to operate within their adopted LOS with the development's impacts. The 2023 build traffic volumes were derived by adding the total site-generated trips to the 2023 no-build traffic volumes. **Figure 7** illustrates the 2023 build morning and afternoon peak-hour traffic volumes. **Table 5** summarizes the 2023 build LOS for the morning and afternoon peak hours. The analysis takes into account the vehicles that are expected to travel on external roadways as a result of valet returning the cars from the car elevator to the main driveway ingress on Michigan Avenue. To provide a conservative analysis we assumed that 92% of the trips will utilize the car elevator. As such, when retrieving cars to bring back to the valet stand, valet will have to travel on external roadways to access the main driveway. These additional trips were also included in the analysis.

Table 5 - 2023 Build Intersection Capacity Analysis Summary

	T.,, #:,		AM	Peak Hour	PM Peak Hour	
Location	Traffic Control	Approach	LOS	Delay (sec/veh)	LOS	Delay (sec/veh)
Alton Road & 4th Street	Signalized	Overall	С	28.8	С	27.8
	Stop-sign controlled	EB	А	7.7	А	8.6
Michigan Avenue & 4th Street		WB	А	7.5	А	9.4
iviichigan Avenue & 4th Street		NB	А	7.8	А	8.8
		SB	А	7.7	А	8.7
Michigan Avenue & 5th Street	Signalized	Overall	С	23.9	В	12.6
Driveway	Stop-sign controlled	EB	А	8.8	А	9.3

^{*}The site driveway LOS represents the Michigan Avenue driveway during the AM and PM.

The analysis for the site driveway at the alley was done based on the proposed operation of the driveway, which will operate as an egress only driveway during the morning and afternoon peak hours.

Site Access & Circulation

The proposed connection to Michigan Avenue will operate as an ingress only driveway during the morning and afternoon peak hours, and the alley will operate as an egress only driveway during the morning and afternoon peak hours. The proposed connection to Michigan Avenue cannot be widen to operate as a two-way driveway all the time due to the existing building at 411 Michigan Avenue and the existing foundations on site. To provide a conservative analysis, we assumed all traffic will drop-off their vehicles at the valet stand, and valet wishing to travel to the car elevator will take the dropped off cars and exit through the alley and drive north to the car elevator. Pick-up will also occur at the valet stand in the main driveway. Valet operators will travel to the car elevator, retrieve the car, and utilize the alley, 4th Street, and Michigan Avenue to reenter the site through the main ingress. Appendix B contains the vehicle circulation figures showing how vehicles will arrive to and depart from the site.

Driveway Volumes

We analyzed the development's proposed driveway connections to Michigan Avenue and the alley for the morning and afternoon peak-hour build conditions and found that they will operate at LOS A. Based on the proposed access operation, site traffic will enter the site from Michigan Avenue and exit via the alley. The alley operates as a two-way northbound and southbound road. The proposed morning and afternoon peak hour onsite circulation will facilitate the valet-parking operation and ensure the vehicle queues remain within the site and not back onto Michigan Avenue. Even though the developer plans to lease out the three parking spaces on Michigan Avenue, allowing some unexperienced patrons to drop-off their cars there to the valet operators, we assumed that 100% of the traffic will enter through Michigan Avenue and exit through the alley to provide a conservative analysis. Gate-controlled access is not proposed and circulation will be managed by valet staff. Table 5 summarizes the 2023 build LOS of the driveway for the morning and afternoon peak hours traffic volumes. **Figure 8** shows the project's driveway volumes and Appendix F contains the capacity-analyses worksheets.

All visitors and employees will have to valet park their vehicles. The alley is a 20-foot wide, local road that operates in the northbound and southbound direction. The proposed development will have 27 off-street parking spaces on the ground floor and 58 off-street parking spaces on the basement floor with access to the alley through a car elevator. In addition, the development will have three on-street parking spaces on Michigan Avenue. The car elevator will be approximately 50 feet north of the proposed alley driveway. The parking spaces on the ground floor will be for visitors and the basement parking spaces will be for employees. The valet operation station will

be located on the ground floor and by the car elevator in the morning peak-hour, where employees and visitors will be able to drop-off their vehicles and the operators will then park the vehicles on the ground floor or basement floor using the car elevator. The valet operators will manage traffic flow to provide for efficient operations. The pick-up operation will be handled on the ground floor where the valet operator will retrieve vehicles from the basement and access the site through the alley. Valet patrons will then exit the site through the alley and can travel north or south once in the alley. Appendix B includes the circulation diagrams for the valet operation during the morning and afternoon peak hour.

Valet Operation and Car Elevator Queuing Analysis

We prepared a queuing analysis for the proposed development's valet operation and found that it will not cause entering traffic to back onto the adjacent public roadway (Michigan Avenue). The proposed development will have a valet-parking station on the ground floor with on-site vehicle-stacking area for five vehicles. All visitors and employees will be required to use the valet operation to park their vehicles. The site plan in Appendix B shows the location of the valet booth and the stacking/queuing area. We used the queuing-analysis methodology from the Transportation and Land Development published by the ITE. This methodology requires hourly rates of vehicle arrival and service times for the valet operation to determine vehicle-queue lengths. The queues resulting from this analysis are 95th percentile queues, which are those expected to be generated 95 percent of the time.

The development will provide nine parking spaces with triple-stack car lifts at the valet court on the ground floor and 29 parking spaces with double-stack car lifts on the basement floor for a total of 85 parking spaces. Vehicle lifts allow two or three vehicles to occupy one parking space by lifting vehicles above the ground and allowing a second or third vehicle to park underneath one another. The parking spaces on the ground floor will be used exclusively for the visitors and the parking spaces on the basement floor will be for employees. The development will also have three on-street parking spaces along Michigan Avenue. Note that there is more than 103 feet of onsite vehicle stacking between the alley and the valet station.

The vehicle-arrival rate was based on the project's peak-hour trip generation, summarized in Table 3. The development is expected to generate 66 (54 ingress and 12 egress) morning peak-hour trips and 105 (33 ingress and 72 egress) afternoon peak hour trips. We estimated the average service time for the valet operation of 3.85 minutes for the drop-off and 4.92 minutes for the pick-up operations. The service time accounts for the time required for the valet attendant to pick-up/drop-off the car, operate the lift, operate the car elevator and return to the valet station. To provide an extremely conservative analysis, we assumed all vehicles would be dropped-off at the valet stand within the proposed development, rather than directly at the car elevator. The analysis indicates that the valet operation will need a minimum of five attendants on the ground floor and one parking attendant on the basement floor. We used 25 feet to convert the number of queued vehicles to linear feet. **Table 6** summarizes the results of the queuing analysis and indicates that queues for the proposed valet operation are not expected to exceed four vehicles. The analyses indicate that the expected 95th percentile queue lengths will not exceed the length of the queuestorage area. **Appendix H** contains excerpts from ITE, the queuing-analysis and service-time calculations.

Table 6 – Valet Operation Queuing Analysis Summary

Time	Storage Capacity	95th Percentile Qu	Exceeds	
Tille	(feet)	Vehicles	Feet	Capacity?
AM	103	1	25	NO
PM	103	4	100	NO

Appendix H contains the queuing-analysis and service-time calculations for the valet operations, as well as the parking stacker specifications. The development will coordinate and setup specific timeframes for the use of the proposed loading area along the alley to avoid any conflict with the valet operation. The specific times frames for the loading area will be outside the expected peakhours of pick-up and drop-off.

In addition, as requested by city staff we performed a queuing analysis for the proposed car elevators and determined that the proposed elevators (2) will be sufficient to serve the expected demand. We used the service time of 1.33 minutes as provided by the car elevator manufacturer. As stated previously, the basement parking spaces will be used mainly by employees. We estimated the number of ingress trips for employees based on rates developed by the Urban Land Institute (ULI) Shared Parking 3 Edition. The proposed access to the car elevators has 25 feet of vehicle stacking. All patrons are expected to drop off their vehicles at the main internal driveway, and valet will take the cars to the car elevator. The development is proposing to lease three parking spaces along Michigan Avenue to serve the demand and the expected queues to avoid vehicles queueing on the alley which will add to a total of four off-street spaces available for the valet-operation. In addition, these on-street parking spaces will allow the internal driveway to operate efficiently and will avoid conflicts with the proposed ground floor stackers. Table 7 summarizes the results of the queuing analysis for the car elevators and indicates that queues are not expected to exceed three vehicles. Appendix B contains a plans showing the valet stand location, the valet parking spaces. Appendix H contains excerpts from ULI, IT and the queuinganalysis.

Table 7 – Car Elevator Queuing Analysis Summary

Time	Storage Capacity	95th Percentile Qu	Exceeds	
Time	(feet)	Vehicles	Feet	Capacity?
AM	75	3	75	NO
PM	75	1	25	NO

Transportation Demand Management Strategies

The site abuts SR-A1A (5th Street) which provides a wide sidewalk, a bicycle lane, special emphasis crosswalks and a transit stop of the southeast corner of Michigan Avenue and 5th Street. The proposed development will provide infrastructure to motivate the use of the available multimodal transportation systems provided by the city and the existing roadway network. This infrastructure will consist of bicycle racks, scooters parking spaces, carpool parking spaces, lockers and showers. In addition, the office spaces will provide Miami-Dade Transit & Miami Beach bus and trolley route information on or near employee bulletin boards to promote the use of public transportation.

The development will work to create Transportation Demand Management (TDM) strategies to support the overall TDM goals of City of Miami Beach and maximize the use of the available transportation systems. The most important action will be doing a regular employees outreach to provide them with the multiple commute options and establish preferences to target TDM efforts. **Table 8** summarizes the proposed TDM strategies.

Table 8 - Proposed TDM Strategies

Action	Details
Employee	Survey employees to determine current commute characteristics establish
Survey	preferences, and target TDM efforts.
Employee Outreach	Provide employees with information regarding multimodal commute options.
Carpool Support	Provide initial coordination and support in setting up carpool parking spaces for employees.
Bicycle Facilities	On-site bike racks will be available for employees who ride their bikes to work.
Travel Mapping	Transit route maps and schedules will be made available on site to employees and visitors.
Flexible Schedule	On site businesses will be encouraged to offer flexible and compressed work schedules to the extent possible.
Valet Operations	The valet operation comprise of six valet attendants and will focus on avoiding vehicles queuing on the public right-of-way. The development will lease three off-street parking spaces to serve the expected demand and avoid queuing during peak-hours.
Loading Area	The loading area will have specific timeframes outside the pick-up/drop-off peak hours to avoid conflicts with the valet-operation.

CONCLUSIONS

Langan performed a traffic-impact analysis for the 411 Michigan development expected to be completed by 2023. The analysis shows the following results for the 2023 build conditions:

- o All study intersections are expected to operate within their adopted LOS during the morning and afternoon peak-hours with and without the development's impacts.
- o The proposed driveway connections to Michigan Avenue and the alley are expected to operate at LOS A during the morning and afternoon peak-hours.
- o The valet operation will not cause entering traffic to back into the adjacent public roadways with a minimum of six parking attendants to serve expected demand.
- o The proposed development will have a have a valet-operation that will control parking throughout the entire day. All patrons will be required to use the valet-operation.
- o The proposed car elevators will be sufficient to serve the expected demand.
- The proposed connection to Michigan Avenue will operate as an ingress only driveway, and the proposed connection to the alley abutting the development will operate as an egress only driveway during all times of the day. The alley will operate as a two-way road between 5th and 4th streets.
- All patrons will be required to drop-off their vehicles at main internal driveway, and the valet operators will drive the vehicles to the car elevator when necessary. This will allow for efficient valet operation without impacting the public right-of-way.
- The developer plans to lease out three parking spaces abutting the property on Michigan Avenue to assist in vehicle drop-off and avoid queues on public right-of-way.
- o The development will not have gate-controlled access at the proposed site driveways.
- o The development site is within the UIA.
- The development will promote the use of different modes of transportation through the implementation of several TDM strategies.

\langan.com\data\MIA\data9\300277901\Project Data_Discipline\Traffic\Reports\2022-02-03 411 Michigan.docx

APPENDIX A FIGURES



15150 NW 79th Court, Suite 200, Miami Lakes, FL 33016 P: 786.264.7221 F: 786.264.7201 www.langan.com FL CERTIFICATE OF AUTHORIZATION No. 00006601

411 MICHIGAN

MIAMI BEACH MIAMI DADE

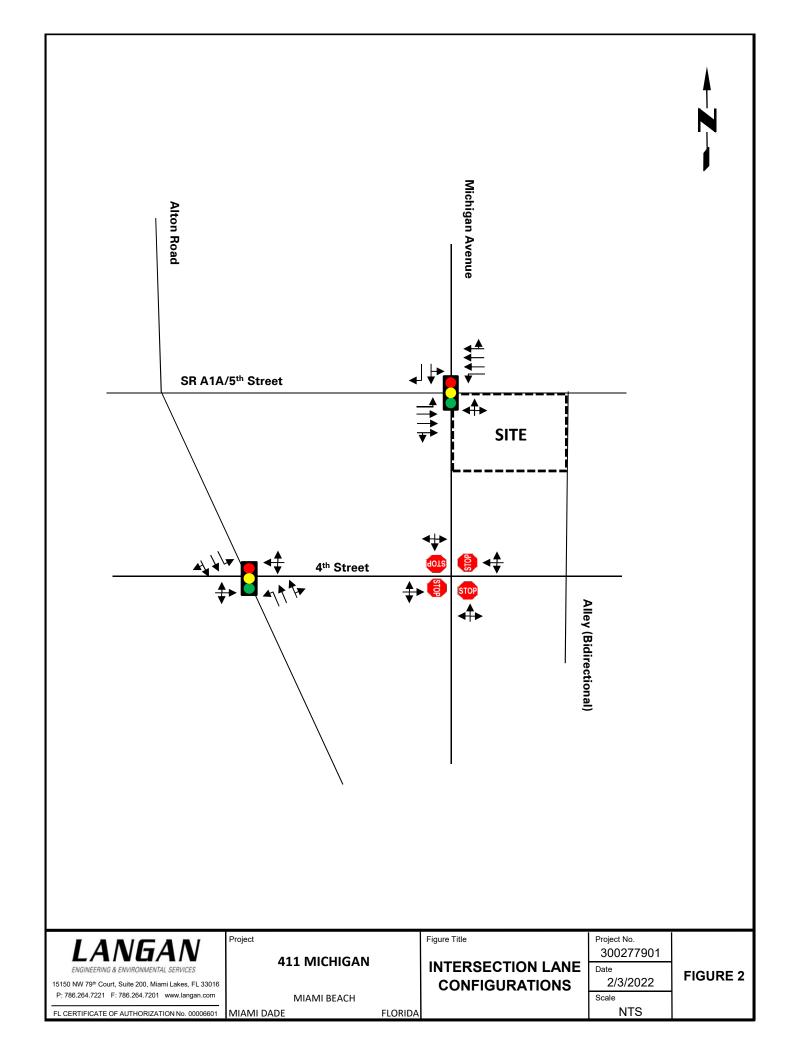
FLORIDA

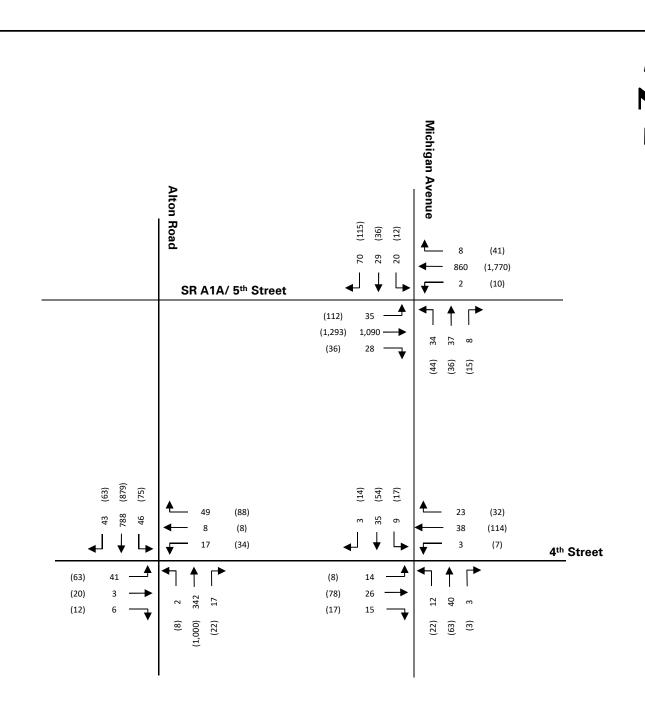
SITE LOCATION MAP

2/3/2022

FIGURE 1

Scale NTS





LEGEND

- # AM Peak Hour
- (#) PM Peak Hour

15150 NW 79th Court, Suite 200, Miami Lakes, FL 33016 P: 786.264.7221 F: 786.264.7201 www.langan.com

FL CERTIFICATE OF AUTHORIZATION No. 00006601

Project

411 MICHIGAN

MIAMI BEACH

Figure Title

2021 EXISTING TRAFFIC VOLUMES Project No. 300277901 Date

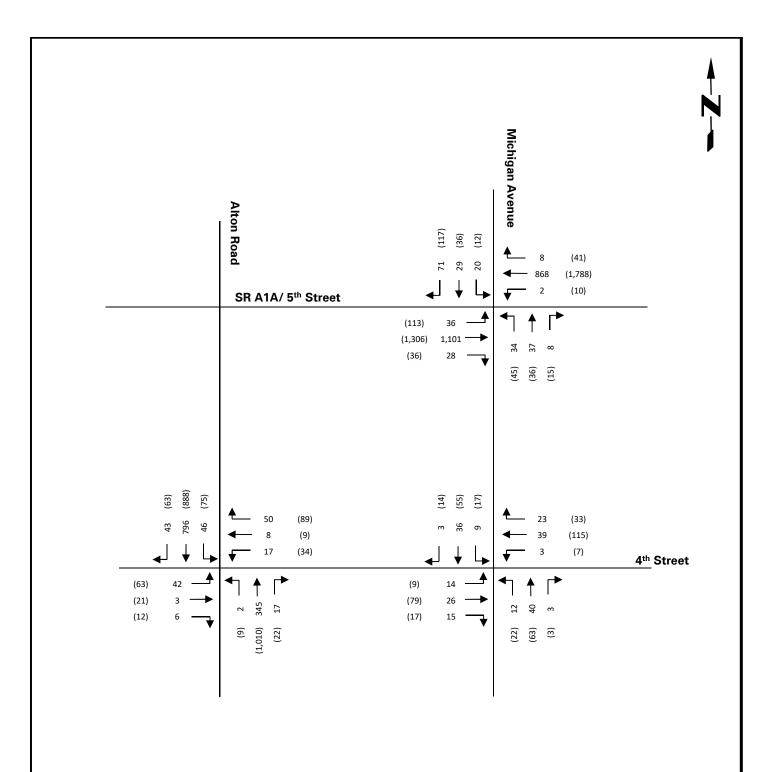
NTS

2/3/2022 Scale

FIGURE 3

MIAMI DADE

FLORIDA



LEGEND

- # AM Peak Hour
- (#) PM Peak Hour

LA		NGAN	
FNGINFFRING	R	ENVIRONMENTAL SERVICES	

15150 NW 79th Court, Suite 200, Miami Lakes, FL 33016 P: 786.264.7221 F: 786.264.7201 www.langan.com

FL CERTIFICATE OF AUTHORIZATION No. 00006601

Project

411 MICHIGAN

MIAMI BEACH MIAMI DADE

2023 NO BUILD TRAFFIC VOLUMES

Figure Title

300277901 Date

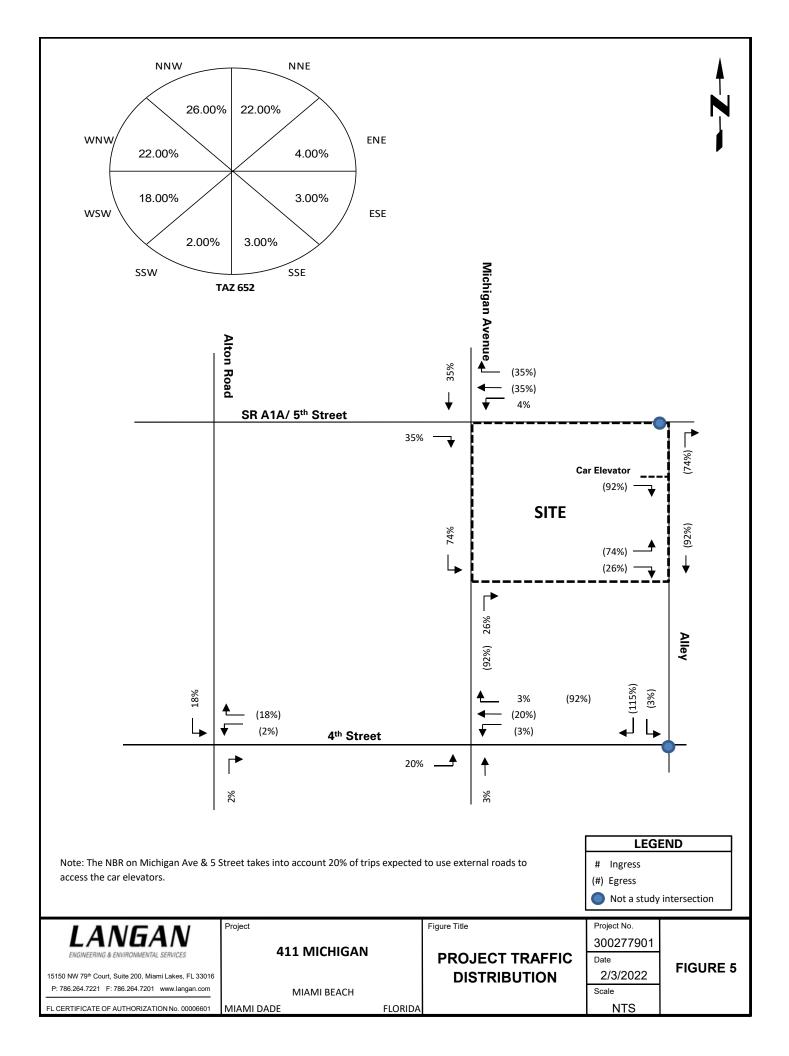
Project No.

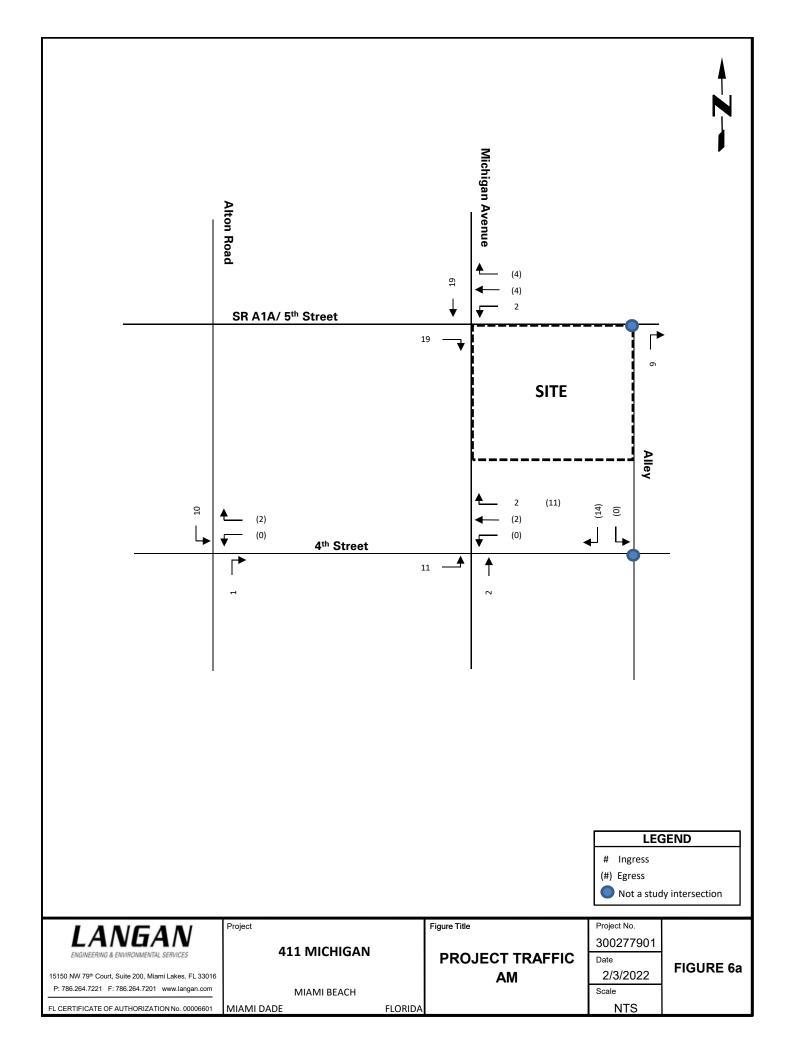
2/3/2022

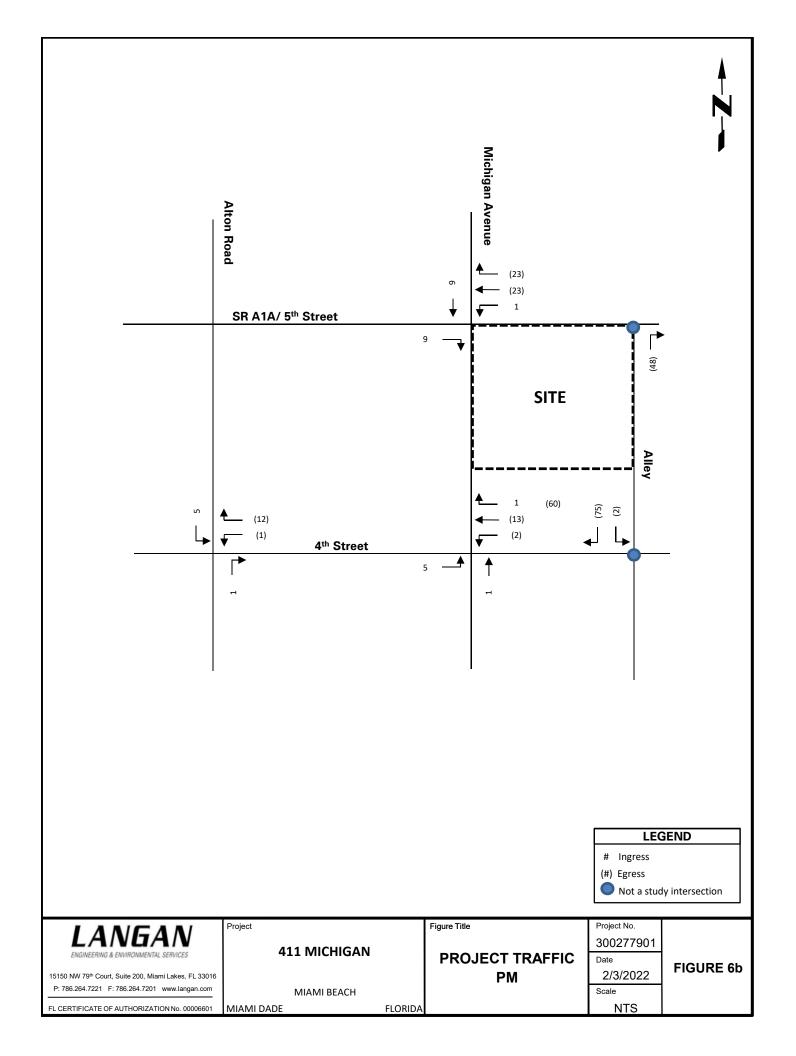
FIGURE 4

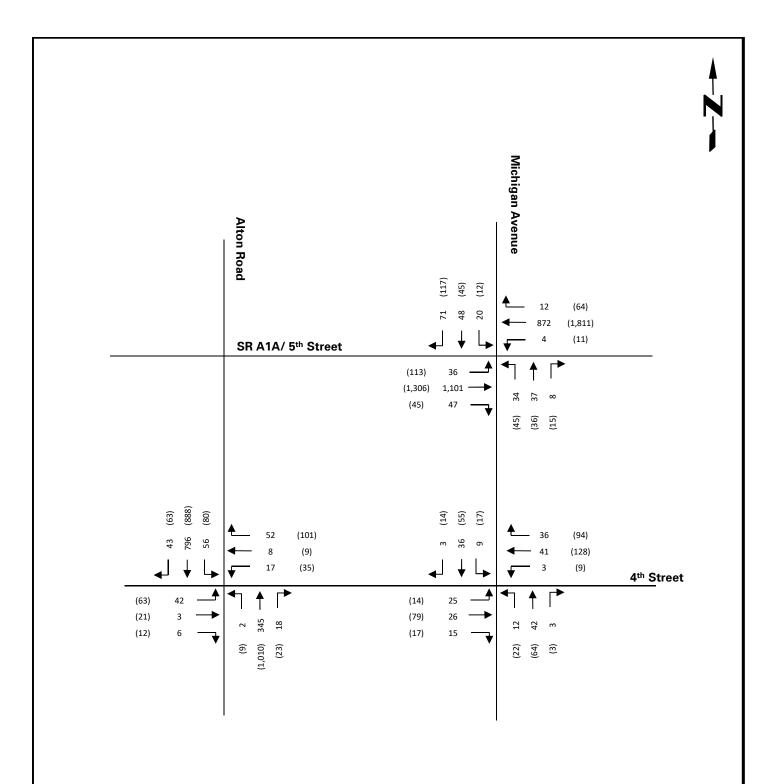
FLORIDA

Scale NTS









LEGEND

- # AM Peak Hour
- (#) PM Peak Hour

LANGAN

15150 NW 79th Court, Suite 200, Miami Lakes, FL 33016 P: 786.264.7221 F: 786.264.7201 www.langan.com

FL CERTIFICATE OF AUTHORIZATION No. 00006601

Project

411 MICHIGAN

MIAMI BEACH MIAMI DADE Figure Title

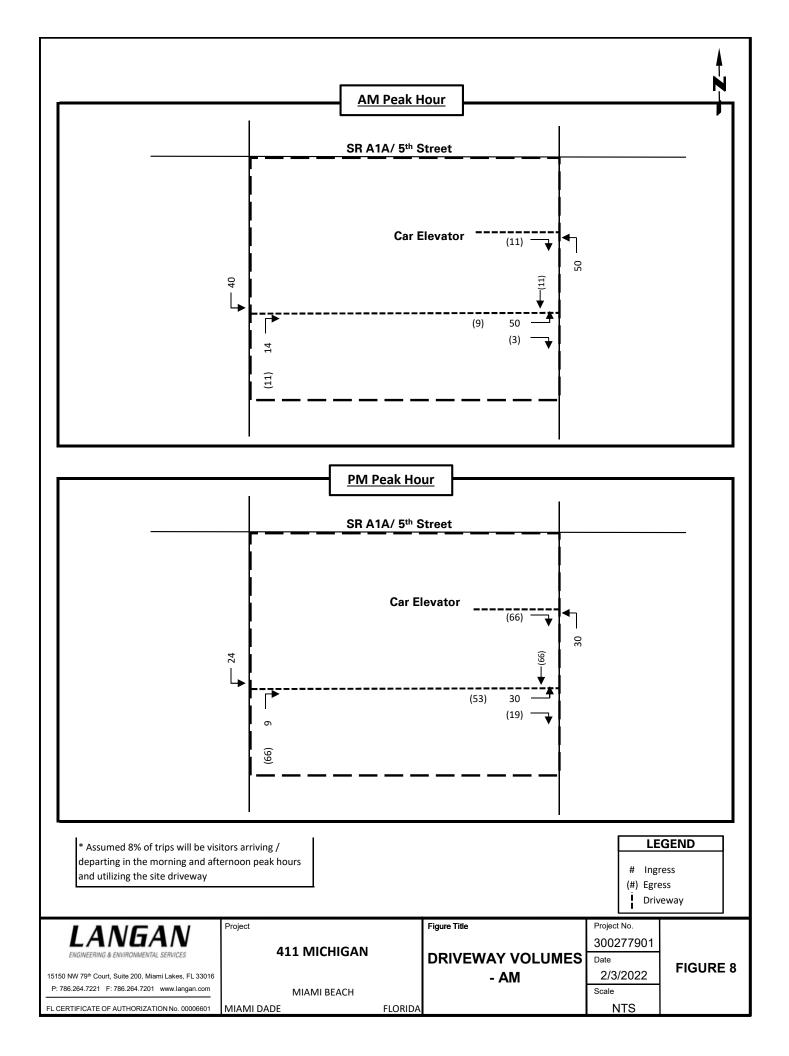
FLORIDA

2023 BUILD TRAFFIC VOLUMES

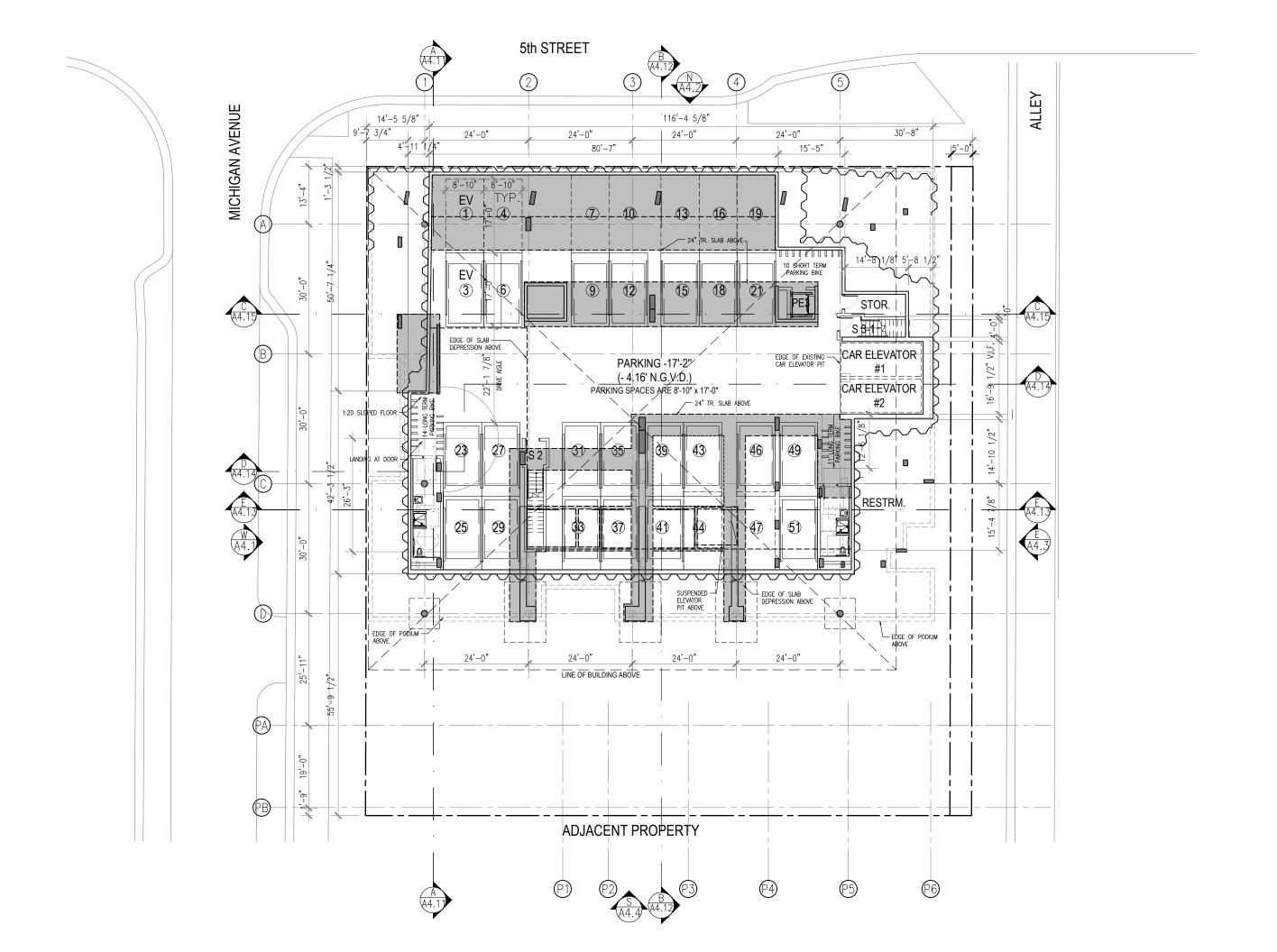
Project No. 300277901 Date

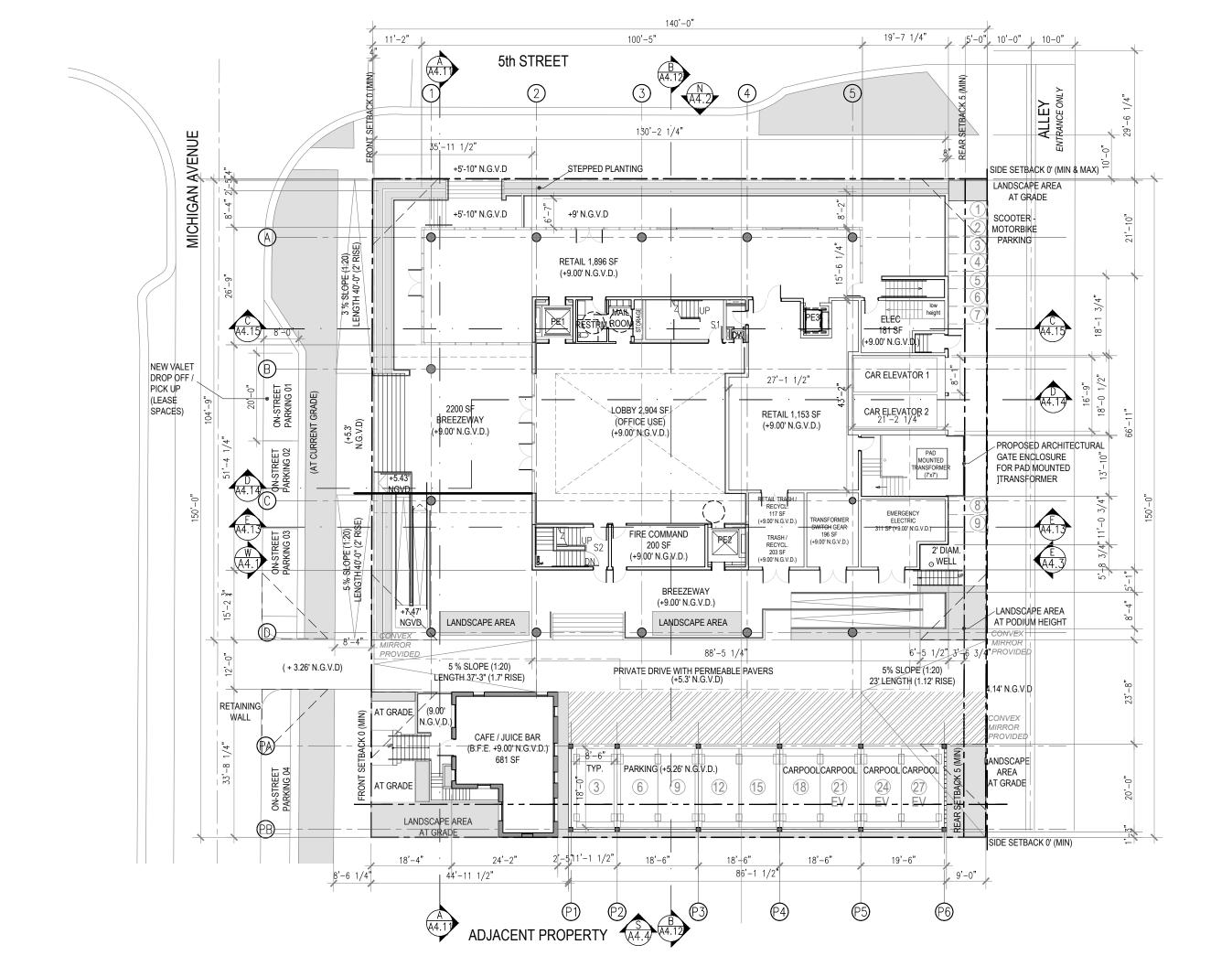
2/3/2022 FIG

Scale NTS FIGURE 7



APPENDIX B SITE PLAN







Planning Department, 1700 Convention Center Drive, 2nd Floor Miami Beach, Florida 33139, www.miamibeachfl.gov 305.673.7550

MULTIFAMILY - COMMERCIAL - ZONING DATA SHEET

ITEM #	Project Information							
1	Address:	411-419 Michigan Av	e, 944 5 Street					
2	Board and file numbers :	PB21-0469						
3	Folio number(s):	02-4203-010-0030, 0	02-4203-010-0030, 02-4203-009-6170, 02-4203-009-6160					
4	Year constructed:	N/A	Zoning District:	CPS-2				
5	Based Flood Elevation:	8	Grade value in NGVD:	4				
6	Adjusted grade (Flood+Grade/2):	6	Lot Area:	21,000				
7	Lot width:	140'	Lot Depth:	150'				
8	Minimum Unit Size	N/A	Average Unit Size	N/A				
9	Existing use:	N/A	Proposed use:	Commercial				

	Zoning Information / Calculations	Maximum	Existing	Proposed	Deficiencies
10	Height	75'	0'		Pursuant to in- process Code Amendment
11	Number of Stories	N/A	N/A	5	
12	FAR	42,000	0	41,936	
13	Gross square footage	N/A	9,500	92,356	
14	Square Footage by use	N/A	9,500	3,123 Retail, 38,813 Offi	ce
15	Number of units Residential	N/A	N/A	N/A	
16	Number of units Hotel	N/A	N/A	N/A	
17	Number of seats	N/A	N/A	N/A	
18	Occupancy load	N/A	N/A	N/A	

	Setbacks	Required	Existing	Proposed	Deficiencies
	Subterranean:				
19	Front Setback facing Michigan:	0	0	0	
20	Side Setback:	0	0	0	
22	Side Setback facing 5th street:	0	0	0	
23	Rear Setback facing Alley:	5'	10'	9'	
	At Grade Parking:				
24	Front Setback facing Michigan:	0	0	0	
25	Side Setback:	0	0	0	
27	Side Setback facing 5th street:	0	0	0	
28	Rear Setback Facing Alley:	5'	10'	9'	
	Pedestal and Tower:				
29	Front Setback facing Michigan:	0	0	4"	
30	Side Setback:	0	1'-6"	0	
31	Side Setback facing 5th street:	0	0	4"	
32	Rear Setback Facing Alley:	5'	10'	9'	
	Parking	Required	Existing	Proposed	Deficiencies
39	Parking District	1	1	1	
40	Total # of parking spaces	84	0	85	103 Required before Reductions (see chart)
41	# of parking spaces per use (Provide a separate chart for a breakdown calculation)	see chart	0	see chart	



Planning Department, 1700 Convention Center Drive, 2nd Floor Miami Beach, Florida 33139, www.miamibeachfl.gov 305.673.7550

42	# of parking spaces per level (Provide a separate chart for a breakdown calculation)	N/A	0	Basement - 58 Ground Floor -27	
43	Parking Space Dimensions	8.5' x 18'	0	8.5' x 18'	
44	Parking Space configuration (450, 600, 900,				
44	Parallel)	90	0	90	
45	ADA Spaces				
46	Tandem Spaces	N/A	0	15	
47	Drive aisle width	22'	0	22'	
48	Valet drop off and pick up	Υ	N	Υ	
49	Loading zones and Trash collection areas	3	0	1 in alley	Waiver Requested
50	Bicycle parking, location and Number of			25 Long Term in	
50	racks	0	0	Basement	

	Restaurants, Cafes, Bars, Lounges,				
	Nightclubs	Required	Existing	Proposed	Deficiencies
51	Type of use				
52	Number of seats located outside on private				
52	property	N/A	N/A	N/A	
53	Number of seats inside	N/A	N/A	N/A	
54	Total number of seats	N/A	N/A	N/A	
55	Total number of seats per venue (Provide a separate chart for a breakdown calculation)	N/A	N/A	N/A	
56	Total occupant content	N/A	N/A	N/A	
57	Occupant content per venue (Provide a separate chart for a breakdown calculation)	N/A	N/A	N/A	

58	Proposed hours of operation	8am-8pm
59	Is this an NIE? (Neighboot Impact	
59	stablishment, see CMB 141-1361)	N
60	Is dancing and/or entertainment proposed?	
60	(see CMB 141-1361)	N
61	Is this a contributing building?	Yes
62	Located within a Local Historic District?	Yes

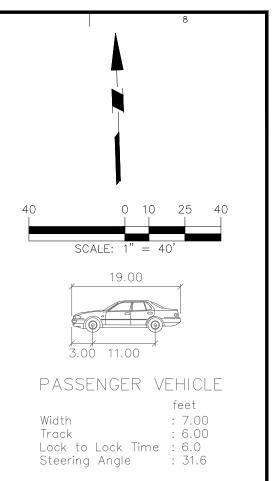
Notes:

If not applicable write N/A

N/A

<u> </u>		Total FAR SF	Required # Parking Space
Office or Office Building	Ground floor - One Space per 300 square feet of floor area	2,904	9.7
1	Upper floors - One space per 400 square feet of floor area		
	SF 2nd Floor	8,440	21.1
	SF Typical Floor (3-5) + Roof	24,795	62.0
	Total	36,139	
	Office or Office Building Required Parking		92.8 Spaces
Retail Parking (One space per 300 square feet of floor area		
	Ground Floor Retail Space (New Building)	3,123	10.41
	Retail Required Parking		10 Spaces
en e	TOTAL OFFICE + RETAIL PARKING REQUIRED		102.8 SPACES
Ilternative Parking Incentives - Sec. 130-40		Provided	# Spaces Reduced
Showers	2 parking less for each shower (max -8)	3	6
Scooters - Motorcycles	1 parking less for 3 parkings (max 15%)	5	5
Long term Bikes	1 parking less for 5 parkings (max 15%)	25	5
Short term Bikes	1 parking less for 10 parkings (max 15%)	0	0
Carpool / Vanpool	3 parking less for each parking (max 10%)	9	3
	Total # Reduced Parking Spaces		19
μι 	TOTAL PARKING REQUIRED AFTER DEDUCTIONS		83.8 SPACES
ng Spaces Delivered			
	Existing Cellar	58	
	· ·		
	Triple Stacker	27	





15150 NW 79th Court, Suite 200
Miami Lakes, FL 33016
T: 786.264.7200 F: 786.264.7201 www.langan.com
L Certificate of Authorization No. 00006601/LB8172/LB8198

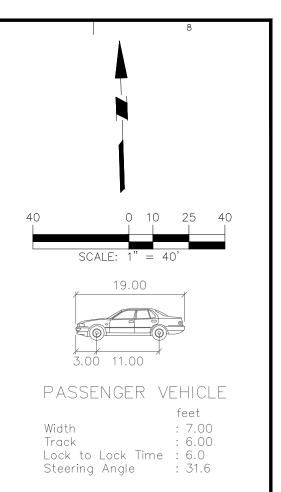
419 MICHIGAN AVENUE

VEHICLE CIRCULATION FIGURE

Project No. 300277901 Date 09/02/2021 Fig-001 , Drawn By

Checked By MP





Environmental Services, Inc.

15150 NW 79th Court, Suite 200
Miami Lakes, FL 33016
T: 786.264.7200 F: 786.264.7201 www.langan.com
L Certificate of Authorization No. 00006601/LB8172/LB8198

419 MICHIGAN AVENUE

MIAMI-DADE

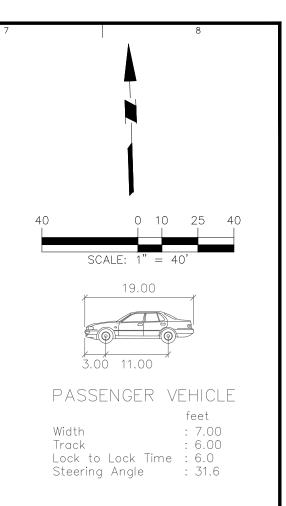
FLORIDA

VEHICLE CIRCULATION FIGURE

Project No. 300277901 Date 09/02/2021 , Drawn By Checked By MP

Fig-002





LANGAN

Langan Engineering and Environmental Services, Inc. 15150 NW 79th Court, Suite 200 Miami Lakes, FL 33016 T: 786.264.7200 F: 786.264.7201 www.langan.com

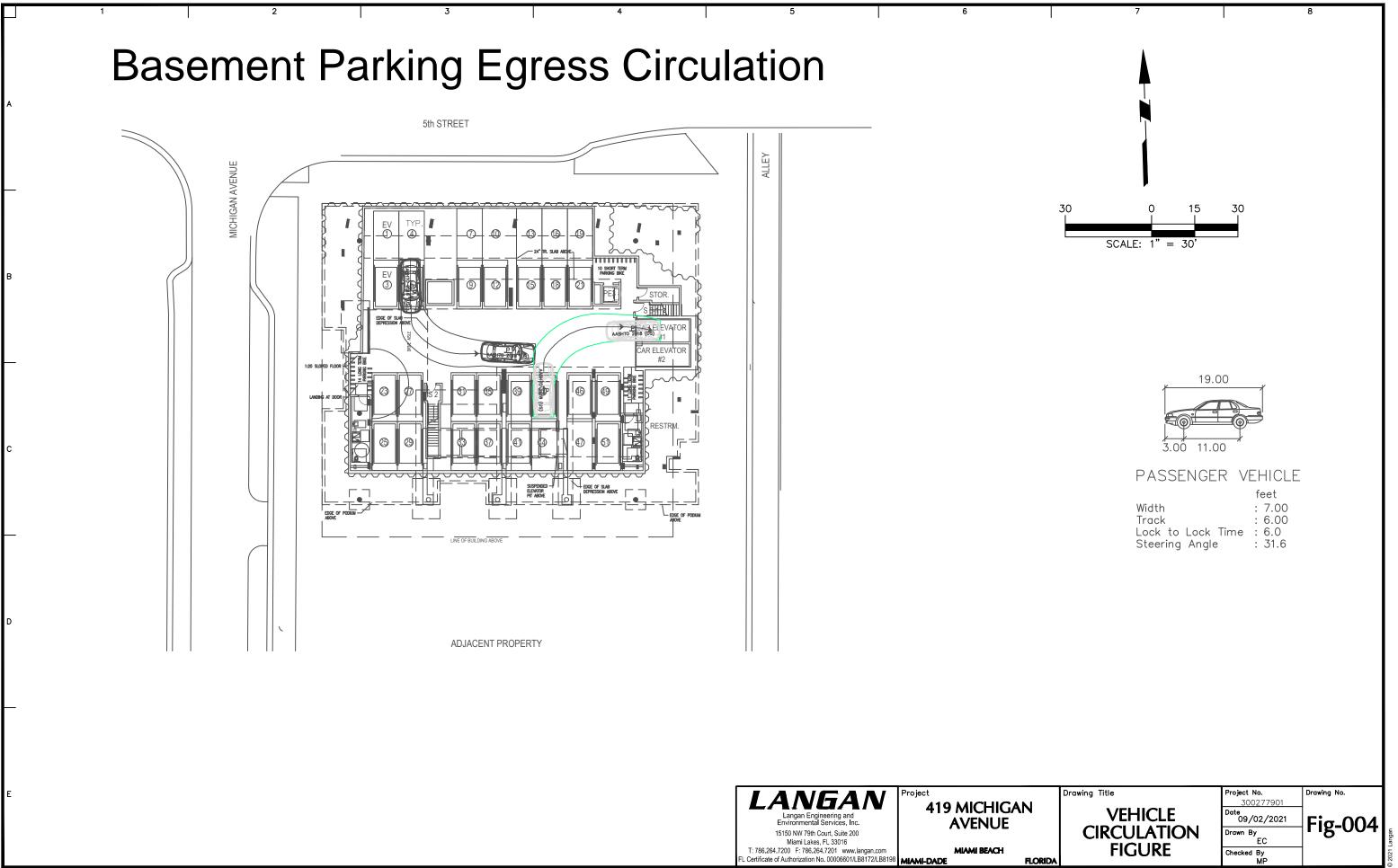
. Certificate of Authorization No. 00006601/LB8172/LB8198

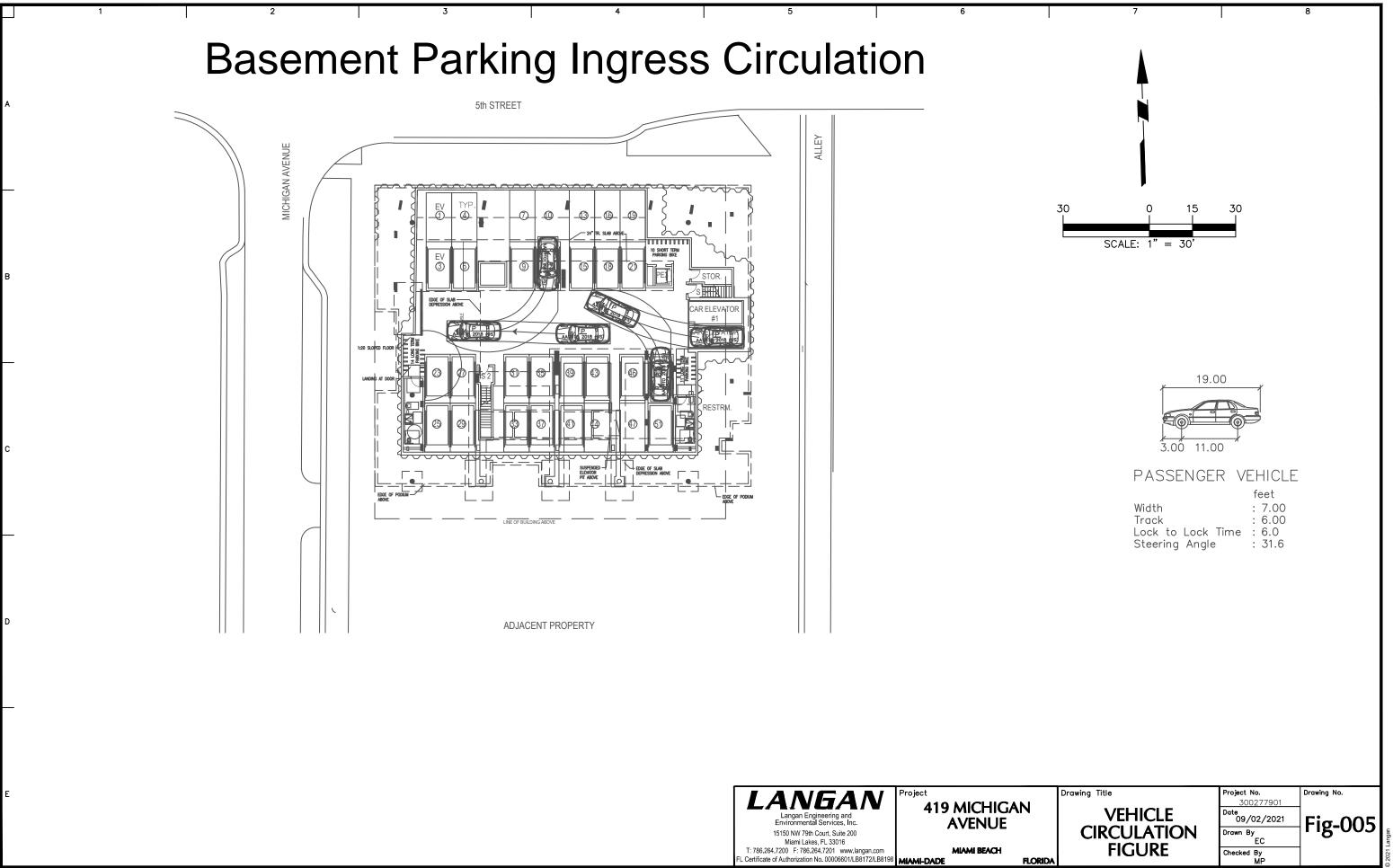
419 MICHIGAN AVENUE

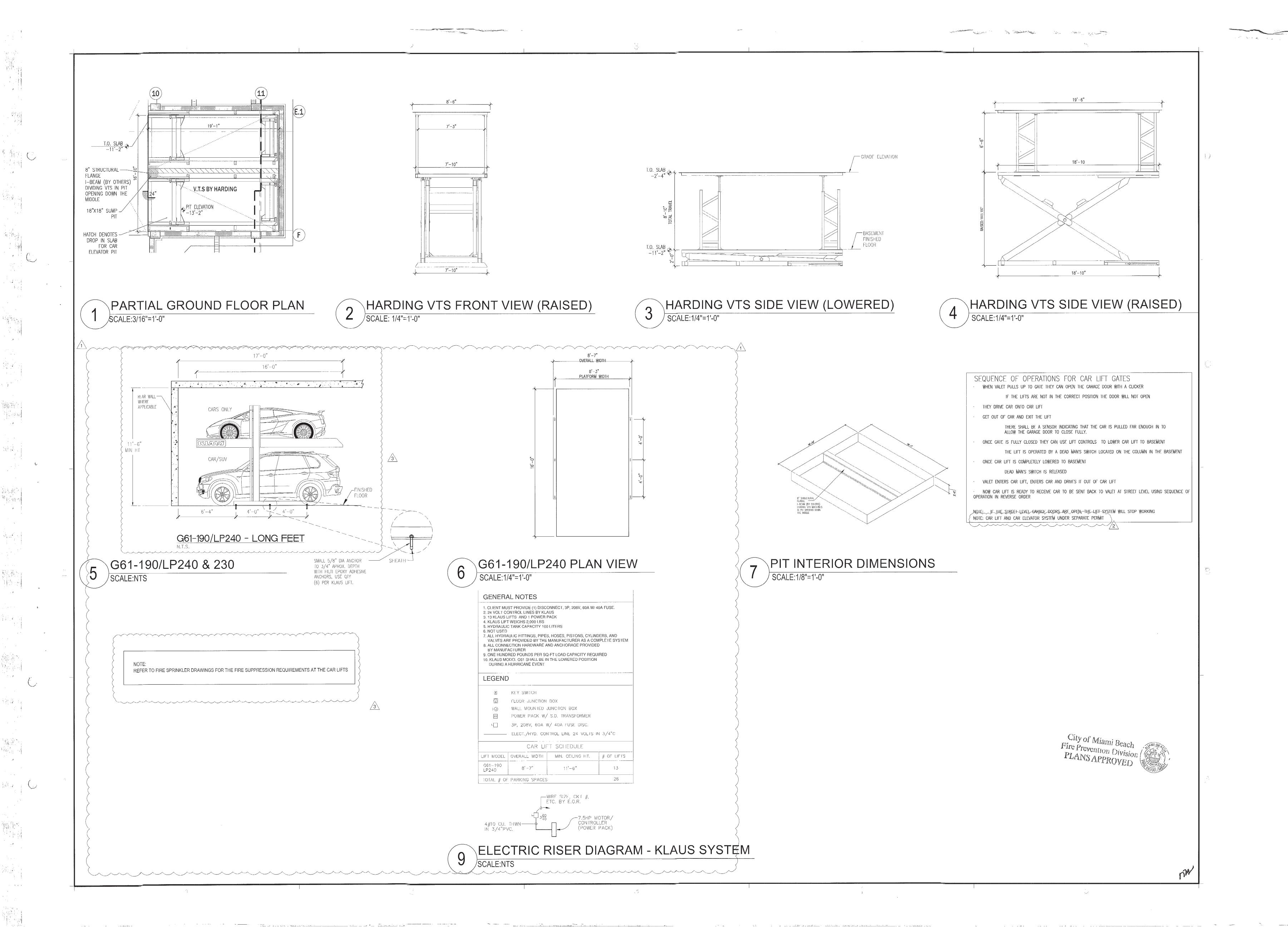
MIAMI BEACH

VEHICLE CIRCULATION FIGURE Project No. 300277901 Date 09/02/2021 Drawn By EC Checked By MP

Fig-003









OFFICE OF THE PROPERTY APPRAISER

Summary Report

Generated On: 8/17/2021

Property Information			
Folio:	02-4203-010-0030		
Property Address:	944 5 ST Miami Beach, FL 33139-6514		
Owner	411 MICHIGAN SOFI OWNER LLC		
Mailing Address	520 W 27 ST NEW YORK, NY 10022 USA		
PA Primary Zone	6503 COMMERCIAL		
Primary Land Use	1081 VACANT LAND - COMMERCIAL : VACANT LAND		
Beds / Baths / Half	0/0/0		
Floors	0		
Living Units	0		
Actual Area	0 Sq.Ft		
Living Area	0 Sq.Ft		
Adjusted Area	0 Sq.Ft		
Lot Size	7,000 Sq.Ft		
Year Built	0		

Assessment Information				
Year	2021	2020	2019	
Land Value	\$3,500,000	\$3,500,000	\$3,500,000	
Building Value	\$0	\$0	\$0	
XF Value	\$0	\$0	\$0	
Market Value	\$3,500,000	\$3,500,000	\$3,500,000	
Assessed Value	\$3,500,000	\$2,818,392	\$2,562,175	

Benefits Information				
Benefit	Туре	2021	2020	2019
Non-Homestead Cap Assessment Reduction \$681,608 \$937,825				
Note: Not all benefits are applicable to all Taxable Values (i.e. County, School				

Short Legal Description
3-4 54 42 34 53 42
WITHAMS RE-SUB PB 9-10
LOTS 1 TO 5 INC LESS N90FT
THEREOF FOR R/W BLK 83
LOT SIZE 140.000 X 50



Taxable Value Information				
	2021	2020	2019	
County				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,818,392	\$2,562,175	
School Board				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$3,500,000	\$3,500,000	
City				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,818,392	\$2,562,175	
Regional				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,818,392	\$2,562,175	

Sales Information				
Previous Sale	Price	OR Book- Page	Qualification Description	
06/11/2021	\$7,000,000	32578-2150	Qual on DOS, multi-parcel sale	
11/05/2020	\$2,100	32194-4520	Federal, state or local government agency	
06/11/2014	\$4,250,000	29190-2460	Qual on DOS, multi-parcel sale	
08/01/2000	\$765,000	19257-3689	Sales which are qualified	

The Office of the Property Appraiser is continually editing and updating the tax roll. This website may not reflect the most current information on record. The Property Appraiser and Miami-Dade County assumes no liability, see full disclaimer and User Agreement at http://www.miamidade.gov/info/disclaimer.asp

Version:

Board, City, Regional).



OFFICE OF THE PROPERTY APPRAISER

Summary Report

Generated On: 8/17/2021

Property Information		
Folio:	02-4203-009-6170	
Property Address:	419 MICHIGAN AVE Miami Beach, FL 33139-6509	
Owner	411 MICHIGAN SOFI OWNER LLC	
Mailing Address	520 W 27 ST NEW YORK, NY 10022 USA	
PA Primary Zone	6503 COMMERCIAL	
Primary Land Use	1081 VACANT LAND - COMMERCIAL : VACANT LAND	
Beds / Baths / Half	0/0/0	
Floors	0	
Living Units	0	
Actual Area	0 Sq.Ft	
Living Area	0 Sq.Ft	
Adjusted Area	0 Sq.Ft	
Lot Size	7,000 Sq.Ft	
Year Built	0	

Assessment Information				
Year	2021	2020	2019	
Land Value	\$3,500,000	\$3,500,000	\$3,500,000	
Building Value	\$0	\$0	\$0	
XF Value	\$0	\$0	\$0	
Market Value	\$3,500,000	\$3,500,000	\$3,500,000	
Assessed Value	\$3,500,000	\$2,137,837	\$1,943,489	

Benefits Information				
Benefit	Туре	202	1 2020	2019
Non-Homestead Assessment Reduction \$1,362,163 \$1,556,517				
Note: Not all honofits are applicable to all Tayable Values (i.e. County, School				

Note: Not all benefits are applicable to all Taxable Values (i.e. County, Schoo Board, City, Regional).

Short Legal Description
OCEAN BEACH ADD NO 3 PB 2-81
LOT 9 BLK 83
LOT SIZE 50.000 X 140
OR 19588-3015 0301 6



Taxable Value Information				
	2021	2020	2019	
County				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,137,837	\$1,943,489	
School Board				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$3,500,000	\$3,500,000	
City				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,137,837	\$1,943,489	
Regional				
Exemption Value	\$0	\$0	\$0	
Taxable Value	\$3,500,000	\$2,137,837	\$1,943,489	

Sales Information				
Previous Sale	Price	OR Book- Page	Qualification Description	
06/11/2021	\$7,000,000	32578-2150	Qual on DOS, multi-parcel sale	
11/05/2020	\$2,100	32194-4520	Federal, state or local government agency	
06/11/2014	\$4,250,000	29190-2460	Qual on DOS, multi-parcel sale	
03/01/2001	\$870,000	19588-3015	Other disqualified	

The Office of the Property Appraiser is continually editing and updating the tax roll. This website may not reflect the most current information on record. The Property Appraiser and Miami-Dade County assumes no liability, see full disclaimer and User Agreement at http://www.miamidade.gov/info/disclaimer.asp

Version:

APPENDIX C METHODOLOGY LETTER

Maximo Polanco

From: Akcay, Firat < FiratAkcay@miamibeachfl.gov>

Sent: Friday, July 23, 2021 10:56 AM

To: Maximo Polanco **Cc:** John Kim; Joe Goldberg

Subject: [External] RE: 419 Michigan Avenue Traffic Methodology

Maximo,

Pleasure to speak with you and your team. Please see notes from today's meeting. Please let me know if I missed anything that was discussed.

Parking queueing:

Triple Stacker: Parking queueing study.

Car elevator: Number of spaces is the limiting factor, provide queueing study. Identify alternative parking for spill over.

Narrative on identifying users of parking locations.

Circulation diagram for parking of vehicles. Valet operations plan and assumptions

Breezeway stacking diagram.

Roadway & Circulation:

Based on your queueing please indicate if the alley (Jerusalem Street) should be converted to NB instead.

Maneuverability Diagrams for the breezeway, alley loading and elevators and basement parking.

Intersection LOS Study:

Intersections: Michigan Ave x 4th and 5th Streets, and Alton Road and 4th Street.

Trip Distribution: Agreed on the methodology below.

Trip Gen:

Develop matrix with # of employees as well.

Average x fitted trip generation figures, assume fitted curve for a conservative analysis

Multimodal:

Provide bicycle parking.

Provide lockers and bicycle facilities, if feasible.

Clarification:

Retail portion will be restricted to shopping or other uses will be evaluated.



Firat Akcay, M.S.C.E. MBA
Transportation Engineer
Transportation and Mobility Department
1688 Meridian Avenue, Suite 801, Miami Beach, FL 33139
Tel: 305-673-7000, ext 26839

We are committed to providing excellent public service and safety to all who live, work and play in our vibrant, tropical, historic community.



From: Maximo Polanco <mpolanco@langan.com>

Sent: Tuesday, July 20, 2021 9:59 AM

To: Akcay, Firat <FiratAkcay@miamibeachfl.gov>

Cc: John Kim <jkim@langan.com>; Joe Goldberg <jgoldberg@langan.com>

Subject: RE: 419 Michigan Avenue Traffic Methodology

[THIS MESSAGE COMES FROM AN EXTERNAL EMAIL - USE CAUTION WHEN REPLYING AND OPENING LINKS OR ATTACHMENTS]

Hi Firat,

We have scheduled the meeting for Friday 23rd at 10:00 AM. Here is a narrative and plans to discuss in our meeting.

Proposed Traffic Methodology for 419 Michigan

The proposed development is a mixed-use development of office uses and retail uses to be constructed on three parcels (Folio Nos. 02-4203-010-0030; 02-4203-009-6170 and 02-4203-009-6160). The proposed development will move the existing historical house to be adjacent to Michigan Avenue to maximize the developable area of the site. The development will use the existing foundations of the previously approved development and construct a parking lot in the basement that will be accessed through a vehicle elevator. The proposed development will be served by a valet parking operation that all patrons will be have to use. The development will have triple vehicle stackers on the ground floor and double stackers in most of the spaces of the basement floor. Please find attach a schematic design of the proposed development.

Based on our understanding we propose the following tasks for the traffic-impact analysis for the proposed mixed-use development at 419 Michigan Avenue, Miami Beach, FL.

Data Collection

- Collect morning (7 to 9 AM) and afternoon (4 to 6 PM) peak-hour vehicle turning-movement volumes at the following study intersections:
 - Michigan Avenue & SR-A1A
 - Michigan Avenue & 4th Street
- Collect 24-hour bidirectional counts at the roadway segment of SR-A1A between Michigan & Washington avenues.
- Develop a COVID-adjustment factor by comparing PRE-COVID traffic data to 2021 traffic data along the segment of SR-A1A to convert the traffic data into peak-season volumes.
- Adjust the peak-season volumes with FDOT's 2019 PSCF.
- <u>Trip Generation</u> will be based on information contained in the Institute of Transportation Engineer's (ITE), Trip Generation Manual, 10th Edition.
- <u>Project Distribution</u> will be based on the cardinal distribution of the Traffic Analysis Zone 652 of the Miami-Dade County 2045 Transportation Model.
- <u>Future traffic</u> volumes will be developed by applying a compound growth rate to the collected traffic data. The growth rate will be based on a FDOT historical data from traffic count stations in the vicinity of the project. A one-half percent annual growth rate will be used if a negative growth rate is determined.
 - o We will include any approved and unbuilt projects that can be provided by the City of Miami Beach.

- We will include any roadway improvement planned within the first three years of the county's Transportation Improvement Program.
- <u>Intersection capacity analyses</u> will be performed for the study intersections using software based on the Highway Capacity Manual methodology. The analysis will be performed for the morning and afternoon peakhours conditions using Synchro software.
 - The analysis scenarios will include the existing (2021), no-build (2023 without project) and build (2023 with project). Conditions.
 - Project Driveways will be analyzed for the build conditions.
- **Queueing Analysis** We will prepare a queuing analysis for the proposed valet operation to determine the number of required valet-parking attendant to avoid traffic to queue back onto the adjacent public roadways.
 - The analysis will be based on the queuing-analysis methodology from the Transportation and Land Development published by the ITE. This methodology requires hourly rates of arrival and service times for the valet parking to determine queue lengths.

Table 1 - Trip Generation Estimates *

Use	Size	Dally	Weekday	Morning F	Peak Hour	Weekda	ay Afternoo
Use	Size	Daily	In	Out	Total	In	Out
Proposed Uses							
General Office	36,442 SF	399	36	6	42	7	35
Shopping Center**	4,320 SF	710	2	2	4	16	18
	Total	1,109	38	8	46	23	53

^{*} Based on Trip Generation Manual 10th Ed.

Regards,

Maximo Polanco Senior Staff Engineer

LANGAN

Direct: 954.320.2155 Mobile: 305.570.8593 File Sharing Link www.langan.com

FLORIDA NEW JERSEY NEW YORK CONNECTICUT MASSACHUSETTS PENNSYLVANIA WASHINGTON, DC VIRGINIA OHIO ILLINOIS TEXAS ARIZONA COLORADO WASHINGTON CALIFORNIA ATHENS CALGARY DUBAI LONDON PANAMA

A Carbon-Neutral Firm | Langan's goal is to be SAFE (Stay Accident Free Everyday)

Build your career with a premier firm. Join Langan.

From: Akcay, Firat <FiratAkcay@miamibeachfl.gov>

Sent: Monday, July 19, 2021 2:54 PM

To: Maximo Polanco < mpolanco@langan.com >

Cc: John Kim < jkim@langan.com>; Joe Goldberg < jgoldberg@langan.com>

Subject: [External] RE: 419 Michigan Avenue Traffic Methodology

Maximo,

Please see my availability via the attached link: https://calendly.com/d/xfxb-s823/30-minute-meeting Thank you

^{**} Includes 36% afternoon pass-by.



Firat Akcay, M.S.C.E. MBA
Transportation Engineer
Transportation and Mobility Department
1688 Meridian Avenue, Suite 801, Miami Beach, FL 33139
Tel: 305-673-7000, ext 26839

We are committed to providing excellent public service and safety to all who live, work and play in our vibrant, tropical, historic community.



Please do not print this e-mail unless necessary.

From: Maximo Polanco < mpolanco@langan.com >

Sent: Wednesday, July 14, 2021 9:08 AM

To: Akcay, Firat < FiratAkcay@miamibeachfl.gov >

Cc: John Kim < <u>jkim@langan.com</u>>; Joe Goldberg < <u>jgoldberg@langan.com</u>>

Subject: 419 Michigan Avenue Traffic Methodology

[THIS MESSAGE COMES FROM AN EXTERNAL EMAIL - USE CAUTION WHEN REPLYING AND OPENING LINKS OR ATTACHMENTS]

Good morning Firat,

Can we schedule a conference call to discuss a proposed mixed-use development at 419 Michigan Avenue.

Please let us know your availability.

Regards,

Maximo Polanco Senior Staff Engineer

LANGAN

Direct: 954.320.2155 Mobile: 305.570.8593 File Sharing Link

Phone: 954.320.2100 Fax: 954.320.2101 110 East Broward Boulevard, Suite 1500

Fort Lauderdale, FL 33301

www.langan.com

FLORIDA NEW JERSEY NEW YORK CONNECTICUT MASSACHUSETTS PENNSYLVANIA WASHINGTON, DC VIRGINIA OHIO ILLINOIS TEXAS ARIZONA COLORADO WASHINGTON CALIFORNIA ATHENS CALGARY DUBAI LONDON PANAMA

A Carbon-Neutral Firm | Langan's goal is to be SAFE (Stay Accident Free Everyday)

Build your career with a premier firm. Join Langan.



This message may contain confidential, proprietary, or privileged information. Confidentiality or privilege is not intended to be waived or lost by erroneous transmission of this message. If you receive this message in error, please notify the sender immediately by return email and delete this message from your system. Disclosure, use, distribution, or copying of a message or any of its attachments by anyone other than the intended recipient is strictly prohibited. This message may contain confidential, proprietary, or privileged information. Confidentiality or privilege is not intended to be waived or lost by erroneous transmission of this message. If you receive this message in error, please notify the sender immediately by return email and delete this message from your system. Disclosure, use, distribution, or copying of a message or any of its attachments by anyone other than the intended recipient is strictly prohibited.

APPENDIX D TRAFFIC, TAZ, SIGNAL TIMING DATA, CENSUS DATA & FDOT TABLES

National Data & Surveying Services Intersection Turning Movement Count

Location: Michigan Ave & 5th St City: Miami Beach Control: Signalized

Project ID: 21-140172-003 **Date:** 8/5/2021

,							_	Data - Tota	Total								
NS/EW Streets:		Michigan Ave	n Ave			Michigan Ave	η Ave			5th St	St			5th St	St		
		NORTHBOUND	BOUND			SOUTHBOUND	SOUND			EASTBOUND	DNNC			WESTE	WESTBOUND		
Σ	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	Ŋ	N	NR R	₽	SF	ST	SR	S	ᆸ	Ш	Ж	EU	WL	LΜ	WR	M	TOTAL
7:00 AM	2	1	0	0	3	1	9	0	3	126	2	0	0	108	3	0	255
7:15 AM	٣		0	0	0		4	0	9	115	٣	0		121	0	0	255
7:30 AM	2	2	-	0		4	13	0	0	115	1	0		92	0	0	238
7:45 AM	٣		0	0	2	2	7	-	2	148	٣	1	2	106	က	_	288
8:00 AM	3	7	1	0	2	3	10	0	2	156	2	0	1	115	2	0	307
8:15 AM	7	٣	-	0	1	2	6	0	7	184	9	0	0	140	0	0	363
8:30 AM	7	6	2	0	2	2	15	-	9	182	9	0	0	137	2	0	377
8:45 AM	2	2		0	4	9	12	0	4	189	4	1	0	169		0	401
	٦N	LN	NR	NO	SF	ST	SR		П		ER	EU	ML	MT	WR	NM	TOTAL
TOTAL VOLUMES:	35	29	9	0	18	30	9/		36		27	2	2	991	11	П	2484
APPROACH %'s:	20.00%	41.43%	8.57%	0.00%	14 29%	23.81%	60.32%	1.59%	2.81%	94.92%	2.11%	0.16%	0.50%	98.31%	1.09%	0.10%	
PEAK HR:		08:00 AM - 09:00 AM	MA 00:60														TOTAL
PEAK HR VOL :	22	24	2	0	12	19	46	-	22	711	18	1	1	561	2	0	1448
PEAK HR FACTOR:	0.786	0.667	0.625	0000		0.792	0.767	0.250	0.786	0.940	0.750	0.250	0.250	0.830	0.625	0.00	000
		0.708	38			0.750	0.			0.949	6:			0.834	34		202.0

		NORTHBOUND	BOUND			SOUTH	BOUND			EASTBOUND	OUND			WESTBOUND	30UND		
≥	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	¥	۲	NR	N	SF	ST	SR	SO	ᆸ	Ь	띪	3	WL	MT	WR	M	TOTAL
4:00 PM	14	80	2	0	3	2	18	0	11	183	6	2	1	237	1	0	491
4:15 PM	15	4	က	0	9	7	16	0	12	168	2	4	-	566	٣	1	511
4:30 PM	18	7	က	0	4	٣	14	0	22	192	2	1	0	241	4	0	514
4:45 PM	4	∞	1	0	2	2	12	0	19	196	7	m	0	247	7	0	208
5:00 PM	œ	9	3	0	2	œ	18	0	16	185	3	2	2	234	9	1	494
5:15 PM	6	٣	2	0	0	2	21	1	13	205	7	0	0	569	2	-	544
5:30 PM	2	4	0	0	2	9	17	0	10	176	4	က	2	293	9	0	528
5:45 PM	9	2	2	0	က	2	14	0	14	154	2	-	2	262	0	0	470
	N۲	LN	NR	N	SF	ST	SR	SU	П	Ш	ER	EU	ML	MT	WR	MU	TOTAL
TOTAL VOLUMES:	79	45	19	0	22	38	130	1	117	1459	42	16	œ	2049	32	m	4060
APPROACH %'s:	55.24%	31.47%	13.29%	0.00%	11.52%	19.90%	%90 89	0.52%	7.16%	89.29%	2.57%	%86 0	0.38%	97.94%	1.53%	0.14%	
PEAK HR:)	04:45 PM - 05:45 PM	05:45 PM														TOTAL
PEAK HR VOL:	56	21	6	0	9	21	89	П	28	762	21	80	4	1043	24	2	2074
PEAK HR FACTOR:	0.722	0.656	0.450	0.000	0.750	0.656	0.810	0.250	0.763	0.929	0.750	0.667	0.500	0.890	0.857	0.500	2
		0.824	4.			0.857	27			0.943	13			0.891	91		0.953

PEAK HR		- MS 00:8	:00 AM - 9:00 AM														TOTAL
PEAK HR VOLUME	77	24	5	0	12	19	46	1	22	711	18	-	-	561	2	0	1448
PEAK HR		4:45 PM -	:45 PM - 5:45 PM														TOTAL
PEAK HR VOLUME	56	21	6	0	9	21	89	1	58	762	21	8	4	1043	24	2	2074

National Data & Surveying Services Intersection Turning Movement Count

Location: Michigan Ave & 5th St City: Miami Beach Control: Signalized

Project ID: 21-140172-003 Date: 8/5/2021

	га	В		

								Data -	Bikes								
NS/EW Streets	s:	Michig	an Ave			Michiga	in Ave			5th	St			5th	St		
		NORTI	HBOUND			SOUTH	BOUND			EASTE	OUND			WESTE	BOUND		
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 A		1	0	0	0	0	0	0	0	1	0	0	0	1	0	0	3
7:15 A		0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
7:30 A		0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3
7:45 A		1	0	0	0	2	0	0	0	4	0	0	0	0	0	0	7
8:00 A		0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
8:15 A		0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	2
8:30 A		0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2
8:45 A	М 0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES	: 1	3	0	0	0	5	1	0	0	8	0	0	0	3	0	0	21
APPROACH %'s	25.00	% 75.00%	0.00%	0.00%	0.00%	83.33%	16.67%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	
PEAK HR	1:	08:00 AM	- 09:00 AM														TOTAL
PEAK HR VOL		1	0	0	0	3	1	0	0	1	0	0	0	0	0	0	7
PEAK HR FACTOR	0.250	0.250	0.000	0.000	0.000	0.750	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.875
		0.!	500			0.5	00			0.2	50						0.073
200			HBOUND			SOUTH				EASTE				WESTE			
PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 P		1	0	0	0	2	0	0	0	0	2	0	1	2	2	0	10
4:15 P		1	0	0	0	1	0	0	1	1	0	0	1	1	0	0	6
4:30 P		0	0	0	0	2	0	0	0	0	0	0	0	1	0	0	4
4:45 P		0	0	0	0	2	0	0	0	0	0	0	0	2	0	0	
5:00 P 5:15 P		0	0	0	0	6	0	0	0	1	0	0	0	3	1	0	6 9
5:30 P		0	0	0	1	2	0	0	0	3	0	0	0	4	1	0	12
5:45 P		4	0	0	0	2	0	0	0	3	0	0	0	1	0	0	11
3.73 F	111	•	0						•	,		Ť					
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES		7	0	0	1	20	0	0	2	9	2	0	3	15	4	0	67
APPROACH %'s				0.00%	4.76%	95.24%	0.00%	0.00%	15.38%	69.23%	15.38%	0.00%	13.64%	68.18%	18.18%	0.00%	
PEAK HR			- 05:45 PM														TOTAL
PEAK HR VOL		1	0	0	1	13	0	0	0	5	0	0	1	10	2	0	36
PEAK HR FACTOR	0.750	0.250	0.000	0.000	0.250	0.542	0.000	0.000	0.000	0.417	0.000	0.000	0.250	0.625	0.500	0.000	0.750
		0.5	500			0.5	83			0.4	17			0.6	50		0.750

National Data & Surveying Services Intersection Turning

Movement Count

Project ID: 21-140172-003

Date: 8/5/2021

Location: Michigan Ave & 5th St **City:** Miami Beach

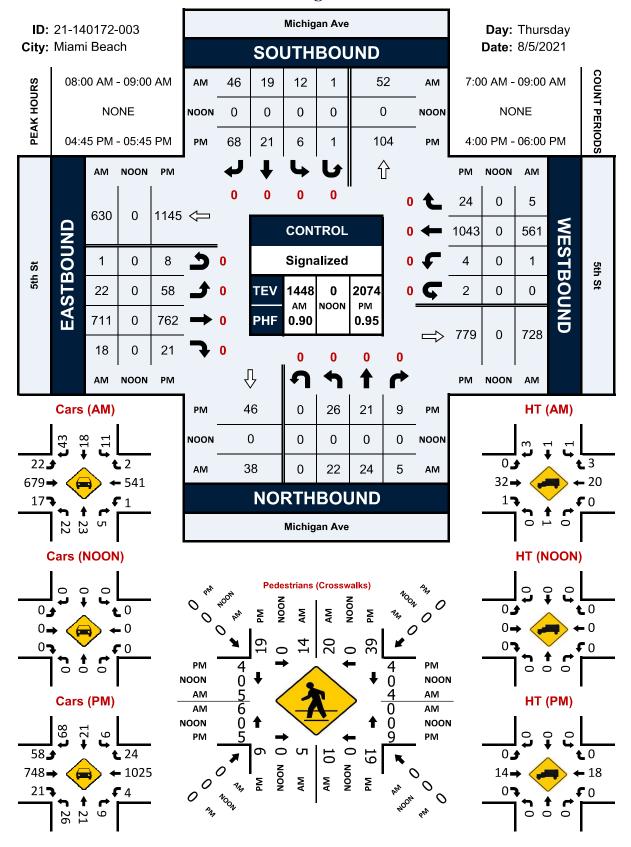
Data - Pedestrians (Crosswalks)

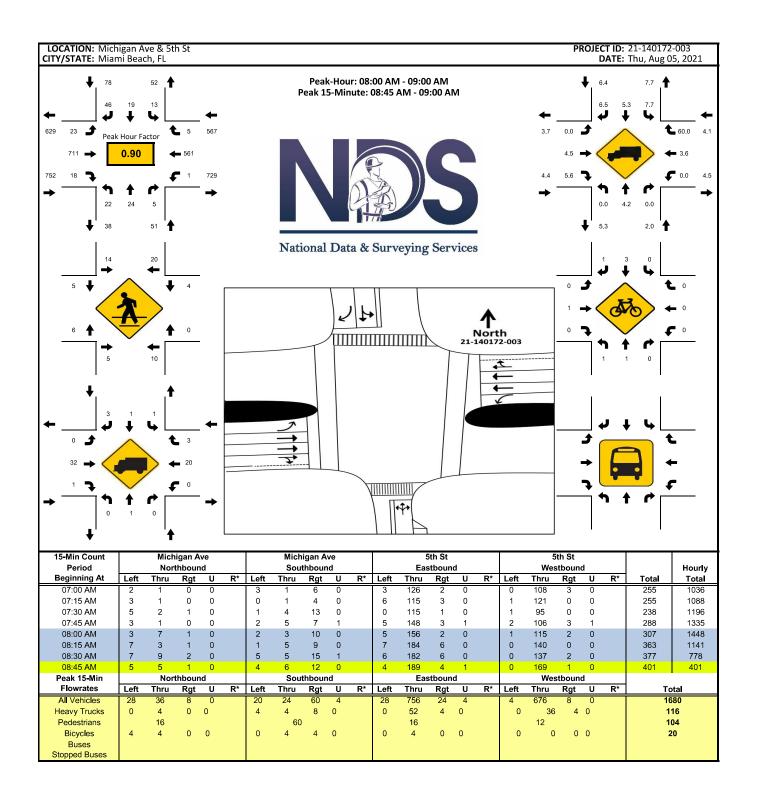
NS/EW Streets:	Michig	an Ave	Michig	an Ave	5tl	h St	5th	n St	
A	NORT	H LEG	SOUT	H LEG	EAS	T LEG	WES ⁻	T LEG	
AM	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
7:00 AM	0	4	4	0	1	0	0	0	9
7:15 AM	1	2	2	1	1	0	1	3	11
7:30 AM	7	4	0	3	0	1	0	1	16
7:45 AM	2	6	1	1	2	1	2	2	17
8:00 AM	3	3	3	1	0	0	2	1	13
8:15 AM	4	4	1	2	0	1	0	2	14
8:30 AM	0	5	0	4	0	0	3	1	13
8:45 AM	7	8	1	3	0	3	1	1	24
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
TOTAL VOLUMES:	24	36	12	15	4	6	9	11	117
APPROACH %'s:	40.00%	60.00%	44.44%	55.56%	40.00%	60.00%	45.00%	55.00%	
PEAK HR:	08:00 AM	- 09:00 AM							TOTAL
PEAK HR VOL:	14	20	5	10	0	4	6	5	64
PEAK HR FACTOR:	0.500	0.625	0.417	0.625		0.333	0.500	0.625	0.667
	0.5	567	0.9	938	0.	333	0.6	588	0.667

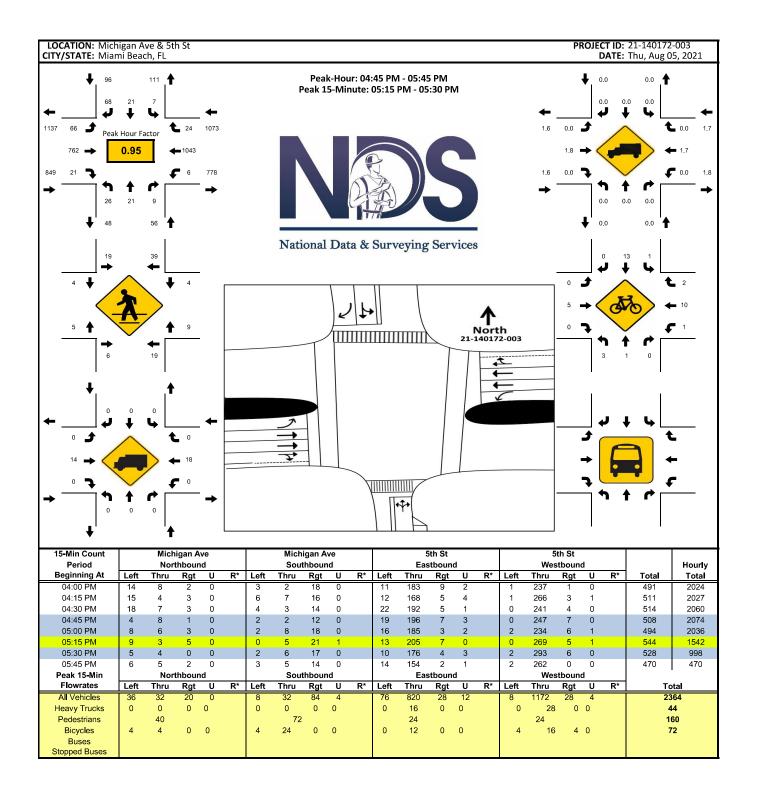
DM	NORT	H LEG	SOUT	H LEG	EAS ⁻	Γ LEG	WES	T LEG	
PM	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
4:00 PM	2	5	2	1	0	3	2	4	19
4:15 PM	11	7	4	3	0	0	3	2	30
4:30 PM	9	8	2	3	0	1	3	2	28
4:45 PM	3	9	2	6	1	2	2	0	25
5:00 PM	5	8	0	4	1	0	0	0	18
5:15 PM	5	10	2	8	1	2	0	1	29
5:30 PM	6	12	2	1	6	0	3	3	33
5:45 PM	6	16	4	2	0	2	1	4	35
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
TOTAL VOLUMES:	47	75	18	28	9	10	14	16	217
APPROACH %'s:	38.52%	61.48%	39.13%	60.87%	47.37%	52.63%	46.67%	53.33%	
PEAK HR :	04:45 PM	- 05:45 PM							TOTAL
PEAK HR VOL :	19	39	6	19	9	4	5	4	105
PEAK HR FACTOR :	0.792	0.813	0.750	0.594	0.375	0.500	0.417	0.333	0.795
	0.0	306	0.6	525	0	542	0.3	375	0.795

Michigan Ave & 5th St

Peak Hour Turning Movement Count







National Data & Surveying Services Intersection Turning Movement Count

Location: Alton Rd & 4th St City: Miami Beach Control: Signalized

Project ID: 21-140172-001 **Date:** 8/5/2021

ignalized

•							1	Data - Tota	Total					,	(- (-		
NS/EW Streets:		Alton Rd	Rd			Alton Rd	Rd			4th St	**			4th St	St		
		NORTHBOUND	BOUND			SOUTHBOUND	30UND			EASTBOUND	DUND			WESTBOUND	30UND		
ΔA	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	N	LN	NR	NO	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	42	1	0	4	29	9	0	4	0	1	0	3	-	4	0	133
7:15 AM	0	31		1	2	71	4	0	2		0	0		0	7	0	121
7:30 AM	0	47	2	0	4	73	4	-	7			0		0	2	0	146
7:45 AM		52	2	0	2	68	4	-	9	0		0		0	2	0	164
8:00 AM	0	41	4	0	7	100	3	1	9	0	1	0	1	0	4	0	168
8:15 AM	0	52	4	0	80	116	6	-	3	1	2	0	8		80	0	208
8:30 AM	0	63	2	0	7	117	9	0	6	1	1		1	7	11	0	221
8:45 AM	-	29	1	0	2	181	10	1	∞	0	0	0	9	2	6	0	291
	٦N	LN	NR	NN	TS	ST		SN	E	ET	ER	EU	ML	MT		NM	TOTAL
TOTAL VOLUMES:	2	395	17	П		814	46	2	45	4	7	1		9	23	0	1452
APPROACH %'s:	0 48%	0 48% 95 18%	4.10%	0.24%	4.31%	90.04%		0.55%	78.95%	7.02%	12.28%	1.75%	22.37%	7.89%		0.00%	
PEAK HR:)	08:00 AM - 09:00 AM	09:00 AM														TOTAL
PEAK HR VOL :	1	223	11	0	27	514	28	n	56		4		11	2	32	0	888
PEAK HR FACTOR:	0.250	0.832	0.688	0000	0.844	0,710	0.700	0.750	0.722	0.500	0.500	0.250	0.458	0.625	0.727	0.00	222
		0.851	51			0.726	9			0.688	8			0.706	90		0.702

		NORTHBOUND	BOUND			SOUTHBOUNE	30UND			EASTBOUND	OUND			WESTBOUND	OUND		
≥	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	٦	ħ	NR R	2	S	ST	SR	S	ᆸ	Ы	H	<u> </u>	WL	LΜ	WR	M	TOTAL
4:00 PM	1	143	9	0	6	133	10	0	6	3	1	0	4	1	6	0	329
4:15 PM	က	159	က	-	7	169	80	0	9	٣	٣	0	9	٣	15	0	386
4:30 PM	2	130	4	0	11	128	7	0	4	٣	٣	0	2	4	14	0	312
4:45 PM	2	138	2	-	7	110	12	0	6	2	٣	0	٣		9	0	302
5:00 PM	0	168	1	0	6	145	111	1	10	4	2	0	2	3	12	0	371
5:15 PM		144	က	0	10	134	6	-	10	-1	2	0	9	0	18	0	339
5:30 PM	-	139	4	0	15	129	2	-	80	2	0	0	9		16	0	327
5:45 PM	1	134	2	0	12	140	2		8	٣	1	0	1	2	18	0	328
	N	TN	NR	N	SF	ST	SR	SU	П	Ш	ER	EU	ML	MT	WR	MU	TOTAL
TOTAL VOLUMES:	11	1155	28	2	80	1088	29	4	64	24	15	0	33	15	108	0	2694
APPROACH %'s:	0.92%	0.92% 96.57%	2.34%	0.17%	6.46%	87.81%	5.41%	0.32%	62.14%	23.30%	14.56%	0.00%	21.15%	9.62%	69.23%	0.00%	
PEAK HR :)	04:15 PM - 05:15 PM	05:15 PM														TOTAL
PEAK HR VOL:	7	595	13	2	34	552	38	П	29		11	0	16	11	47	0	1371
PEAK HR FACTOR:	0.583	0.885	0.650	0.500	0.773	0.817	0.792	0.250	0.725	0.750	0.917	0000	0.667	0.688	0.783	0000	
		0.913	13			0.849	61			8	608.0			0.771	71		0.888

PEAK HR		8:00 AM - 9:00 AM	9:00 AM														TOTAL
PEAK HR VOLUME	1	223	11	0	27	514	28	m	26	2	4	1	11	2	32	0	888
PEAK HR		4:45 PM -	4:45 PM - 5:45 PM														TOTAL
PEAK HR VOLUME	4	289	13	1	41	518	37	m	37	12	7	0	20	Ŋ	52	0	1339

National Data & Surveying Services Intersection Turning Movement Count

Location: Alton Rd & 4th St City: Miami Beach

Project ID: 21-140172-001

Control:	Signalized													Date: 8	3/5/2021		
	-							Data -	Bikes								
NS/EW Streets:		Alton	Dd			Alton				4th	C+			4th :	C+		1
NS/EW Sireets:																	
454		NORTH		_	_	SOUTH		_	_	EASTE		_	_	WESTB			
AM	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	TOTAL
7:00 AM	0 0	1	INK.	0	0 0	3	0 0	0	0	0	0 0	0	1 1	0	0	0	6 6
7:15 AM	0	Ô	ō	0	0	2	1	0	0	0	0	0	0	0	Ö	0	3
7:30 AM	Ö	Ö	Õ	Ö	ő	ō	ō	Ö	Ō	Ö	Ö	Ŏ	2	Ö	Ö	Ö	2
7:45 AM	0	2	0	0	0	5	0	0	0	0	0	0	0	0	0	0	7
8:00 AM	0	3	0	0	0	2	0	0	0	0	0	0	0	0	0	0	5
8:15 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
8:30 AM 8:45 AM	0	1 0	0 1	0	0	1 8	0	0 2	0	0	0	0	0	0	0	0	2 12
6:45 AM	U	U	1	U	U	0	U	2	U	U	U	U	1	U	U	U	12
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES :	0	8	2	0	0	22	1	2	0	0	0	0	4	0	0	0	39
APPROACH %'s:	0.00%	80.00%	20.00%	0.00%	0.00%	88.00%	4.00%	8.00%					100.00%	0.00%	0.00%	0.00%	
PEAK HR :		- MA 00:80															TOTAL
PEAK HR VOL : PEAK HR FACTOR :	0.000	5 0.417	1 0.250	0 0.000	0 0.000	12 0.375	0.000	2 0.250	0 0.000	0 0.000	0.000	0 0.000	1 0.250	0 0.000	0 0.000	0.000	21
PEAK HK FACTOR :	0.000	0.417		0.000	0.000	0.375		0.250	0.000	0.000	0.000	0.000	0.250	0.000		0.000	0.438
		0.5	00			0.5.	,							0.2.	,,,		
		NORTH	BOUND			SOUTH	BOUND			EASTE	BOUND			WESTE	OUND		
PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM 4:15 PM	0	0 1	2	0	0	1	0	0	0	0	0	0	2	0	0	0	5 2
4:15 PM 4:30 PM	0	3	0	0	0	1	0	0	0	0	0	0	4	0	0	0	8
4:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	2
5:00 PM	0	3	2	0	0	4	0	0	0	0	0	0	0	0	0	0	9
5:15 PM	0	1	1	0	2	2	0	0	0	0	0	0	0	0	0	0	6
5:30 PM	0	2	1	0	0	1	0	0	1	0	0	0	0	0	0	0	5
5:45 PM	0	1	0	0	0	4	0	0	0	0	0	0	2	0	0	0	7
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES :	0	11	6	0	2	15	0	0	1	0	0	0	9	0	0	0	44
APPROACH %'s:	0.00%	64.71%	35.29%	0.00%	11.76%	88.24%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	
PEAK HR :		04:15 PM -															TOTAL
PEAK HR VOL :	0	7	2	0	0	7	0	0	0	0	0	0	5	0	0	0	21
PEAK HR FACTOR :	0.000	0.583	0.250	0.000	0.000	0.438	0.000	0.000	0.000	0.000	0.000	0.000	0.313	0.000	0.000	0.000	0.583
		0.4	50			0.43	58						_	0.31	.3		
PEAK HR		8:00 AM -	9:00 AM														TOTAL
PEAK HR VOLUME	0	5	1	0	0	12	0	2	0	0	0	0	1	0	0	0	21
PEAK HR	_	4:45 PM -															TOTAL
PEAK HR VOLUME	0	6	4	0	2	8	0	0	1	0	0	0	1	0	0	0	22

National Data & Surveying Services Intersection Turning

Movement Count
Project ID: 21-140172-001
Date: 8/5/2021

Location: Alton Rd & 4th St **City:** Miami Beach

Data - Pedestrians (Crosswalks)

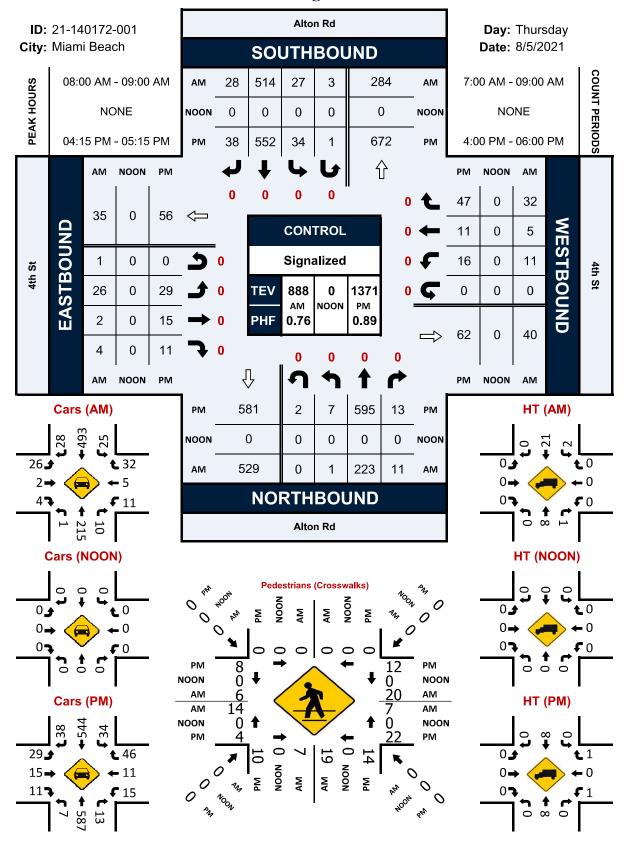
NS/EW Streets:	Altor	n Rd	Alto	n Rd	4th	n St	4th	St St	
AM	NORTI EB	H LEG WB	SOUT EB	H LEG WB	EAS ⁻ NB	Γ LEG SB	WES ⁻ NB	Γ LEG SB	TOTAL
7:00 AM		1	2	0	2	1	1	1	8
7:15 AM		1	1	1	0	3	3	1	11
7:30 AM	0	0	1	1	1	4	3	0	10
7:45 AM	1	0	3	7	3	6	3	2	25
8:00 AM	0	0	2	4	3	5	2	3	19
8:15 AM	0	0	2	6	2	6	6	2	24
8:30 AM	0	0	3	5	1	5	1	1	16
8:45 AM	0	0	0	4	1	4	5	0	14
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
TOTAL VOLUMES :	2	2	14	28	13	34	24	10	127
APPROACH %'s:	50.00%	50.00%	33.33%	66.67%	27.66%	72.34%	70.59%	29.41%	
PEAK HR:	- MA 00:80	09:00 AM							TOTAL
PEAK HR VOL:	0	0	7	19	7	20	14	6	73
PEAK HR FACTOR :			0.583	0.792	0.583	0.833	0.583	0.500	0.760
			3.0	313	0.0	344	0.6	525	0.700

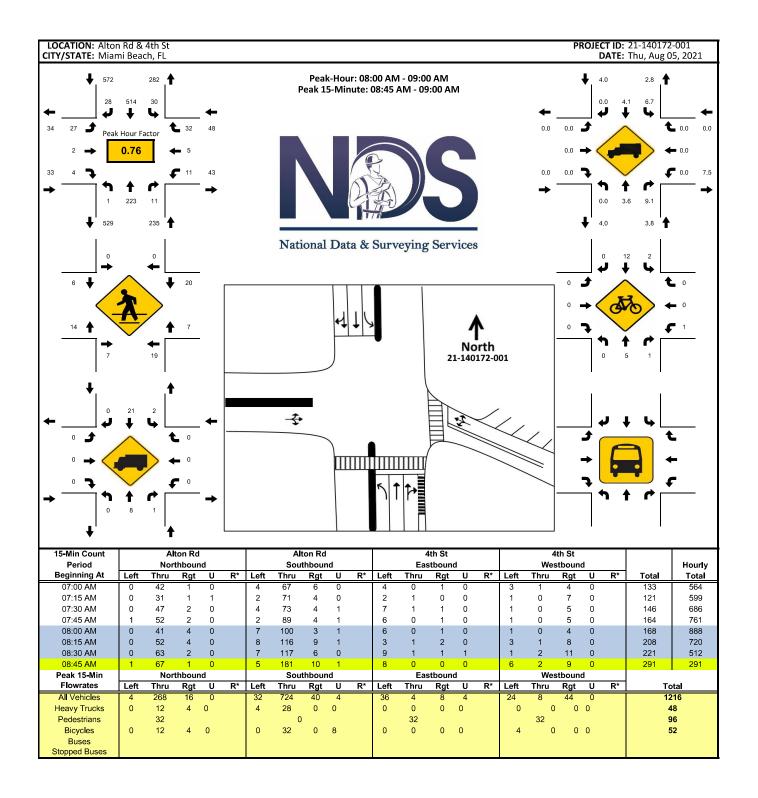
PM	NORT	'H LEG	SOUT	H LEG	EAS ⁻	T LEG	WES ⁻	T LEG	
PIVI	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
4:00 PM	0	0	3	3	2	0	0	1	9
4:15 PM	0	0	1	0	2	4	1	3	11
4:30 PM	0	0	5	3	6	2	2	1	19
4:45 PM	0	0	3	9	7	6	1	3	29
5:00 PM	0	0	1	2	7	0	0	1	11
5:15 PM	3	0	0	5	0	2	1	0	11
5:30 PM	0	0	4	3	6	0	0	1	14
5:45 PM	0	1	2	2	4	2	1	0	12
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
TOTAL VOLUMES:	3	1	19	27	34	16	6	10	116
APPROACH %'s:	75.00%	25.00%	41.30%	58.70%	68.00%	32.00%	37.50%	62.50%	
PEAK HR :	04:15 PM	- 05:15 PM							TOTAL
PEAK HR VOL :	0	0	10	14	22	12	4	8	70
PEAK HR FACTOR :			0.500	0.389	0.786	0.500	0.500	0.667	0.602
			0.5	500	0.0	654	0.7	750	0.603

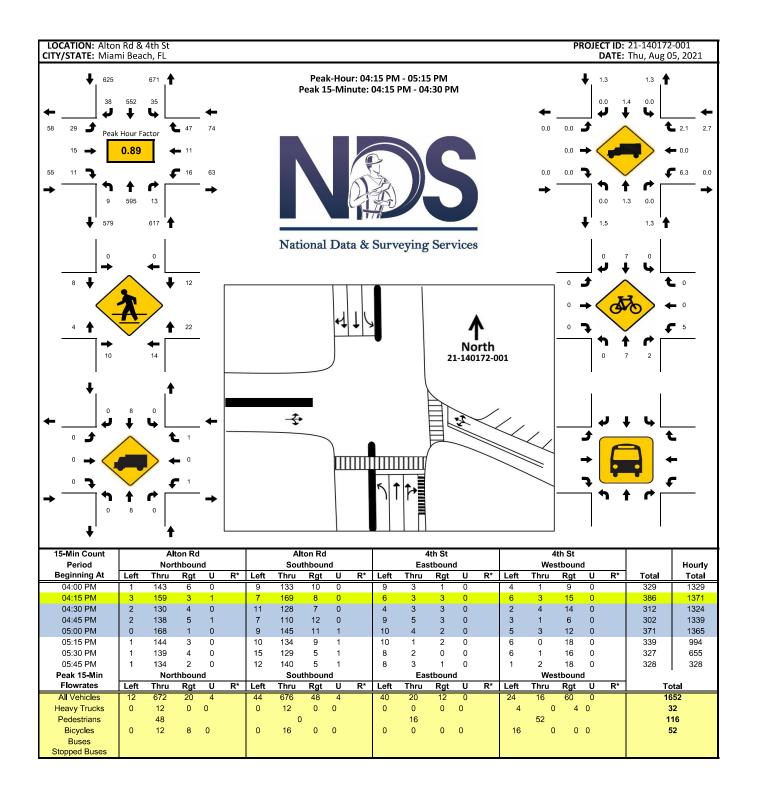
PEAK HR :	04:45 -	05:45 PM							TOTAL
PEAK HR VOL :	3	0	8	19	20	8	2	5	65

Alton Rd & 4th St

Peak Hour Turning Movement Count







National Data & Surveying Services Intersection Turning Movement Count

Location: Michigan Ave & 4th St City: Miami Beach Control: 4-Way Stop

Project ID: 21-140172-002 **Date:** 8/5/2021

Stop
4-Way §
trol: 4-

•								Data -	lotal								
NS/EW Streets:		Michigan Ave	א ר Ave			Michigan Ave	n Ave			4th St	St			4th St	St		
		NORTHBOUND	BOUND			SOUTHBOUND	BOUND			EASTBOUND	QNNC			WESTE	WESTBOUND		
ΣY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	ď	F	R	N	SF	ST	SR	SU	ᆸ	Ш	R	H	WL	LΜ	WR	MU	TOTAL
7:00 AM	3	3	0	0	1	2	0	0	0	2	1	0	0	4	0	0	16
7:15 AM	-	0	0	0		٣	0	0	1			0	0	4	٣	0	15
7:30 AM	1	7	0	0	0	4	0	0	0	4	2	0	0	4	0	0	22
7:45 AM	0	٣	0	0	-	4	0	0	0	4	2	0	0	2	٣	0	22
8:00 AM	3	7	1	0	0	9	0	1	2	3	3	0	0	4	0	0	30
8:15 AM	2	8	0	0	2	2	1	0	2	4	1	0	0	4	7	0	31
8:30 AM	1	10	0	0	1	2	0	0	4	9	٣	0	0	6	4	0	43
8:45 AM	2	9	-	0	2	7	-	0	1	4	3	0	2	∞	4	0	41
	٦N	LN	NR	N	TS	ST	SR	SN	Е	Ы	ER	EU	ML	MT	WR	MU	TOTAL
TOTAL VOLUMES:	13	39	2	0	∞	36	2	1	10	28	16	0	2	42	21	0	220
APPROACH %'s:	24.07%	24.07% 72.22%	3.70%	0.00%	17.02%	%09-92	4.26%	2.13%	18.52%	51.85%	29.63%	0.00%	3.08%	64.62%	32.31%	%00 ⁻ 0	
PEAK HR:)	08:00 AM - 09:00 AM	09:00 AM														TOTAL
PEAK HR VOL :	8	26	2	0	2	23	2	П	6	17	10	0	2	25	15	0	145
PEAK HR FACTOR:	0.667	0.650	0.500	0.000	0.625	0.821	0.500	0.250	0.563	0.708	0.833	0.000	0.250	0.694	0.536	0000	0 073
		0.818	8.			0.775	75			0.692	20			0.750	20) Fo.0
																	I

		NORTHBOUND	BOUND			SOUTHBOUND	BOUND			EASTBOUND	DNNC			WESTE	WESTBOUND		
≥	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	N	ħ	R) N	S	TS	SR	S	ᆸ	Ш	Ж	3	WL	LΜ	WR	NN	TOTAL
4:00 PM	3	12	0	0	1	6	2	0	2	8	2	0	1	7	2	0	22
4:15 PM	m	15	2	0	က	11		0	က	16	٣	0	-	12	4	0	74
4:30 PM	2	16	2	0	က	٣	2	0	4	12	1	0	2	12	m	0	65
4:45 PM	4	12	0	0	0	7	2	0		15	1	0	0	13	2	0	22
Md 00:5	2	10	1	0	3	7	1	0	3	10	3	0	1	16	3	0	09
5:15 PM	4	10	0	0	9	10	0	0	0	10	2	0	က	19	&	0	72
5:30 PM	m	2		0	-	00	2	0		11	4	0	0	19	9	0	64
5:45 PM	2	6	0	0	-	7	0	0	4	15	2	0	0	6	2	0	22
	٦N	TN	NR	NN	SF	ST	SR	SU	П	ET	ER	EU	ML	MT	WR	MN	TOTAL
TOTAL VOLUMES:	29	68	9	0	18	62	13	0	18	26	21	0	8	107	36	0	504
APPROACH %'s:	23.39%	71 77%	4.84%	0.00%	19 35%	%29 99	13.98%	0.00%	13.24%	71.32%	15.44%	0.00%	2.30%	%98.02	23.84%	0.00%	
PEAK HR :)	04:15 PM - 05:15 PM	05:15 PM														TOTAL
PEAK HR VOL:	14	53	2	0	6	28	9	0	11	23	œ	0	4	53	12	0	256
PEAK HR FACTOR:	0.700	0.828	0.625	0.000	0.750	0.636	0.750	0000	0.688	0.828	0.667	0000	0.500	0.828	0.750	0.000	1000
		0.783	33			0.7	17			0.81	œ			0.8	63		0.805

PEAK HR		8:00 AM -	9:00 AM														TOTAL
PEAK HR VOLUME	8	52	2	0	2	23	2	1	6	17	10	0	2	25	15	0	145
PEAK HR		4:45 PM - 5:45 PM	5:45 PM														TOTAL
PEAK HR VOLUME	13	37	2	0	10	32	œ	0	2	46	10	0	4	29	19	0	253

${\tt National\ Data\ \&\ Surveying\ Services} \\ Intersection\ Turning\ Movement\ Count$

Location: Michigan Ave & 4th St City: Miami Beach Control: 4-Way Stop

Project ID: 21-140172-002 Date: 8/5/2021

-								Data -	Bikes								_
NS/EW Streets:		Michiga	in Ave			Michiga	in Ave			4th	St			4th	St		ĺ
		NORTH	BOUND			SOUTH	BOUND			EASTE	BOUND			WESTE	BOUND		
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	i I
7	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	2	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
7:45 AM	0	1	0	0	0	3	0	0	0	0	1	0	0	0	0	0	5
8:00 AM	2	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3
8:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES :	2	4	0	0	0	6	0	0	0	0	2	0	0	2	0	0	16
APPROACH %'s:	33.33%	66.67%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	100.00%	0.00%	0.00%	
PEAK HR :	(- MA 00:80	09:00 AM														TOTAL
PEAK HR VOL :	2	1	0	0	0	2	0	0	0	0	1	0	0	0	0	0	6
PEAK HR FACTOR ·	0.250	0.250	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	

		NORTH	BOUND			SOUTH	BOUND			EASTE	OUND			WESTE	BOUND		
PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM	0	0	0	0	1	3	2	0	0	0	0	0	0	0	0	0	6
4:15 PM	0	2	0	0	0	1	1	0	0	2	0	0	0	2	0	0	8
4:30 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2
4:45 PM	0	2	0	0	0	1	3	0	0	0	2	0	0	1	0	0	9
5:00 PM	0	0	0	0	0	2	0	0	0	0	3	0	0	1	0	0	6
5:15 PM	0	1	1	0	0	3	1	0	0	0	0	0	0	0	0	0	6
5:30 PM	0	1	0	0	1	2	0	0	0	0	0	0	0	0	0	0	4
5:45 PM	0	3	0	0	1	3	0	0	0	0	0	0	0	1	1	0	9
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES:	0	9	1	0	3	17	7	0	0	2	5	0	0	5	1	0	50
APPROACH %'s:	0.00%	90.00%	10.00%	0.00%	11.11%	62.96%	25.93%	0.00%	0.00%	28.57%	71.43%	0.00%	0.00%	83.33%	16.67%	0.00%	
PEAK HR:		04:15 PM -	05:15 PM														TOTAL
PEAK HR VOL :	0	4	0	0	0	6	4	0	0	2	5	0	0	4	0	0	25
PEAK HR FACTOR :	0.000	0.500	0.000	0.000	0.000	0.750	0.333	0.000	0.000	0.250	0.417	0.000	0.000	0.500	0.000	0.000	0.694
		0.50	00			0.6	25			0.5	83			0.50	00		0.094

PEAK HR		4:45 PM	- 5:45 PM														TOTAL
PEAK HR VOLUME	0	4	1	0	1	8	4	0	0	0	5	0	0	2	0	0	25

National Data & Surveying Services Intersection Turning

Movement Count

Project ID: 21-140172-002
Date: 8/5/2021

Location: Michigan Ave & 4th St **City:** Miami Beach

Data - Pedestrians (Crosswalks)

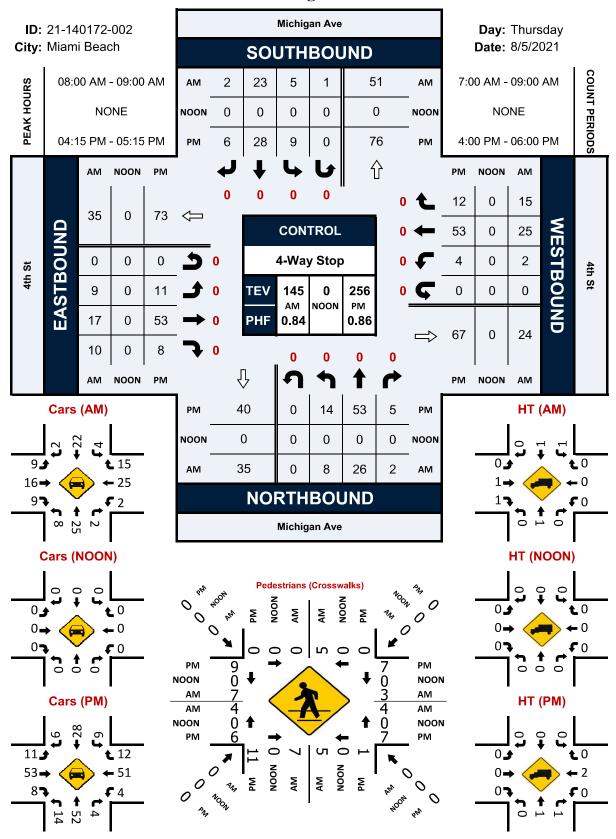
NS/EW Streets:	Michig	gan Ave	Michig	an Ave	4tl	n St	4th	St	
AM	NORT EB	TH LEG WB	SOUT EB	H LEG WB	EAS ⁻ NB	T LEG SB	WES ⁻ NB	Γ LEG SB	TOTAL
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM	0 0 0 0 0	0 0 0 1 2 0 1	2 1 1 0 1 1 1 2	0 0 0 2 3 0	1 0 0 2 1 1 2	1 0 0 0 0 0 2	0 0 0 1 1 0 0	1 2 2 2 2 2 3 1	5 3 3 8 10 7
8:45 AM	0	2	3	1	0	1	3	1	11
TOTAL VOLUMES : APPROACH %'s :	EB 0 0.00%	WB 6 100.00%	EB 11 61.11%	WB 7 38.89%	NB 7 63.64%	SB 4 36.36%	NB 5 26.32%	SB 14 73.68%	TOTAL 54
PEAK HR : PEAK HR VOL : PEAK HR FACTOR :	0	- 09:00 AM 5 0.625 625	7 0.583 0.7	5 0.417 750	4 0.500 0.1	3 0 . 375 583	4 0.333 0.6	7 0.583 588	TOTAL 35 0.795

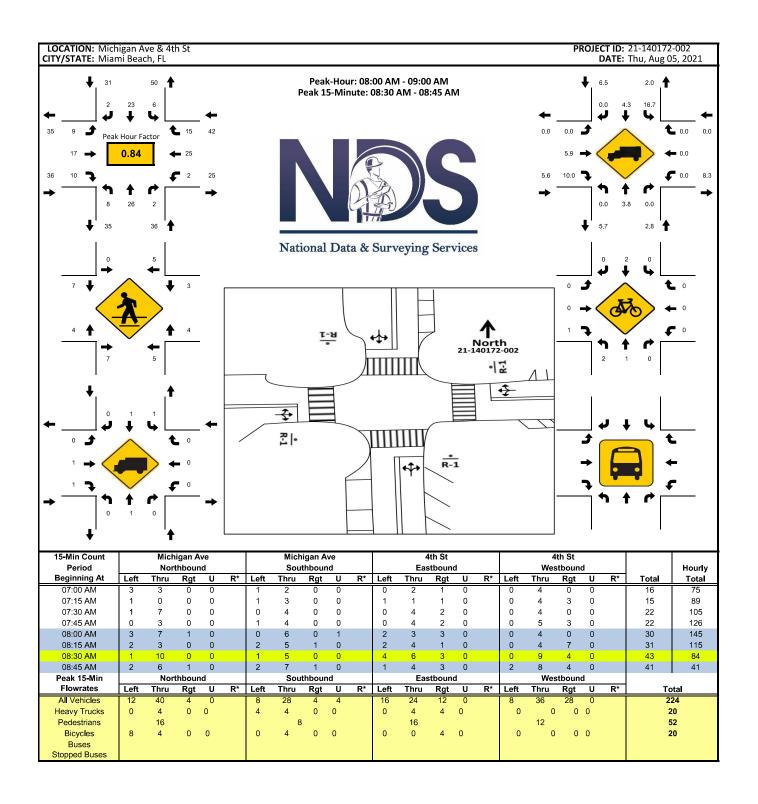
PM	NORT	'H LEG	SOUT	'H LEG	EAS ⁻	T LEG	WES	Γ LEG	
PIVI	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
4:00 PM	0	0	0	0	2	1	3	3	9
4:15 PM	0	0	2	1	1	3	1	2	10
4:30 PM	0	0	1	0	0	0	1	4	6
4:45 PM	0	0	5	0	3	4	4	3	19
5:00 PM	0	0	3	0	3	0	0	0	6
5:15 PM	1	2	0	1	1	0	1	1	7
5:30 PM	0	3	2	1	5	0	0	4	15
5:45 PM	0	0	0	2	2	1	2	2	9
	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
TOTAL VOLUMES:	1	5	13	5	17	9	12	19	81
APPROACH %'s:	16.67%	83.33%	72.22%	27.78%	65.38%	34.62%	38.71%	61.29%	
PEAK HR :	04:15 PM	- 05:15 PM						·	TOTAL
PEAK HR VOL :	0	0	11	1	7	7	6	9	41
PEAK HR FACTOR :			0.550	0.250	0.583	0.438	0.375	0.563	0.530
			0.6	500	0.	500	0.5	536	0.539

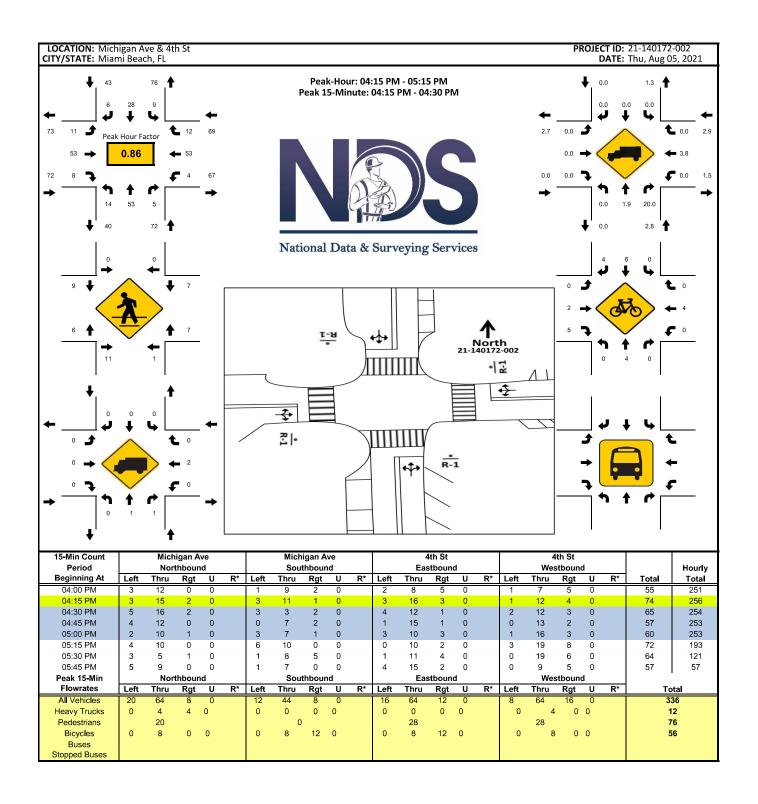
PEAK HR :	04:45 -	05:45 PM							TOTAL
PEAK HR VOL :	1	5	10	2	12	4	5	8	47

Michigan Ave & 4th St

Peak Hour Turning Movement Count







VOLUME

5th St Bet. Meridian Ave & Euclid Ave

Day: Thursday Date: 8/5/2021

City: Miami Beach
Project #: FL21_140173_001

	DAILY TOTALS			NB		SB		EB	WB							otal
	DAILT TOTALS			0		0		11,222	14,421						25,	,643
AM Period	NB SB	EB		WB		ТО	TAL	PM Period	NB	SB	EB		WB		ТО	TAL
00:00		93		134		227		12:00			190		182		372	
00:15 00:30		92 73		116 126		208 199		12:15 12:30			165 150		212 190		377 340	
00:45		80	338	100	476	180	814	12:45			163	668	213	797	376	1465
01:00		58		105		163		13:00			151		212		363	
01:15		68		101		169		13:15			174		197		371	
01:30		44	246	70	250	114	566	13:30			162	624	197	026	359	1460
01:45 02:00		46 26	216	74 67	350	120 93	566	13:45 14:00			147 176	634	220 245	826	367 421	1460
02:00		46		51		97		14:15			186		243		430	
02:30		25		34		59		14:30			190		244		434	
02:45		36	133	52	204	88	337	14:45			199	751	254	987	453	1738
03:00		24		44		68		15:00			164		218		382	
03:15		22		49		71		15:15			167		225		392	
03:30 03:45		25 26	97	38 49	180	63 75	277	15:30 15:45			178 190	699	216 204	863	394 394	1562
04:00		32	97	62	100	94	2//	16:00			150	099	210	803	360	1302
04:15		37		65		102		16:15			154		247		401	
04:30		31		71		102		16:30			154		224		378	
04:45		31	131	48	246	79	377	16:45			185	643	214	895	399	1538
05:00		26		58		84		17:00			161		222		383	
05:15		20		37		57		17:15			165		240		405	
05:30 05:45		33 70	149	38 49	182	71 119	331	17:30 17:45			146 147	619	211 206	879	357 353	1498
06:00		50	143	60	102	110	331	18:00			163	013	218	675	381	1436
06:15		57		62		119		18:15			127		250		377	
06:30		75		46		121		18:30			130		226		356	
06:45		84	266	45	213	129	479	18:45			176	596	178	872	354	1468
07:00		116		109		225		19:00			139		210		349	
07:15 07:30		119 101		70 101		189 202		19:15 19:30			120 152		229 209		349 361	
07:45		137	473	95	375	232	848	19:45			135	546	183	831	318	1377
08:00		143	.,,	86	0,0	229	0.0	20:00			123	0.0	185	001	308	2077
08:15		136		141		277		20:15			147		192		339	
08:30		153		108		261		20:30			155		231		386	
08:45		181	613	122	457	303	1070	20:45			109	534	182	790	291	1324
09:00 09:15		145 143		146 133		291 276		21:00 21:15			120 140		192 160		312 300	
09:30		160		167		327		21:30			117		158		275	
09:45		146	594	169	615	315	1209	21:45			102	479	162	672	264	1151
10:00		150		163		313		22:00			125		171		296	
10:15		165		180		345		22:15			146		165		311	
10:30		141	62.4	189	747	330	40.44	22:30			88	47.4	136	643	224	1006
10:45 11:00		168 180	624	185 203	717	353 383	1341	22:45 23:00			115 102	474	140 169	612	255 271	1086
11:00		145		183		328		23:15			56		157		213	
11:30		141		205		346		23:30			77		136		213	
11:45		155	621	197	788	352	1409	23:45			89	324	132	594	221	918
TOTALS			4255		4803		9058	TOTALS				6967		9618		16585
SPLIT %			47.0%		53.0%		35.3%	SPLIT %				42.0%		58.0%		64.7%
				NB		SB		ЕВ	WB						I	otal
	DAILY TOTALS			0		0		11,222	14,421							,643
AM Peak Hour			11:45		11:30		11:30	PM Peak Hour				14:00		14:00		14:00
AM Pk Volume			660		796		1447	PM Pk Volume				751		987		1738
Pk Hr Factor			0.868		0.939		0.960	Pk Hr Factor				0.943		0.971		0.959
7 - 9 Volume	0 0		1086		832		1918	4 - 6 Volume	. 0	0		1262		1774		3036
7 - 9 Peak Hour			08:00		08:00		08:00	4 - 6 Peak Hour				16:30		16:15		16:30
7 - 9 Pk Volume			613		457		1070	4 - 6 Pk Volume				665		907		1565
Pk Hr Factor	0.000 0.000		0.847		0.810		0.883	Pk Hr Factor	0.000	0.00	0	0.899		0.918		0.966

County: 87 Station: 2528

Description: SR A1A/MACARTHUR CSWY, 150' N OF MERIDIAN AVE

Start Date: 03/10/2020 Start Time: 0000

24-Hour Totals: 19426 22260 41686

Dools Wolyma Information

	Directi	on: E	Combined	Directions			
F	Iour	V <u>olume</u>	Hour	<u>Volum</u> e	Hour	Volume	
A.M.	830	1192	800	915	830	2059	
P.M.	1515	1246	1615	1566	1515	2745	
Daily	1515	1246	1615	1566	1515	2745	

Generated by SPS 5.0.53P

County: 87 Station: 2528

Description: SR A1A/MACARTHUR CSWY, 150' N OF MERIDIAN AVE

Start Date: 03/11/2020 Start Time: 0000

24-Hour Totals: 19988 23565 43553

Peak Volume Information

		,						
Directi	on: E	Direction	n: W	Combined	Combined Directions			
Iour	Volume	Hour	Volume	Hour	Volume			
830	1173	800	858	830	2017			
1530	1280	1500	1485	1515	2734			
1530	1280	1500	1485	1515	2734			
	Hour 830 1530	830 1173 1530 1280	Hour Volume Hour 830 1173 800 1530 1280 1500	Hour Volume 830 1173 1530 1280 1500 1485	Hour Volume Hour Volume Hour 830 1173 800 858 830 1530 1280 1500 1485 1515			

Generated by SPS 5.0.53P

County: Station:

Description: SR A1A/MACARTHUR CSWY, 150' N OF MERIDIAN AVE

Start Date: 03/12/2020 Start Time: 0000

24-Hour Totals: 23269 43199 _____

Peak Volume Information Direction: E Direction: W

Combined Directions Hour Hour Hour Volume Volume A.M. P.M. Daily

Volume

Print Date: 4/8/2020

Print T	ime:
2:25	AM

<u>Asset</u>		Intersection			TOD_ Schedule	Op Mode	<u>Plan</u>		Cyc		<u>Offset</u>	TOD Setting	<u>Active</u> <u>PhaseBank</u>	
7699		Alton Rd&4	St	D	OW-4	TOD		N/A	0	(0	N/A	0	Max 0
			<u>s</u>	<u>plits</u>										
<u>PH 1</u>	<u>PH 2</u>	<u>PH 3</u>	<u>PH 4</u>	<u>PH 5</u>	<u>PH 6</u>	<u>PH 7</u>	<u>PH 8</u>							
SEL	SET	XPD	EBT	NWL	NWT	-	NBT							
0	0	0	0	0	0	0	0							
>	1	N/A	\rightarrow	4	lack		lack							

se Bank:	<u>Pha</u>	se Bank 1								
<u>Walk</u>		Don't Walk	Mir	<u>Initial</u>	<u>v</u>	<u>eh Ext</u>	Max Limit	<u>Max 2</u>	<u>Yellow</u>	<u>Red</u>
Phase Ba										
1 2	3	1 2 3	1	2 3	1	2 3	1 2 3	1 2 3		
0 - 0	- 0	0 - 0 - 0	5	- 5 - 5	3	- 3 - 3	9 - 9 - 9	18 - 18 - 18	4	3.4
6 - 0	- 7	19 - 0 - 19	7	- 7 - 7	1	- 1 - 1	35 - 49 - 49	0 - 0 - 0	4	3.4
0 - 0	- 0	0 - 0 - 0	0	- 0 - 0	0	-0-0	0 - 0 - 0	0 - 0 - 0	0	0
0 - 0	- 0	0 - 0 - 0	7	- 7 - 7	2.5	-2.5 - 2.5	30 - 49 - 49	65 - 65 - 65	4	2.6
0 - 0	- 0	0 - 0 - 0	5	- 5 - 5	3	- 3 - 3	9 - 9 - 9	18 - 18 - 18	4	3.4
6 - 0	- 7	19 - 0 - 19	7	- 7 - 7	1	- 1 - 1	35 - 49 - 49	0 - 0 - 0	4	3.4
0 - 0	- 0	0 - 0 - 0	0	- 0 - 0	0	-0-0	0 - 0 - 0	0 - 0 - 0	0	0
6 - 0	- 7	20 - 0 - 20	7	- 7 - 7	2.5	-2.5 - 2.5	30 - 49 - 49	65 - 65 - 65	4	2.6
	Walk Phase Ba 1	Walk Phase Bank 1 2 3 0 - 0 - 0 6 - 0 - 7 0 - 0 - 0 0 - 0 - 0 0 - 0 - 0 0 - 0 - 0	Walk Phase Bank Don't Walk 1 2 3 1 2 3 0 - 0 0 0 0 0 0 0 6 - 0 7 19 0 - 19 0 - 0 0 0 0 0 0 0 0 - 0 0 0 0 0 0 0 0 0 - 0 0 0 0 0 0 0 19 0 - 0<	Walk Phase Bank Don't Walk Mir 1 2 3 1 2 3 1 0 - 0 0 0 0 0 5 6 - 0 7 19 0 19 7 0 - 0 0 0 0 0 0 0 0 - 0 0 0 0 0 7 7 0 - 0 0 0 0 0 0 5 6 - 0 7 19 0 19 7 0 - 0 0 0 0 0 0 0	Walk Phase Bank Don't Walk Min Initial 1 2 3 1 2 3 0 - 0 0 0 0 0 5 5 5 5 6 - 0 - 7 19 0 - 19 7 - 7 - 7 0 - 0	Walk Phase Bank Don't Walk Min Initial Very Number of Section 1 1 2 3 1 2 3 1 2 3 1 0 - 0 - 0 - 0 - 0 5 - 5 - 5 3 6 - 0 - 7 - 7 - 7 - 7 1 0 - 0 - 0 - 0 0 - 0	Walk Phase Bank Don't Walk Min Initial Veh Ext 1 2 3 1 2 3 1 2 3 1 2 3 0 - 0 0 0 0 0 0 5 5 5 5 3 -3 3 6 - 0 7 7 7 7 7 1 -1 1 1 1 1 1 2 3 2 3 2 3 2 3 2 3 2 3 2	Walk Phase Bank Don't Walk Min Initial Veh Ext Max Limit 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 3 9	Walk Phase Bank Don't Walk Min Initial Veh Ext Max Limit Max 2 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 3 9 9 9 18	Walk Phase Bank Don't Walk Min Initial Veh Ext Max Limit Max 2 Yellow 1 2 3 18 18 18

Permitted Phases <u>12345678</u> 123456-8 Default External Permit 0 123456-8 123456-8 External Permit 1 External Permit 2 123456-8

unknown

Last In Service Date:

						Green 7	<u> </u>					
<u>Current</u>	Diam	Cuala	1	2	3	4	5	6	7	8	Din a Officet	Off4
TOD Schedule	<u>Plan</u>	<u>Cycle</u>	SEL	SET	XPD	EBT	NWL	NWT	-	NBT	Ring Offset	<u>Offset</u>
	1	85	7	29	0	28	7	29	0	28	0	84
	2	150	14	61	0	54	14	61	0	54	0	0
	3	100	10	41	0	28	10	41	0	28	0	0
	5	150	17	72	0	40	17	72	0	40	0	0
	6	180	17	88	0	54	17	88	0	54	0	0
	7	85	7	29	0	28	7	29	0	28	0	40
	8	100	10	41	0	28	10	41	0	28	0	0
	9	90	8	33	0	28	8	33	0	28	0	0
	10	80	5	27	0	27	5	27	0	27	0	77
	22	100	10	41	0	28	10	41	0	28	0	0
	25	140	13	73	0	33	13	73	0	33	0	127
	26	100	10	41	0	28	10	41	0	28	0	79

Local T	OD Schedule		
<u>Time</u>	<u>Plan</u>	<u>DOW</u>	
0000	Free	Su M T W Th F	S
0500	2	M T W Th F	
0500	Free	Su	S
0800	6	M T W Th F	
0850	9	M T W Th F	
1000	10	Su	S
1130	5	M T W Th F	
1300	6	M T W Th F	
1340	9	M T W Th F	
1540	6	M T W Th F	
1615	25	M T W Th F	
1800	1	MTWThF	
1800	7	Su	S
2200	Free	Su M T W Th F	S

Currer	nt Time of Day Function		
<u>Time</u>	<u>Function</u>	Settings *	Day of Week
0000	TOD OUTPUTS	-7	SuM T W ThF S
0500	PED RECALL	8-6-432-	SuM T W ThF S
0700	TOD OUTPUTS	1	M T W ThF
0850	TOD OUTPUTS	-7	M T W ThF
1340	TOD OUTPUTS	1	M T W ThF
1540	TOD OUTPUTS	-7	M T W ThF
2200	PED RECALL		SuM T W ThF S

Local	Time of Day Function		
<u>Time</u>	<u>Function</u>	Settings *	Day of Week
0000	TOD OUTPUTS	-7	SuM T W ThF S
0500	PED RECALL	8-6-432-	SuM T W ThF S
0700	TOD OUTPUTS	1	M T W ThF
0850	TOD OUTPUTS	-7	M T W ThF
1340	TOD OUTPUTS	1	M T W ThF
1540	TOD OUTPUTS	-7	M T W ThF
2200	PED RECALL		SuM T W ThF S

* Settings
Blank - FREE - Phase Bank 1, Max 1
Blank - Plan - Phase Bank 1, Max 2
1 - Phase Bank 2, Max 1
2 - Phase Bank 2, Max 2
3 - Phase Bank 3, Max 1
4 - Phase Bank 3, Max 2
5 - EXTERNAL PERMIT 1
6 - EXTERNAL PERMIT 2
7 - X-PED OMIT
8 - TBA

No Calendar Defined/Enabled

TOD Schedule Report

Print Date: for 2752: Michigan Av&5 St Print Time: 5/8/2020 2:07 AM

<u>Asset</u> 2752		Intersection			TOD Schedule OW-6	Op Mode	<u>Plan #</u> [03] AM PEAK	<u>Cycle</u> 120	<u>Offset</u> 104	TOD Setting	Active PhaseBank	Active Maximum Max 2
			<u> </u>	Splits_								
<u>PH 1</u>	<u>PH 2</u>	<u>PH 3</u>	<u>PH 4</u>	<u>PH 5</u>	<u>PH 6</u>	<u>PH 7</u>	<u>PH 8</u>					
EBL	WBT	-	NBT	WBL	EBT	=	SBT					
12	36	0	54	6	42	0	54					
♪	←		1	F	\rightarrow	•	\					

Active Phase	Bank: P	hase Bank 1								
<u>Phase</u>	<u>Walk</u>	<u>Don't Walk</u>	Min Ir	<u>nitial</u>	<u>V</u>	eh Ext	Max Limit	<u>Max 2</u>	<u>Yellow</u>	<u>Red</u>
	Phase Bank									
	1 2 3	1 2 3	1 2	2 3	1	2 3	1 2 3	1 2 3		
1 EBL	0 - 0 -	0 0 - 0 - 0	5 -	5 - 5	2	- 2 - 2	12 - 5 - 5	30 - 40 - 25	4	2
2 WBT	7 - 7 -	7 12 - 12 - 12	7 -	7 - 7	1	- 1 - 1	30 - 30 - 30	0 - 30 - 30	4	2
3 -	0 - 0 -	0 - 0 - 0	0 -	0 - 0	0	-0-0	0 - 0 - 0	0 - 0 - 0	0	0
4 NBT	7 - 7 -	7 29 - 29 - 29	7 -	7 - 7	2.5	-2.5 - 2.5	8 - 8 - 12	60 - 20 - 18	4	2.5
5 WBL	0 - 0 -	0 - 0 - 0	5 -	5 - 5	2	- 2 - 2	5 - 5 - 5	16 - 12 - 12	4	2.5
6 EBT	7 - 7 -	7 12 - 12 - 12	7 -	7 - 7	1	- 1 - 1	30 - 30 - 30	0 - 30 - 30	4	2
7 -	0 - 0 -	0 - 0 - 0	0 -	0 - 0	0	-0-0	0 - 0 - 0	0 - 0 - 0	0	0
8 SBT	7 - 7 -	7 29 - 29 - 29	7 -	7 - 7	2.5	-2.5 - 2.5	8 - 8 - 12	60 - 20 - 18	4	2.5

Permitted Phases	
	<u>12345678</u>
Default	12-456-8
External Permit 0	12-456-8
External Permit 1	12-456-8
External Permit 2	12-456-8

unknown

Last In Service Date:

						Green 7	<u> Time</u>					
<u>Current</u>			1	2	3	4	5	6	7	8		
TOD Schedule	<u>Plan</u>	<u>Cycle</u>	EBL	WBT	-	NBT	WBL	EBT	-	SBT	Ring Offset	<u>Offset</u>
	1	170	17	86	0	49	9	94	0	49	0	62
	2	150	12	66	0	54	6	72	0	54	0	39
	3	120	12	36	0	54	6	42	0	54	0	104
	4	150	12	66	0	54	6	72	0	54	0	76
	5	150	12	66	0	54	6	72	0	54	0	49
	6	180	12	96	0	54	6	102	0	54	0	128
	7	170	12	86	0	54	6	92	0	54	0	167
	8	160	12	76	0	54	6	82	0	54	0	58
	10	160	13	93	0	36	13	93	0	36	0	58
	14	120	12	36	0	54	6	42	0	54	0	9
	15	130	20	43	0	49	16	47	0	49	0	4
	16	120	12	36	0	54	6	42	0	54	0	7
	22	110	12	26	0	54	6	32	0	54	0	14
	23	110	12	26	0	54	6	32	0	54	0	109
	25	140	12	56	0	54	6	62	0	54	0	54
	26	200	12	116	0	54	6	122	0	54	0	29
	27	140	12	56	0	54	6	62	0	54	0	18
	28	220	35	112	n	5/	6	1/12	<u> </u>	5/	Λ	54

Local TOI) Schedule		
<u>Time</u>	<u>Plan</u>	<u>DOW</u>	
0000	3	Su M T W Th	n F S
0500	2	M T W Th	ı F
0500	3	Su	S
0800	6	M T W Th	۱ F
1000	5	M T W Th	ı F
1000	10	Su	S
1300	6	M T W Th	١F
1615	25	M T W Th	ı F
1800	1	M T W Th	۱ F
1800	7	Su	S
2000	4	Su	S
2200	8	M T W Th	ı F

Currer	nt Time of Day Function		
<u>Time</u>	<u>Function</u>	Settings *	Day of Week
0000	TOD OUTPUTS	8	SuM T W ThF S
0000	TOD LOCAL MULTIFU	4	SuM T W ThF S
0500	TOD LOCAL MULTIFU		SuM T W ThF S
0600	TOD OUTPUTS		M T W ThF
1800	PED RECALL	84	M T W ThF
1800	TOD OUTPUTS	8	M T W ThF
2200	PED RECALL		M T W ThF

<u>Time</u>	Function	Settings *	Day of Wee	.le
<u>1111116</u>	<u>runction</u>	Settings	Day of wee	<u> </u>
0000	TOD OUTPUTS	8	SuM T W Thi	- S
0000	TOD LOCAL MULTIFUNC	T4	SuM T W Thi	= S
0500	TOD LOCAL MULTIFUNC	T	SuM T W Thi	₹S
0600	TOD OUTPUTS		M T W Th	F
0700	TOD OUTPUTS	1	Su	S
0800	TOD OUTPUTS			S
1000	TOD OUTPUTS		Su	
1000	PED RECALL	84	Su	S
1800	PED RECALL		Su	S
1800	PED RECALL	84	M T W Th	F
1800	TOD OUTPUTS	8	M T W Th	F
1900	TOD OUTPUTS	8	Su	S
2200	PED RECALL		M T W Th	F

* Settings
Blank - FREE - Phase Bank 1, Max 1
Blank - Plan - Phase Bank 1, Max 2
1 - Phase Bank 2, Max 1
2 - Phase Bank 2, Max 2
3 - Phase Bank 3, Max 1
4 - Phase Bank 3, Max 2
5 - EXTERNAL PERMIT 1
6 - EXTERNAL PERMIT 2
7 - X-PED OMIT
8 - TBA

Local Time of Day Function

TOD Schedule Report for 2752: Michigan Av&5 St

Print Date:	for 2/52: Michigan Av&5 St	Print Time:
5/8/2020		2:07 AM

No Calendar Defined/Enabled					