Exhibit G

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July 28, 2021

Mr. Firat Akcay, M.S.C.E. MBA Transportation Engineer Miami Beach Transportation and Mobility Department 1688 Meridian Avenue, Suite 801 Miami Beach, FL 33139 (305) 673-7000 FiratAkcay@miamibeachfl.gov

RE: Ritz-Carlton Sagamore Miami Beach Trip Generation and Queuing Analysis - #21111

Dear Firat,

The Ritz-Carlton Sagamore development is located at 1 Lincoln Road in Miami Beach, Florida. (See Attachment A for the site plan). The Ritz-Carlton Sagamore Miami Beach development is proposing an addition to the Sagamore Hotel that will replace 93 rooms within the hotel with a mixed-use tower consisting of 52 multifamily dwelling units and 53 hotel rooms. The traffic caused by the addition will utilize the same parking garage and optional valet drop-off / pick-up service area as the existing site. As such, access to the site will continue to be provided via the existing parking garage and valet area located on Lincoln Road, east of SR-A1A. Short and long-term bicycle parking will be available on the first floor of the parking garage, east of the garage entrance.

The purpose of this traffic statement is to conduct a trip generation analysis and queuing analysis for the proposed residential / hotel tower addition. A trip generation analysis was performed to estimate the trips during the AM and PM peak hour of the adjacent street. A queueing analysis was performed at the valet station to analyze if the queues will exceed the provided valet storage. Trip generation was also performed to establish the critical morning or afternoon peak volumes to use for the queuing analysis.



Trip Generation

A trip generation analysis was conducted for the proposed Sagamore Miami Beach project. The project trip generation was based on the rates/equations published by the Institute of Transportation Engineers (ITE) <u>Trip Generation Manual</u>, 10th Edition. Land Use 221, Multifamily Housing Mid-Rise and Land Use 310, Hotel were used in the analysis. A 20% reduction for other modes of transportation was applied at the request of the City. Trip generation calculations were performed for a typical weekday daily, AM and PM peak hours of the adjacent street. Trip generation for the proposed and existing site are summarized in Exhibit 1. Support documentation is provided in Attachment B.

Proposed ITE Land Use	Number Daily Of Units Vehicle		AM Peak Hour Vehicle Trips			PM Peak Hour Vehicle Trips		
Designation	or Units	Trips	In	Out	Total	In	Out	Total
Hotel Land Use Code: 310	53 Rooms	172	12	9	21	7	7	14
Multifamily Housing (Mid-Rise) Land Use Code: 221	52 DU	282	5	13	18	14	9	23
Total Gross Trips		454	17	22	39	21	16	37
Other Modes of Transportation ²		20.0%	-3	-4	-7	-5	-3	-8
Net Proposed Trips		14	18	32	16	13	29	

Exhibit 1 Trip Generation

Existing ITE Land Use	Number	Number of Units Vehicle		AM Peak Hour Vehicle Trips			PM Peak Hour Vehicle Trips		
Designation	of Units Trips	In	Out	Total	In	Out	Total		
Hotel Land Use Code: 310	93 Rooms	624	24	17	41	22	21	43	
Total Gross Trips		624	24	17	41	22	21	43	
Other Modes of Transportat	20.0%	-5	-3	-8	-4	-4	-8		
Net Existing Trips			13	10	23	12	11	23	

	Daily Vehicle	AM Ve	Peak H hicle Tr	lour ips	PM Ve	Peak H hicle Tr	lour ips
	TTPS	In	Out	Total	In	Out	Total
Proposed	454	14	18	32	16	13	29
Existing	624	13	10	23	12	11	23
Net Trip Difference	-170	1	8	9	4	2	6

¹ Based on ITE Trip Generation Manual, 10th Edition.

 2 A 20% reduction was used for the transportation reduction per City request.



The results of the analysis show that the proposed Ritz-Carlton Sagamore tower will generate 170 less daily trips than the existing hotel and a total of 9 and 6 vehicle trips during morning and afternoon peak hours respectively.

Queuing Analysis

As previously stated, the proposed tower will utilize the existing parking garage and valet station. The existing valet station is located on Lincoln Road at the existing Ritz-Carlton entrance under a porte-cochere. A queuing analysis was performed for the valet station to determine if a queue will form at the proposed valet reception area that will spill back onto Lincoln Road and interfere with the internal circulation of vehicles entering and exiting the parking garage.

The queuing analysis for the proposed valet drop-off / pick-up area was performed based on the methodology outlined in the *Institute of Transportation Engineers (ITE) Transportation and Land Development*. The analysis was performed to determine the number of valet parking attendants required during the peak hour so that the queue does not extend past the valet storage area (95% confidence level analysis). The potential queues were calculated based on the peak hour traffic published by the Institute of Transportation Engineers (ITE) trip generation rates and/or equations. As the valet station currently serves and will continue to serve the traffic from the Ritz-Carlton Hotel and the Sagamore Hotel, trip generation for Ritz-Carlton Hotel and the Sagamore Hotel's proposed development plan was performed to determine the demand at the valet station. The PM peak of generator (worst case scenario) was used for the purpose of calculating the expected queues at the valet station. A 20% reduction for other modes of transportation and a 44% rideshare reduction were applied at the request of the City. As guests have the option to self-park or valet, it was assumed that 80% of the guests would use the valet services and 20% would self-park. The proposed AM and PM peak of generator trip generation is summarized in Exhibit 2. Queuing trip generation documentation is available in Attachment C.





Exhibit 2

ITE Land Use Designation ¹	Number	AM Peak of Generator Vehicle Trips			PM Peak of Generator Vehicle Trips		
TTE Lana oro 2 organite	of Units	In	Out	Total	In	Out	Total
Sagamore Hotel	53 Rooms	19	17	36	20	15	35
Sagamore Multifamily Housing Land Use Code: 221	52 DU	5	15	20	15	10	25
Existing Ritz-Carlton Hotel Land Use Code: 310	374 Rooms	101	86	187	125	90	215
Total Gross Trips		125	118	243	160	115	275
Other Modes of Transportation ²	20%	-25	-24	-49	-32	-23	-55
Rideshare Services ³	44%	-44	-41	-85	-56	-40	-96
Net Proposed Trips		56	53	109	72	52	124
Demand at Valet Station	80%	45	42	87	58	42	100

Valet Trip Generation

Based on ITE Trip Generation Manual, 10th Edition.

² A 20% reduction was used for the transportation reduction per City request.

³ A 44% rideshare reduction was used at the valet station per City request.

The results of the trip generation show that the critical peak hour for valet parking is the PM peak hour of the generator with a total of 100 vehicle trips (in/out).

The queuing analysis used the single-channel waiting line model with Poisson arrivals and exponential service times. The analysis is based on the coefficient of utilization (ρ) which is the ratio of the average arrival rate of vehicles to the average service rate.

 $\rho = \frac{\textit{Average Demand Rate}}{\textit{Average Sevice Rate}}$

The average service rate corresponds to the time it will take a valet parking attendant to park or retrieve a vehicle. If the coefficient of utilization is greater than 1, then the calculation will yield an infinite queue length.

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The required queue storage (M) is determined using the following equation:

$$M = \left[\frac{\ln P(x > M) - \ln Q_M}{\ln \rho}\right] - 1$$

In this equation, P(x > M) is set at 5% to yield a 95% confidence that the queue will not back-up onto the adjacent street.

Since the distance from the valet drop-off / pick-up area differs for inbound and outbound trips, a weighted average was taken of the inbound / outbound valet processing time. The weighted average was based on the inbound / outbound trip distribution, which is 58% inbound and 42% outbound.

The processing rates were calculated by adding the time it will take a valet attendant to process the vehicles (**processing time**), the time it will take the attendant to circulate to the parking space (**driving time**), the time it will take the attendant to park or retrieve a vehicle (**park processing time**), and the time it will take the attendant to walk to/from the parking area (**walking time**).

A processing time of 60 seconds per vehicle was used in the analysis. This information was provided by the City of Miami Beach. The driving time for the valet attendant was calculated on a conservative speed of 15 mph, and the walking time for the valet attendant was calculated on a jogging speed of 5 ft / sec (provided by the City). The valet processing rate for the valet station can be seen in Exhibit 3.

Exhibit 3 Valet Station Processing Rate Valet Drop-off / Pick-up

Valet Time (Inbound) Processing time: Driving time: Park Processing Time: Walking time: Total

 $60 \sec / 60 \sec / 1 \min = 1.00 \min$ $780 \text{ ft} * 1 \min / 5280 \text{ ft} * 1 \ln / 15 \text{ miles} * 60 \min / \ln = 0.59 \min$ $= 0.15 \min$ $480 \text{ ft} / 5 \text{ ft} / \sec / 60 \sec / \min = 1.60 \min$ $= 3.34 \min$

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Valet Time (Outbound)

Processing time:	$60 \sec / 60 \sec / 1 \min = 1.00 \min$
Driving time:	510 ft * 1 mile / 5280 ft * 1hr / 15 miles * 60 min / hr = 0.39 min
Park Processing Time:	= 0.15 min
Walking time:	480 ft / 5 ft / sec / 60 sec / min = 1.60 min
Total	= <u>3.14 min</u>

Weighted Valet Time	
58% Inbound:	0.58*3.34 min = <i>1.94 min</i>
58% Outbound:	0.42*3.14 min = <i>1.32 min</i>
Total	= <u>3.26 min</u>

An iterative approach was used to determine the minimum number of valet attendants required during the PM peak hour to serve both the entering and exiting vehicles that will ensure that the average queue at the valet station will not extend past the valet storage. Exhibit 4 shows the calculations for the inbound / outbound valet drop-off /pick-up area during the PM peak hour of the generator.

Exhibit 4 Valet Station Queuing Calculations

 $Q = \text{Processing Rate} = \frac{60 \text{ min/hr}}{3.26 \text{ min/process}} = 18.43 \text{ process/hr}$ $q = \text{Demand Rate} = 100 \frac{\text{veh}}{\text{hr}}$ N = Service Positions = 9 Attendants $\rho = \text{Utilization factor} = \frac{q}{(NQ)} = \frac{100 \text{ veh/hr}}{9 \times 18.43 \text{ process/hr}} = 0.6028$ $Q_m = \text{Table Value} = 0.1239$ M = queue length which is exceeded 5% of the time [P(x>M)] $M = \frac{\ln P(x>M) - \ln(Q_m)}{\ln(\rho)} - 1 = \frac{\ln(0.05) - \ln(0.1239)}{\ln(0.6028)} - 1 = 0.79, \text{ say 1 Vehicle on queue}$

The results of the analysis show that a total of 9 valet attendants would be able to handle the demand during the PM peak of generator at the valet station with an average queue of approximately one vehicle or less. Based on the site plan, the valet station has approximately 70 feet of storage; this distance is enough to accommodate three vehicles in the queue. It should be



noted that the queuing analysis considers the worst case scenario during the peak hours to make sure that the queue never spills onto the public right-of-way or interferes with site operations. Once operational, the development can assess the actual need for valet attendants at different times of the day.

Conclusion

The Ritz-Carlton Sagamore project is proposing an addition that will replace 93 rooms of the existing Sagamore Hotel with a mixed-use tower consisting of 52 residential units and 53 hotel rooms. The tower is anticipated to generate 170 less daily trips than the existing site and only adds a total of 9 and 6 vehicle trips during morning and afternoon peak hours respectively. As the addition is adding less than 10 trips to the roadway network during peak hours, the additional vehicle trips and impacts on the adjacent roadway network can be consider *de minimis*.

As discussed above, the new tower will utilize the parking within the existing parking garage and the valet station provided on Lincoln Road at the existing Ritz-Carlton entrance. The results of the analysis show that a total of 9 valet attendants would be able to handle the demand during the PM peak of generator (worst case scenario) at the drop-off / pick-up area with an average queue of approximately 1 vehicle or less. It is our professional opinion that the additional trips from the proposed tower will not have an adverse impact on the operations of the existing valet station on Lincoln Road and will not impede the access to/from the existing parking garage.

We stand ready to provide any support needed for this project. Should you have any questions or comments, please call me at (305) 447-0900.

Sincerely

Juan Espinosa, PE Vice-President – Transportation

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Attachment A



Attachment B

Scenario - 1

User Group: No. of Years to Project Traffic :

Dev. phase: 1

Analyst Note:

Scenario Name: Existing

Warning: The time periods among the land uses do not appear to match.

VEHICLE TRIPS BEFORE REDUCTION

Land Use & Data Source	Location	10/	Siza	Time Parind	Method	Entry	Exit	Total		
	Cocasion		Califie	unite aschure	Rate/Equation	Split%	Split%	Terret		
310 - Hotel	General	Rooms	07	Maakday	Best Fit (LIN)	312	312			
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	NUOIIIS	33	weekuay	T = 11.29(X) - 426.97	50%	50%	024		
310(1) - Hotel	General	Poomr	93	93	Rooms 93	Weekday, Peak Hour of	Best Fit (LIN)	24	17	41
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	ROOTIS				35	Adjacent Street Traffic,	T = 0.50(X) - 5.34	59%	41%
310(2) - Hotel	General	Pooms	02	Weekday, Peak Hour of	Best Fit (LIN)	22	21	12		
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	NOOIIIS	33	Adjacent Street Traffic,	T = 0.75(X) - 26.02	51%	49%	45		

Scenario - 2

Scenario Name: Proposed

User Group: No. of Years to Project _D Traffic :

Dev. phase: 1 Analyst Note:

Warning: The time periods among the land uses do not appear to match.

VEHICLE TRIPS BEFORE REDUCTION

Frank Line & Posts Frances	Providence and Providence			Trans Desired	Method	Entry	Exit		
Land Use & Data Source	Location		5140	Time Period	Rate/Equation	Split%	Split%	Total	
310 - Hotel	General	Beams	[2	Mankdow	Best Fit (LIN)	86	86	172	
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	ROUINS		Rooms 55	weekday	T = 11.29(X) - 426.97	50%	50%	1/2
310(1) - Hotel	General	Boome	Pagers 52		Best Fit (LIN)	12	9	21	
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	NUUITIS	33	Adjacent Street Traffic,	T = 0.50(X) - 5.34	59%	41%	21	
310(2) - Hotel	General	Deams	53	Rooms 53	Weekday, Peak Hour of	Best Fit (LIN)	7	7	14
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	ROOTIS			Adjacent Street Traffic,	T = 0.75(X) - 26.02	51%	49%	1 14
221 - Multifamily Housing (Mid-Rise)	General	Dwelling Units	52	Maakday	Best Fit (LIN)	141	141	202	
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Dwening onits		Weekday	T = 5.45(X) - 1.75	50%	50%	282	
221(1) - Multifamily Housing (Mid-Rise)	General	Duvelling Linite		Weekday, Peak Hour	Best Fit (LOG)	5	13	40	
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Dwening Units	52	of Adjacent Street	Ln(T) =0.98Ln(X) - 0.98	26%	74%	18	
221(2) - Multifamily Housing (Mid-Rise)	General	Dwelling Units	52	5 52	Weekday, Peak Hour of	Best Fit (LOG)	14	9	22
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Dwelling Units 52			Adjacent Street Traffic,	Ln(T) =0.96Ln(X) - 0.65	61%	39%	23

COMMUTING CHARACTERISTICS BY SEX



Note: This is a modified view of the original table produced by the U.S. Census Bureau. This download or printed version may have missing information from the original table.

	Census Tract 42.06, Miami-Dade County, Flo	rida
	Total	
Label	Estimate	Margin of Error
✓ Workers 16 years and over	735	±187
✓ MEANS OF TRANSPORTATION TO WORK		
✓ Car, truck, or van	53.2%	±12.5
Drove alone	36.1%	±14.9
✓ Carpooled	17.1%	±10.5
In 2-person carpool	15.5%	±9.8
In 3-person carpool	0.0%	±5.3
In 4-or-more person carpool	1.6%	±2.5
Workers per car, truck, or van	1.20	±0.17
Public transportation (excluding taxicab)	10.6%	±8.8
Walked	25.0%	±10.4
Bicycle	0.4%	±1.2
Taxicab, motorcycle, or other means	7.1%	±5.1
Worked at home	3.7%	±3.9
✓ PLACE OF WORK		
✓ Worked in state of residence	91.8%	±6.6
Worked in county of residence	91.8%	±6.6
Worked outside county of residence	0.0%	±5.3
Worked outside state of residence	8.2%	±6.6
✓ Living in a place	100.0%	±5.3
Worked in place of residence	35.1%	±13.8
Worked outside place of residence	64.9%	±13.8
Not living in a place	0.0%	±5.3
 Living in 12 selected states 	0.0%	±5.3
Worked in minor civil division of residence	0.0%	±5.3
Worked outside minor civil division of residence	0.0%	±5.3
the list of the elected states	100.0%	15.0

Table Notes

COMMUTING CHARACTERISTICS BY SEX

Survey/Program: American Community Survey Year: 2018 Estimates: 5-Year Table ID: S0801

Although the American Community Survey (ACS) produces population, demographic and housing unit estimates, it is the Census Bureau's Population Estimates Program that produces and disseminates the official estimates of the population for the nation, states, counties, cities, and towns and estimates of housing units for states and counties.

Source: U.S. Census Bureau, 2014-2018 American Community Survey 5-Year Estimates

When information is missing or inconsistent, the Census Bureau logically assigns an acceptable value using the response to a related question or questions. If a logical assignment is not possible, data are filled using a statistical process called allocation, which uses a similar individual or household to provide a donor value. The "Allocated" section is the number of respondents who received an allocated value for a particular subject.

Data are based on a sample and are subject to sampling variability. The degree of uncertainty for an estimate arising from sampling variability is represented through the use of a margin of error. The value shown here is the 90 percent margin of error. The margin of error can be interpreted roughly as providing a 90 percent probability that the interval defined by the estimate minus the margin of error and the estimate plus the margin of error (the lower and upper confidence bounds) contains the true value. In addition to sampling variability, the ACS estimates are subject to nonsampling error (for a discussion of nonsampling variability, see ACS Technical Documentation). The effect of nonsampling error is not represented in these tables.

The 12 selected states are Connecticut, Maine, Massachusetts, Michigan, Minnesota, New Hampshire, New Jersey, New York, Pennsylvania, Rhode Island, Vermont, and Wisconsin.

Workers include members of the Armed Forces and civilians who were at work last week.

While the 2014-2018 American Community Survey (ACS) data generally reflect the February 2013 Office of Management and Budget (OMB) definitions of metropolitan and micropolitan statistical areas; in certain instances the names, codes, and boundaries of the principal cities shown in ACS tables may differ from the OMB definitions due to differences in the effective dates of the geographic entities.

Estimates of urban and rural populations, housing units, and characteristics reflect boundaries of urban areas defined based on Census 2010 data. As a result, data for urban and rural areas from the ACS do not necessarily reflect the results of ongoing urbanization.

Explanation of Symbols:

An "**" entry in the margin of error column indicates that either no sample observations or too few sample observations were available to compute a standard error and thus the margin of error. A statistical test is not appropriate.

An "-" entry in the estimate column indicates that either no sample observations or too few sample observations were available to compute an estimate, or a ratio of medians cannot be calculated because one or both of the median estimates falls in the lowest interval or upper interval of an open-ended distribution, or the margin of error associated with a median was larger than the median itself.

An "-" following a median estimate means the median falls in the lowest interval of an open-ended distribution.

An "+" following a median estimate means the median falls in the upper interval of an open-ended distribution.

An "***" entry in the margin of error column indicates that the median falls in the lowest interval or upper interval of an open-ended distribution. A statistical test is not appropriate.

An "*****" entry in the margin of error column indicates that the estimate is controlled. A statistical test for sampling variability is not appropriate.

An "N" entry in the estimate and margin of error columns indicates that data for this geographic area cannot be displayed because the number of sample cases is too small.

An "(X)" means that the estimate is not applicable or not available.

Supporting documentation on code lists, subject definitions, data accuracy, and statistical testing can be found on the American Community Survey website in the Technical Documentation section.

Sample size and data quality measures (including coverage rates, allocation rates, and response rates) can be found on the American Community Survey website in the Methodology section.

Attachment C

Scenario - 3		
Scenario Name: Queuing	User Group:	
Dev. phase: 1	No. of Years to 0 Project Traffic :	
Analyst Note:		

Warning: The time periods among the land uses do not appear to match.

VEHICLE TRIPS BEFORE REDUCTION

		Takes and			Method	Entry	Exit	
Land Use & Data Source	Location	Location IV Size Time Period		Rate/Equation	Split%	Split%	Total	
310(2) - Hotel	General	Passa	F2	Maakday, AM Dook Llour of Concreter	Best Fit (LOG)	19	17	26
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Rooms	53	weekday, Alvi Peak Hour of Generator	Ln(T) =0.84Ln(X) + 0.25	54%	46%	50
221 - Multifamily Housing (Mid-Rise)	General	Dwolling Units	F.2	Weekday, AM Book Hour of Congrator	Best Fit (LOG)	5	15	20
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Dweining Offics	52	weekday, Alvi Feak Hour of Generator	Ln(T) =0.83Ln(X) - 0.27	27%	73%	20
310(1) - Hotel	General	Pearme	274	Weekday, AM Peak Hour of Constator	Best Fit (LOG)	101	86	197
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	KUOIIIS	574	Weekday, Alvi Feak Hour of Generator	Ln(T) =0.84Ln(X) + 0.25	54%	46%	107
310(3) - Hotel	General	Poomo	E2	Weekday, RM Reak Hour of Constator	Best Fit (LOG)	20	15	25
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	ROOTIS	22	Weekuay, FIVI Feak Hour of Generator	Ln(T) =0.93Ln(X) - 0.14	58%	42%	55
221(1) - Multifamily Housing (Mid-Rise)	General	Dwelling Lipite	F.2	Maakday, DM Baak Hous of Conorator	Best Fit (LOG)	15	10	25
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	Dweiling Onits	52	weekday, Pivi Peak Hour of Generator	Ln(T) =0.83Ln(X) - 0.05	60%	40%	2.5
310(4) - Hotel	General	Deama	450	Markday, DM Dock Hour of Concretor	Best Fit (LOG)	125	90	215
Data Source: Trip Gen Manual, 10th Ed	Urban/Suburban	KOOMS	374	weekday, Pivi Peak Hobi of Generator	Ln(T) =0.93Ln(X) - 0.14	58%	42%	213

Applications of Queueing Analysis

location, a 5% probability of back-up onto the adjacent street is judged to be acceptable. Demand on the system for design is expected to be 110 vehicles in a 45-minute period. Average service time was expected to be 2.2 minutes. Is the queue storage adequate?

Such problems can be quickly solved using Equation (8-9b) given in Table 8-10 and repeated below for convenience.

$$M = \left[\frac{\ln P(x > M) - \ln Q_M}{\ln \rho}\right] - 1$$

where:

- M = queue length which is exceeded p percent of the time
- N = number of service channels (drive-in positions)
- Q = service rate per channel (vehicles per hour) G (G)

$$\rho = \frac{\text{demand rate}}{\text{service rate}} = \frac{q}{NQ} = \text{utilization factor}$$

- q = demand rate on the system (vehicles per hour)
- $Q_{\rm M}$ = tabled values of the relationship between queue length, number of channels, and utilization factor (see Table 8.11)

TABLE 8-11

Table of Q_M Values

A	N = 1	2	3	4	6	8	10
0.0	0.0000	0.0000	0.0000	0.0000			
Q.1	.1000	.0182	.0037	8000.	.0000	0.0000	0.0000
2	.2000	.0666	,0247	.0096	.0015	.0002	.0000
.3	.3000	.1385	.0700	.0370	.0111	.0036	.0011
.4	,4000	.2286	.1411	.0907	.0400	.0185	.0068
.5	.5000	.3333	.2368	.1739	.0991	.0591	.0360
.6	.6000	.4501	.3548	.2870	.1965	.1395	.1013
.7	.7000	.5766	.4923	.4286	.3359	.2706	.2218
.8	.8000	.7111	.6472	.5964	.5178	.4576	.4093
.9	.9000	.8526	.6172	.7878	.7401	,7014	.6687
1.0	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000

arrival rate, total p

NO (number of channels) (service rate per channel)

N – number of channels (service positions).

Solution

Stop 11	$G = \frac{60 \text{ min/hr}}{1000 \text{ min/hr}} = 27.2 \text{ complete the last here.}$
otep 1:	$Q = \frac{21.5}{2.2 \text{ min/service}} = 21.5 \text{ services per nour}$
Step 2:	$q = (110 \text{ veh}/45 \text{ min}) \times (60 \text{ min/hr}) = 146.7 \text{ vehicles per hour}$
Step 3:	$\rho = \frac{q}{NQ} = \frac{146.7}{(6)(27.3)} = 0.8956$

- Step 4: $Q_M = 0.7303$ by interpolation between 0.8 and 0.9 for N = 6 from the table of Q_M values (see Table 8-11).
- Step 5: The acceptable probability of the queue, M, being longer than the storage, 18 spaces in this example, was stated to be 5%. P(x > M) = 0.05, and:

$$M = \left[\frac{\ln 0.05 - \ln 0.7303}{\ln 0.8956}\right] - 1 = \left[\frac{-2.996 - (-0.314)}{-0.110}\right] - 1$$

= 24.38 - 1 = 23.38, say 23 vehicles.