MASONRY OPENING

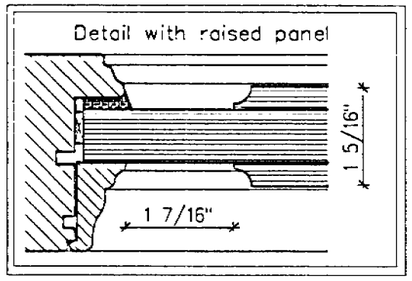
ROUGH OPENING — 1 1/2"

0 3/8" — FRAME WIDTH — 0 3/8"

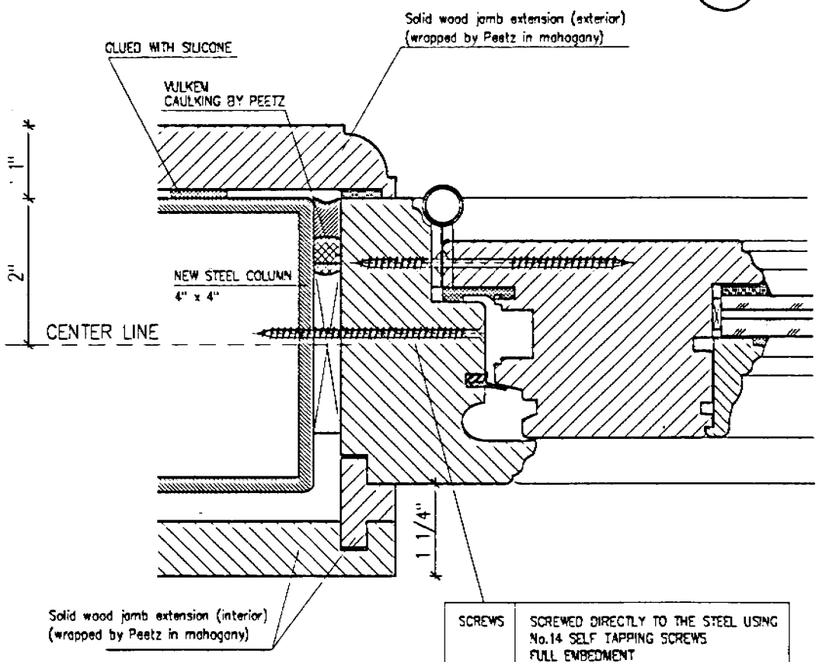
LEAF SIZE

5 11/16" — 1 3/8" — 4 5/16"

5 11/16" — 4 5/16" — 1 3/8"



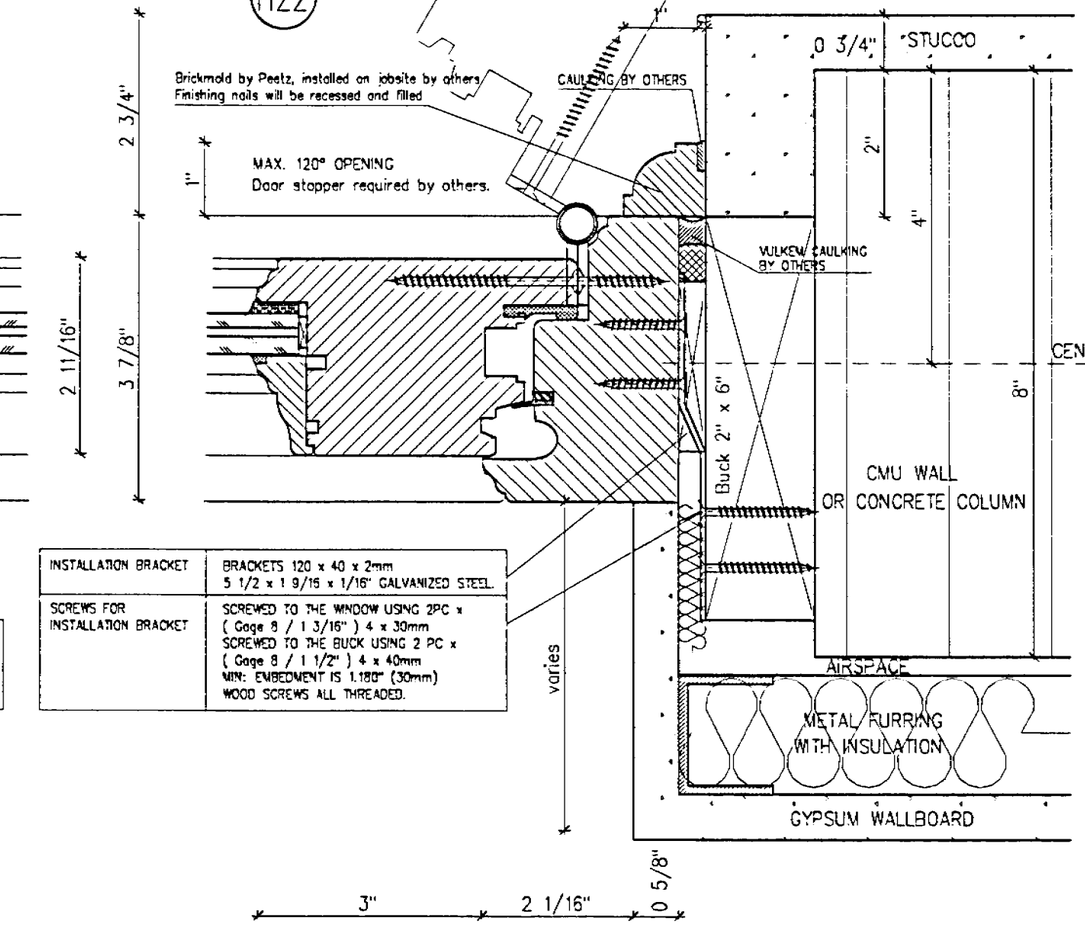
H21



SCREWS	SCREWED DIRECTLY TO THE STEEL USING No.14 SELF TAPPING SCREWS FULL EMBEDMENT MIN. EDGE DISTANCE IS 1/2" (13mm)

0 3/4" — 1 15/16" — 3"

H22



INSTALLATION BRACKET	BRACKETS 120 x 40 x 2mm 5 1/2 x 1 9/16 x 1/16" GALVANIZED STEEL.
SCREWS FOR INSTALLATION BRACKET	SCREWED TO THE WINDOW USING 2PC x (Gage 8 / 1 3/16") 4 x 30mm SCREWED TO THE BUCK USING 2 PC x (Gage 8 / 1 1/2") 4 x 40mm MIN: EMBEDMENT IS 1.180" (30mm) WOOD SCREWS ALL THREADED.

3" — 2 1/16" — 0 5/8"

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WINDOWS AND DOORS

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Horizontal
Outswing Door
H21 – H22

GAINOR RESIDENCE
5800 North Bay Road
Miami Beach, Florida

Mr. Greg Shirley
HANDCRAFT CONSTRUCTION
7610 N.W. 6th AVENUE
BOCA RATON, FLORIDA 33487
Tel: (561) 241-9911

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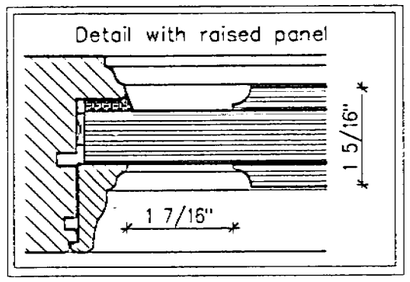

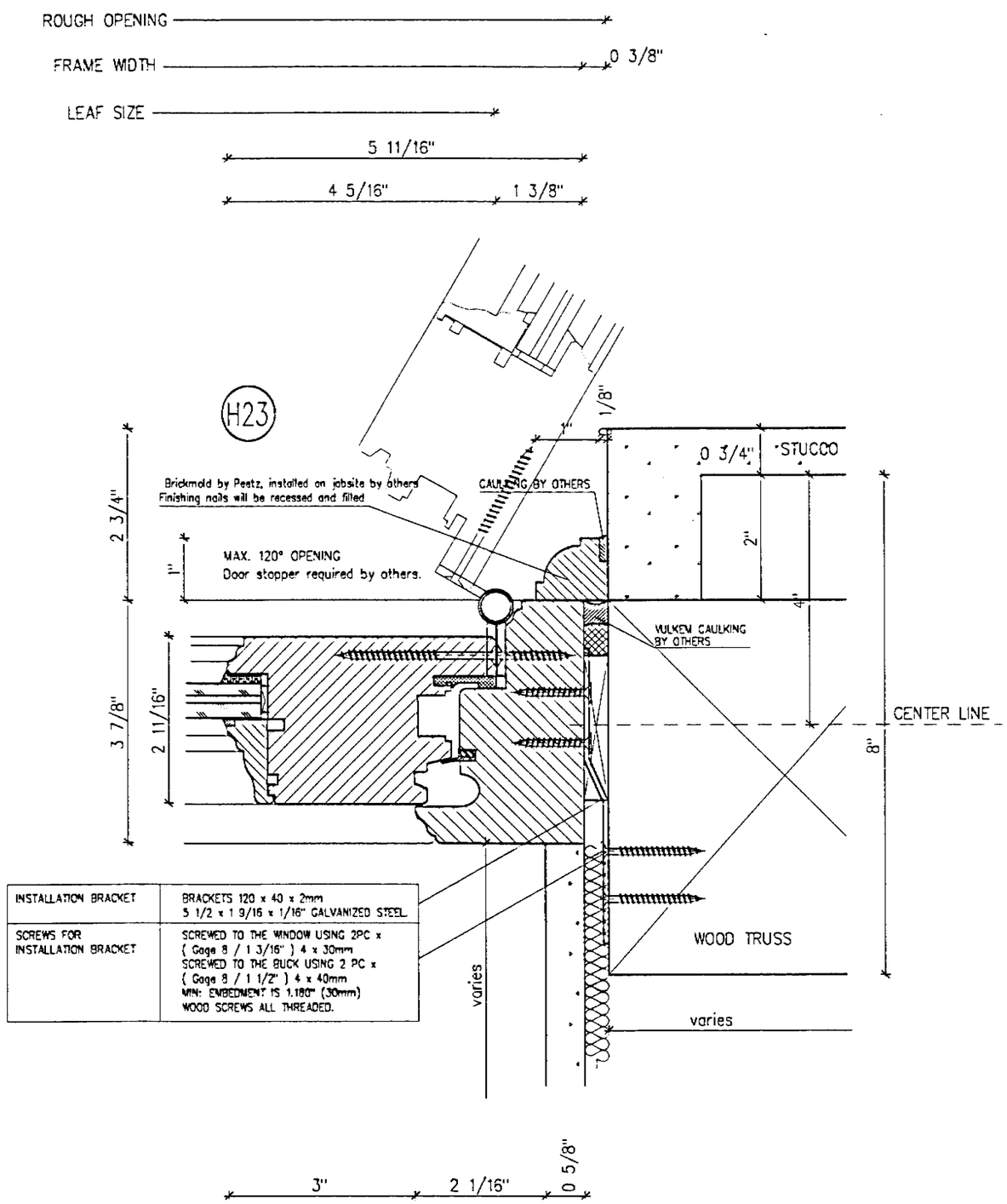

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Horizontal
Outswing Door
H23

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MASONRY OPENING

ROUGH OPENING — 1 1/2"

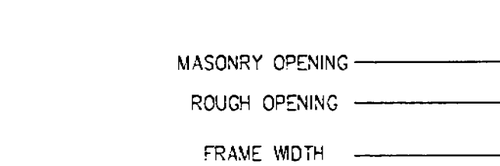
FRAME WIDTH — 0 3/8"

LEAF SIZE

5 11/16"

4 5/16" — 1 3/8"

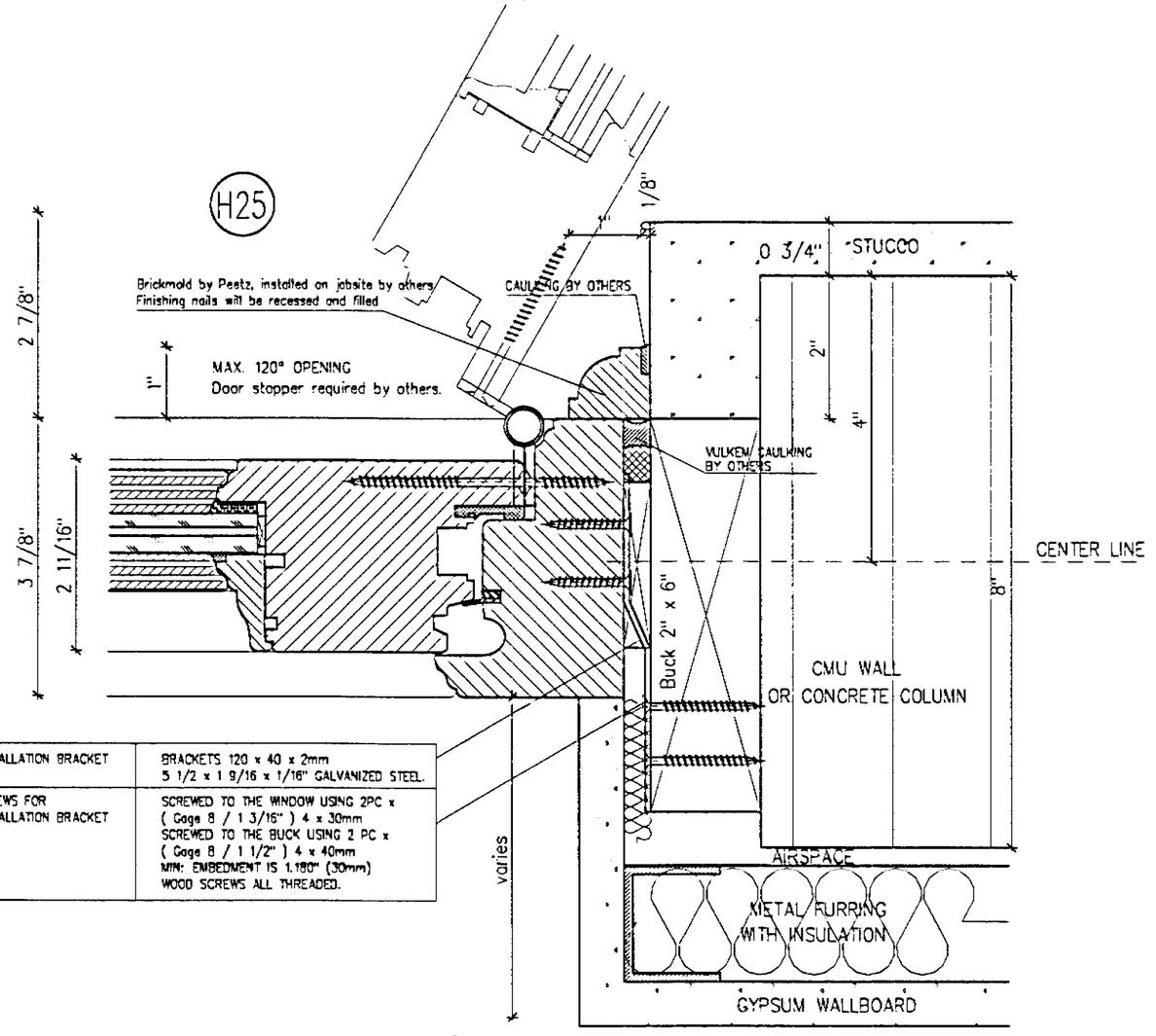
H24



0 9/16"

0 9/16"

H25



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Horizontal
Outswing Door
H24 – H25

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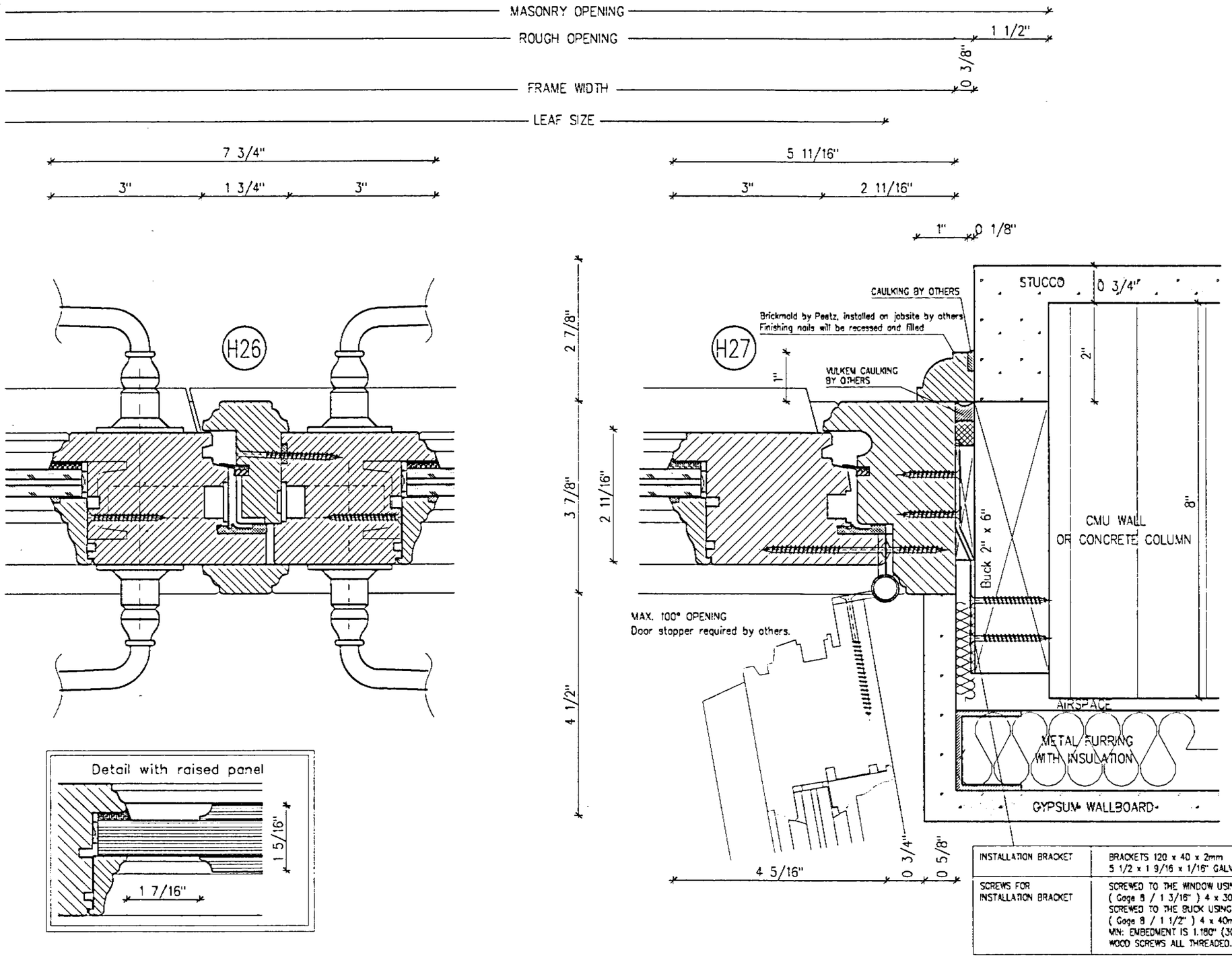

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Horizontal
Inswing Door
H26 – H27

GAINOR RESIDENCE
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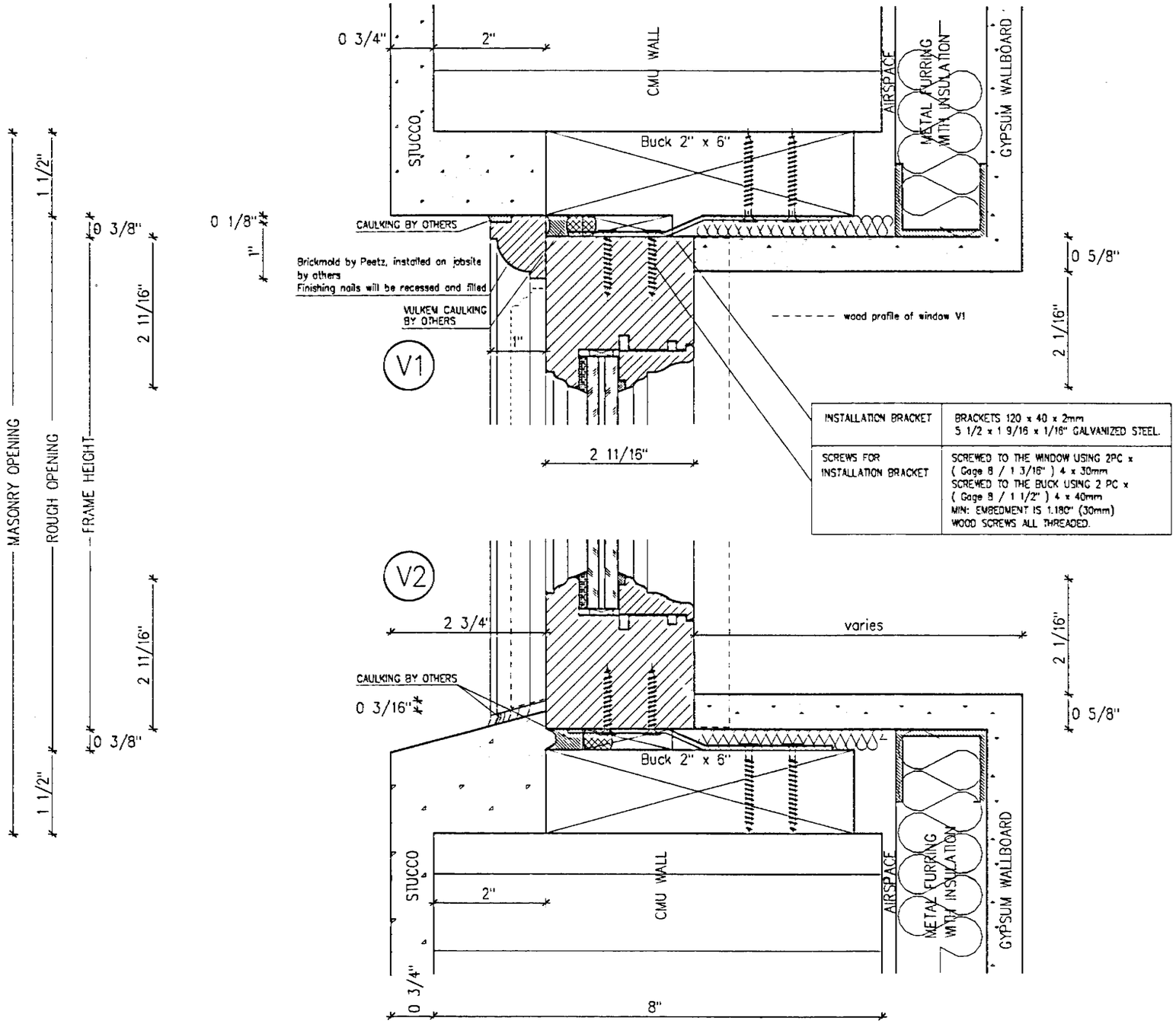

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Vertical
Direct Set Window
V1 – V2

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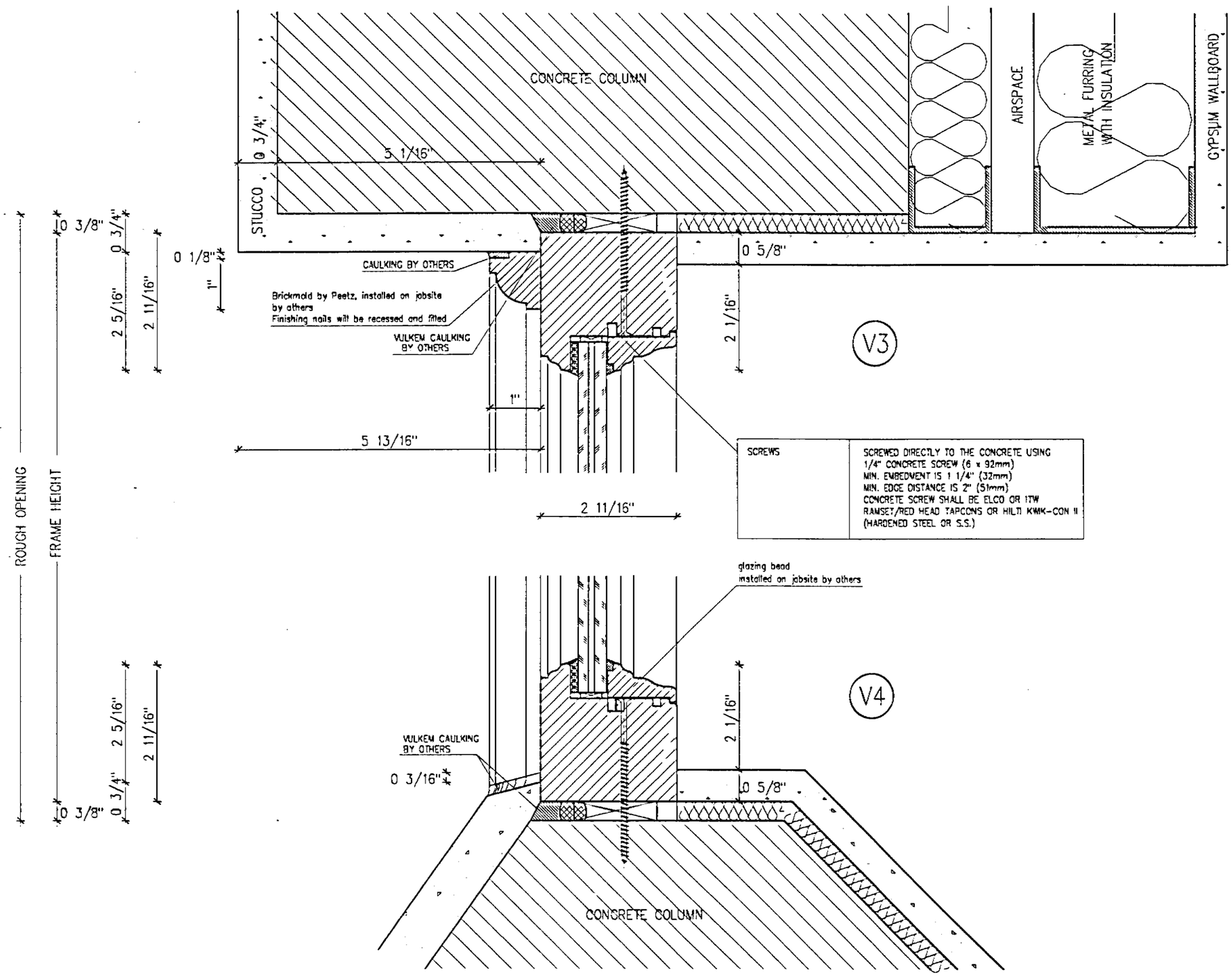

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Vertical
Direct Set Window
V3 - V4

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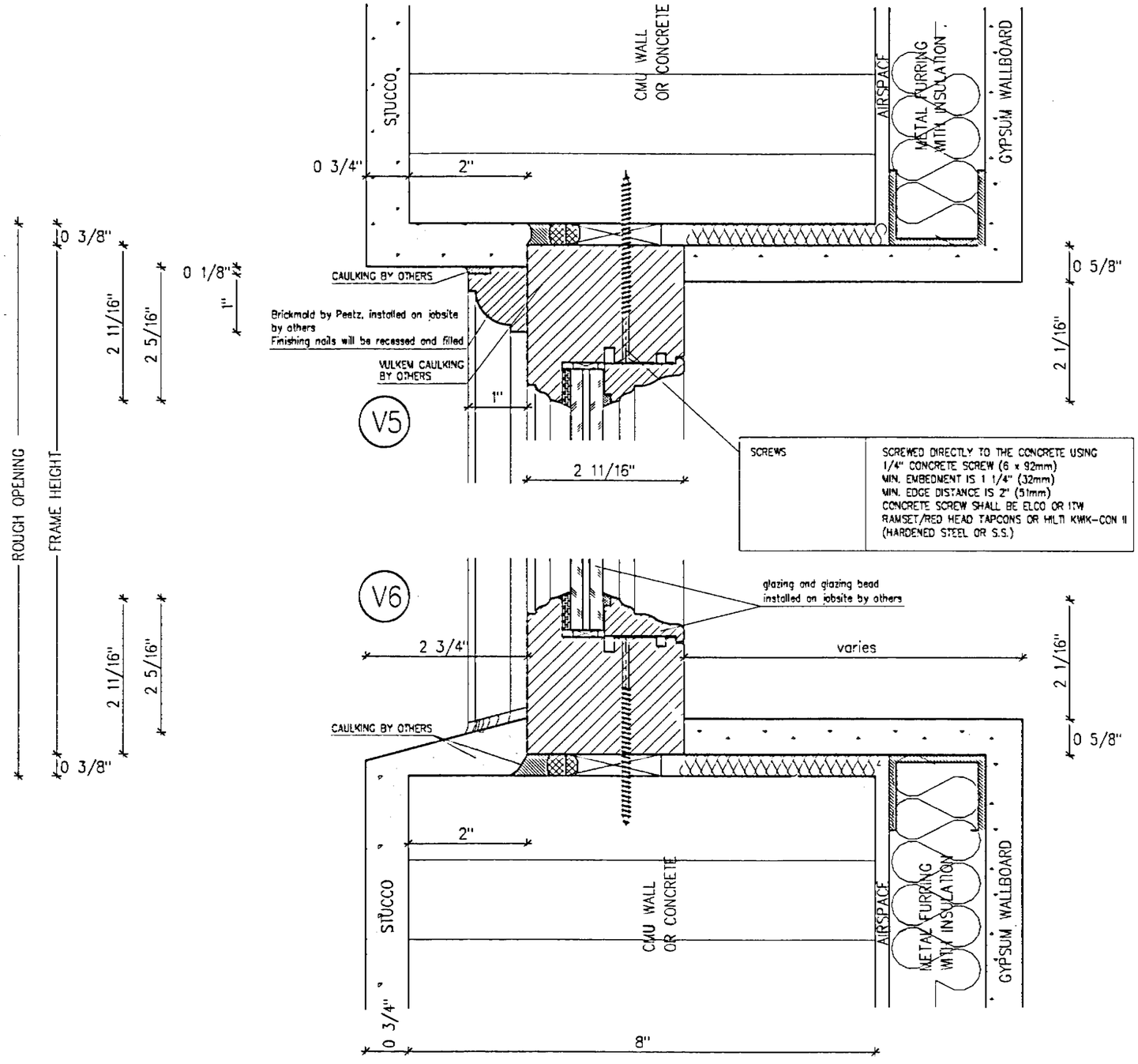

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Vertical
Direct Set Window
V5 - V6

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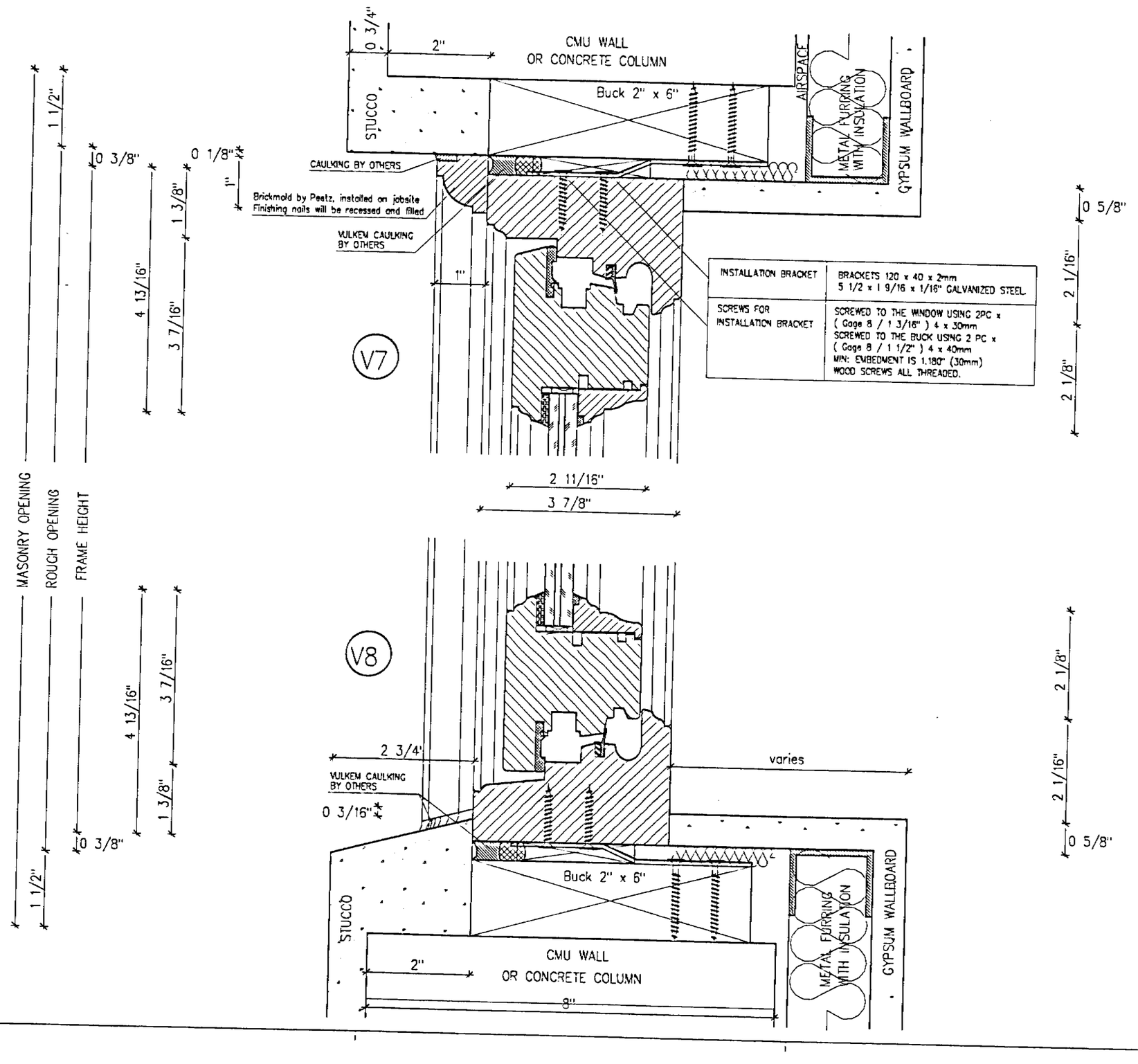

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Vertical
Casement Window
V7 – V8

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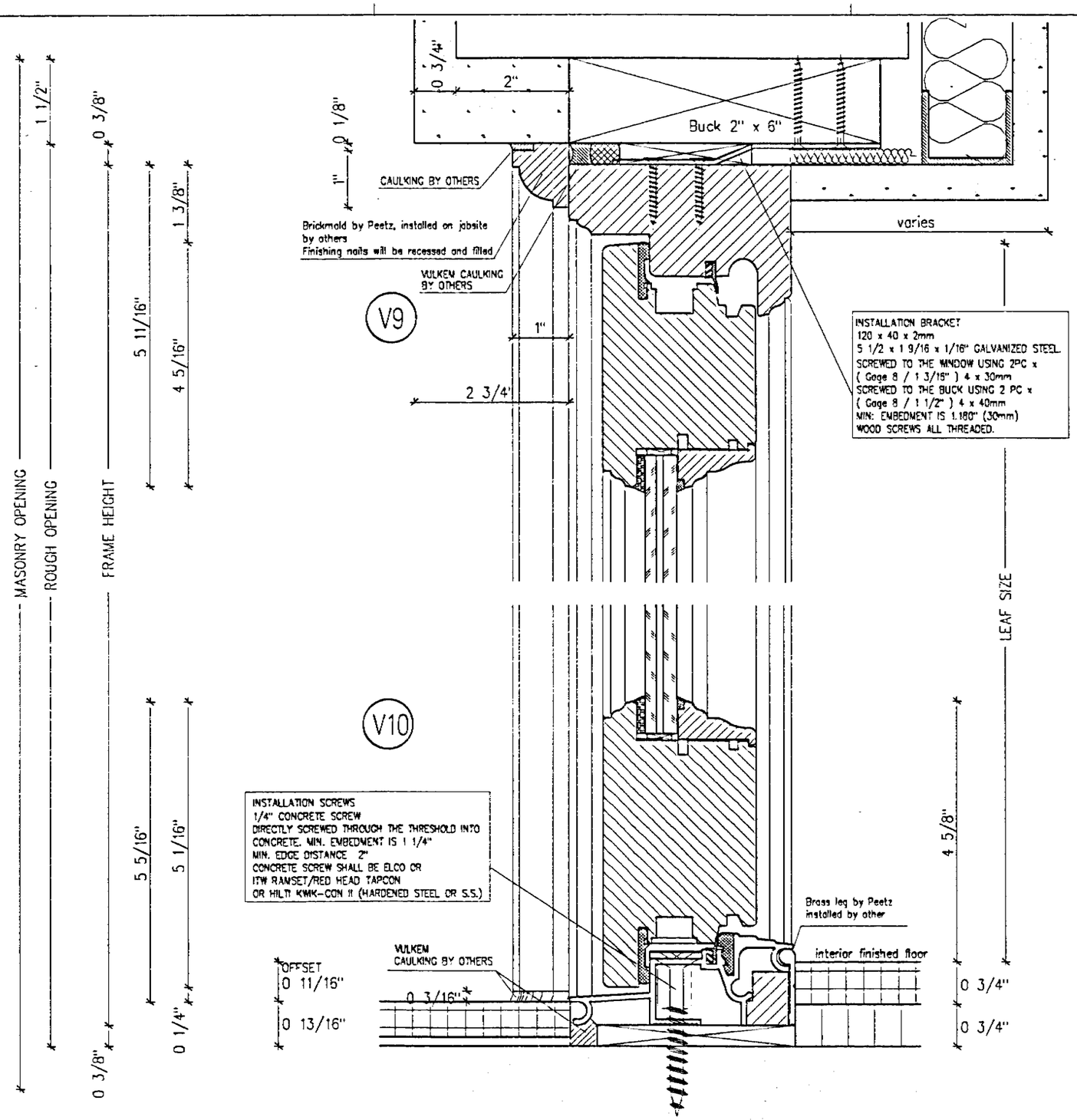

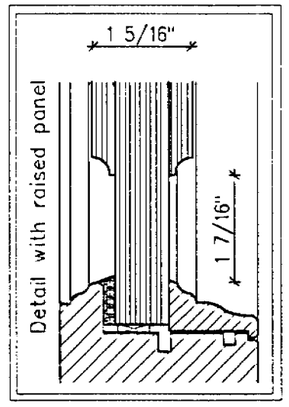

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Vertical
Outswing Door
V9 - V10

GAINOR RESIDENCE
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Miami Beach, Florida

Mr. Greg Shirley
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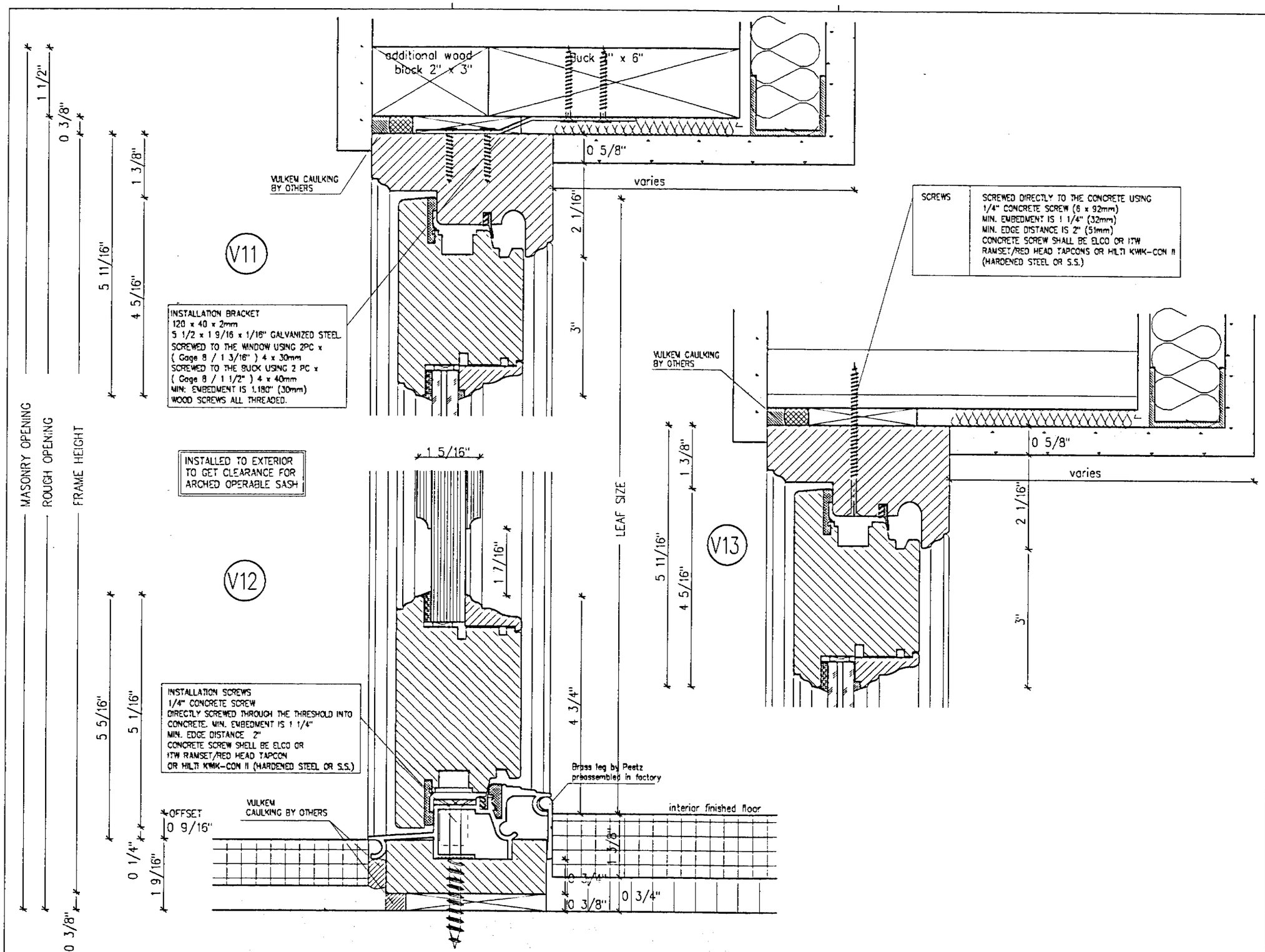

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Vertical
Outswing Door
V11 – V13

GAINOR RESIDENCE
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Miami Beach, Florida

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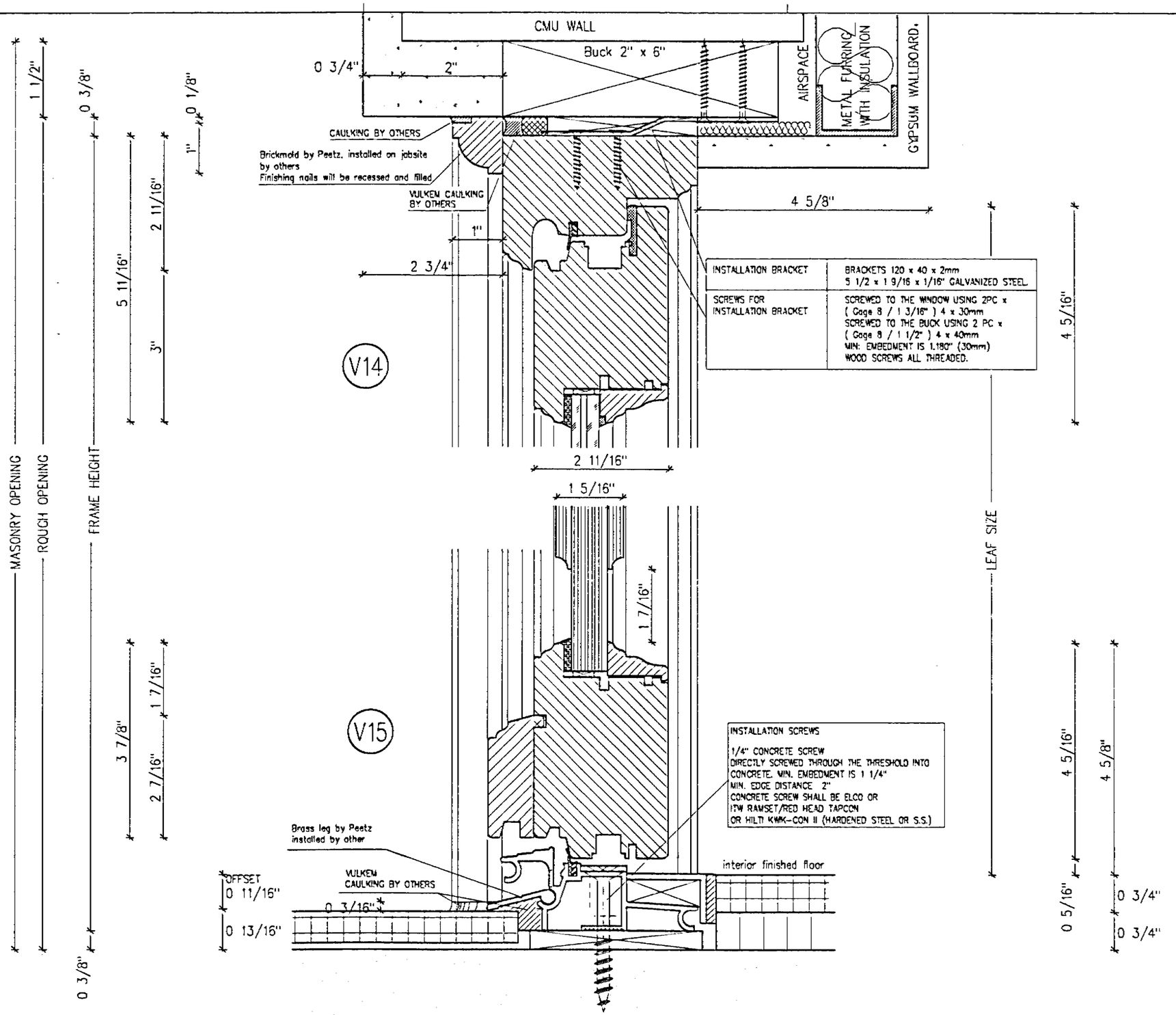

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Vertical
Inswing Door
V14 – V15

GAINOR RESIDENCE
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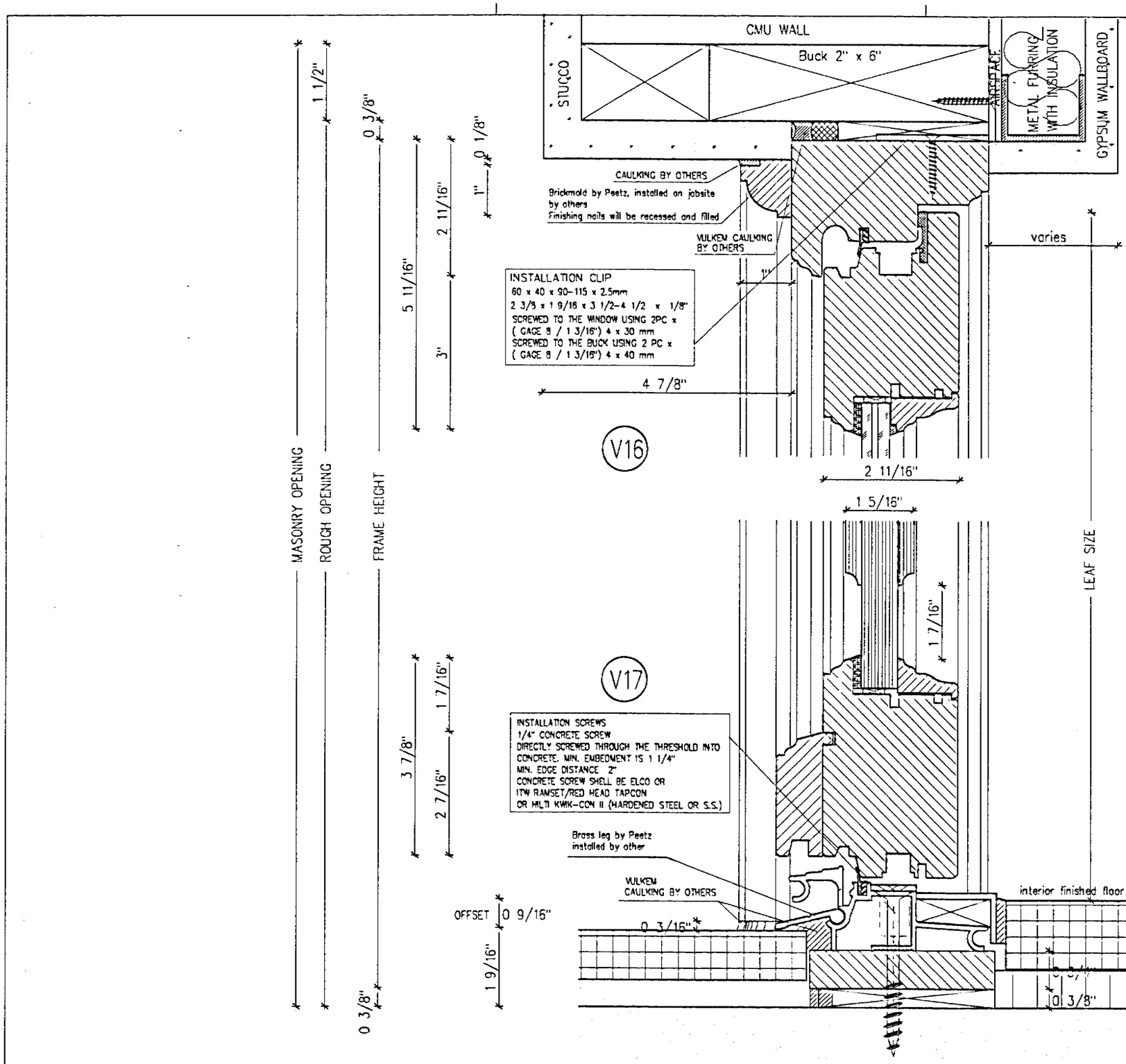

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Vertical
Inswing Door
V16 – V17

GAINOR RESIDENCE
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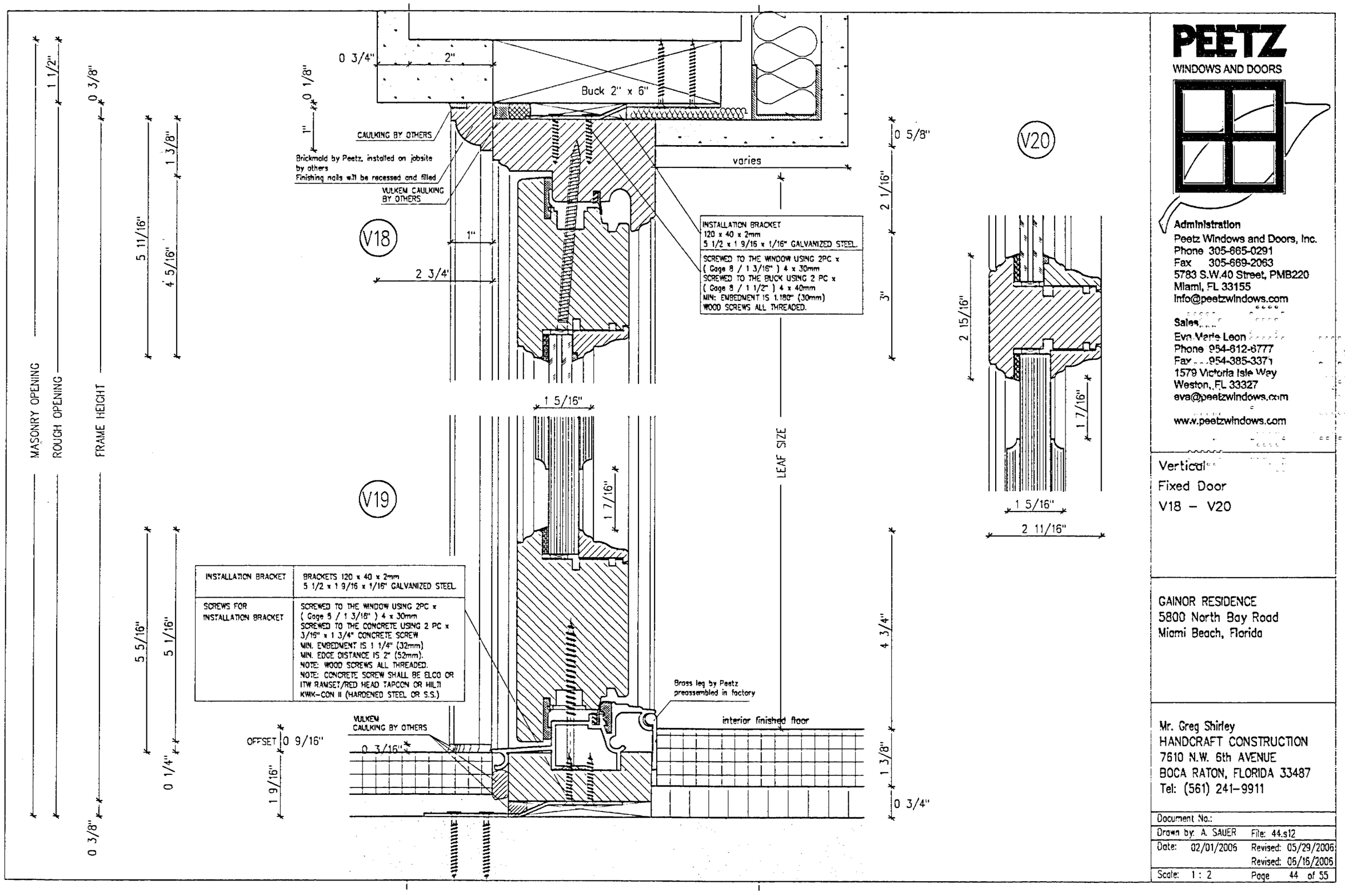

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Vertical
Fixed Door
V18 – V20

GAINOR RESIDENCE
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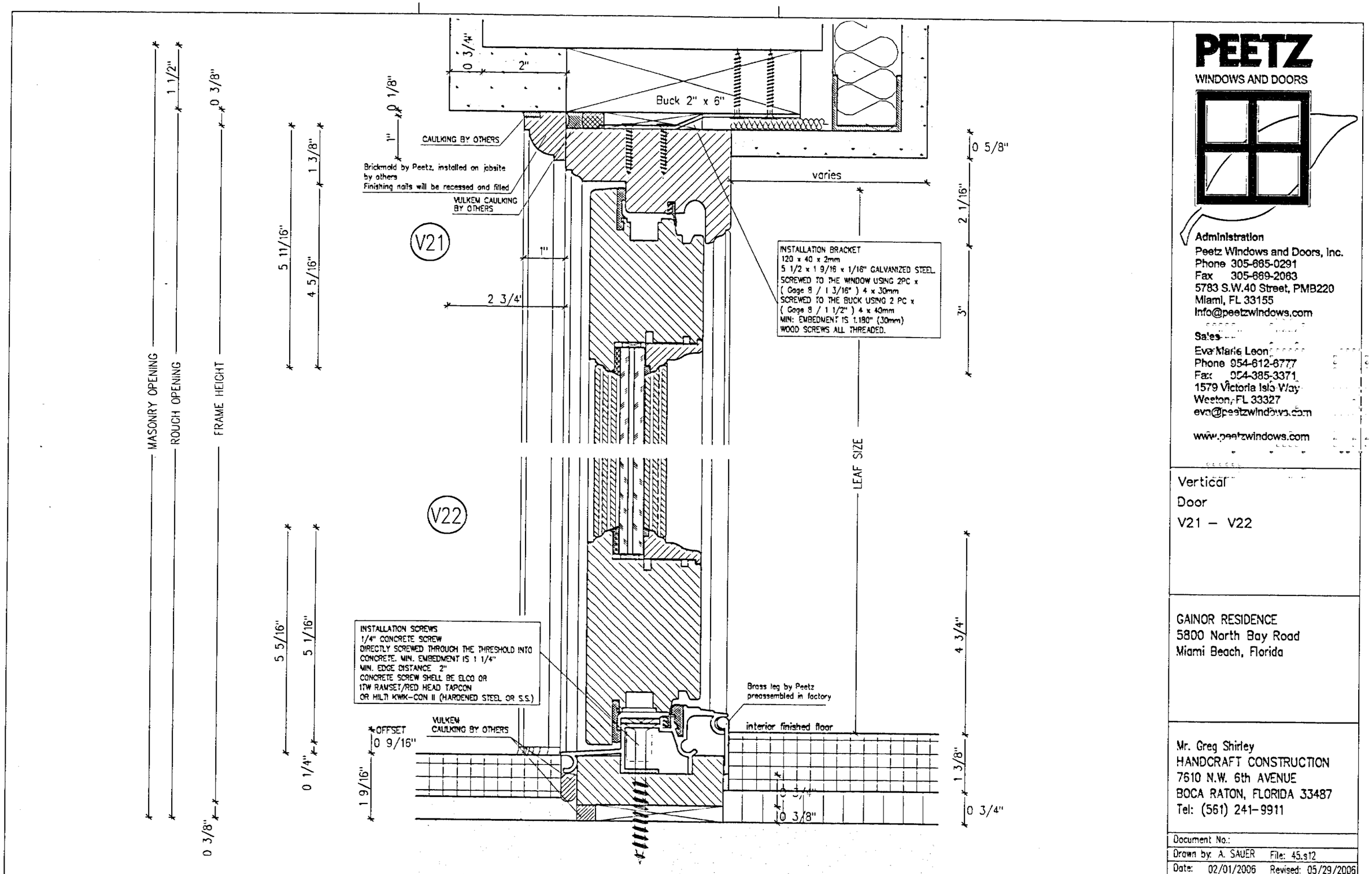

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Vertical
Door
V21 – V22

GAINOR RESIDENCE
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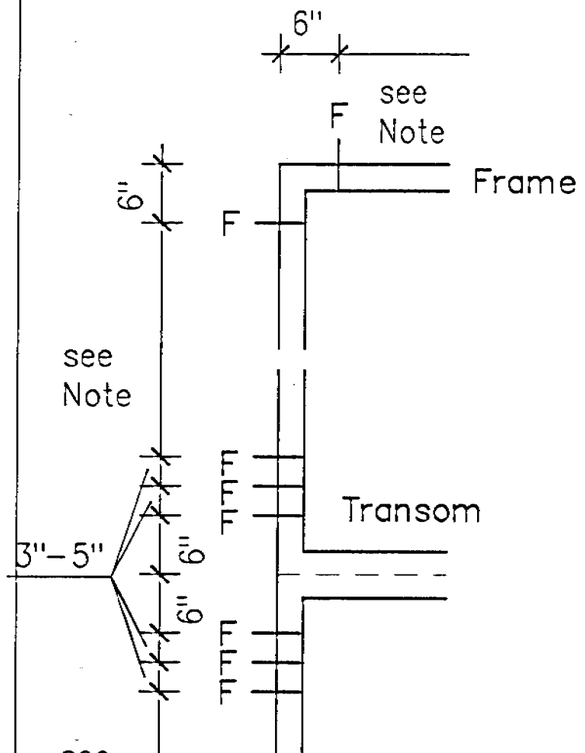
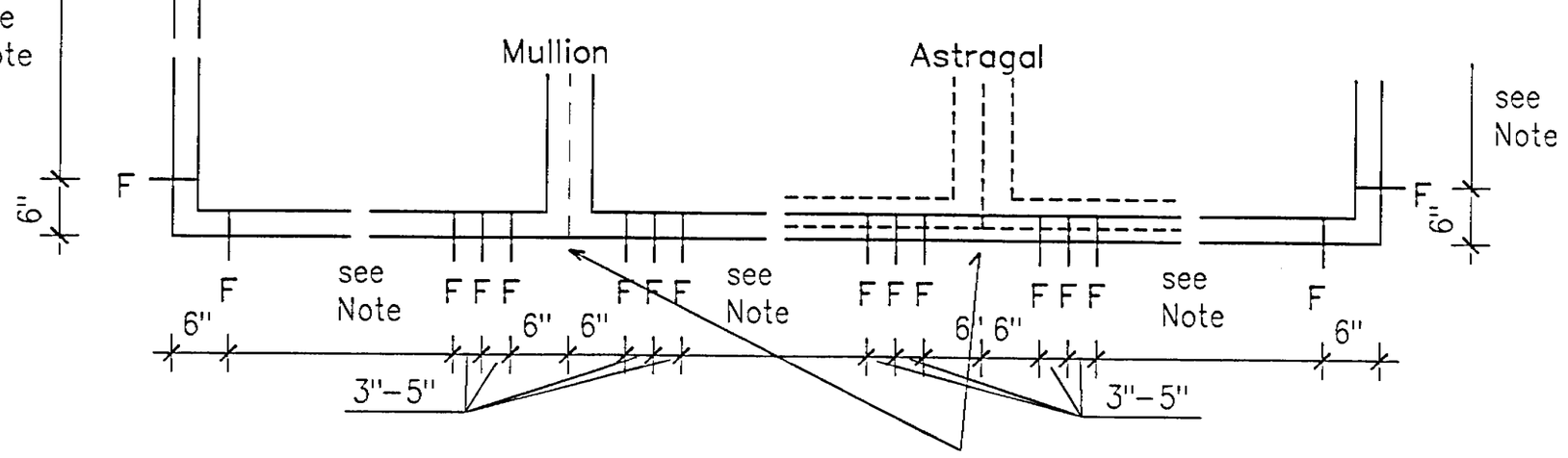


Installation of windows and doors

F = fastening point
use fastening system as shown in the detail

Note:
Distance between the fastening points
Doors 15" O.C. for all attachments (clips, brackets or screws)
Casements 16" O.C. for all attachments (clips, brackets or screws)
Fixed casements (sash set)16" O.C. with brackets
12" O.C. with clips
Direct glazed 12" O.C. with screws and clips
16" O.C. with brackets



Number of Fastening Points at Mullion, Astragal and Transom 6 Pc.

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INSTALLATION

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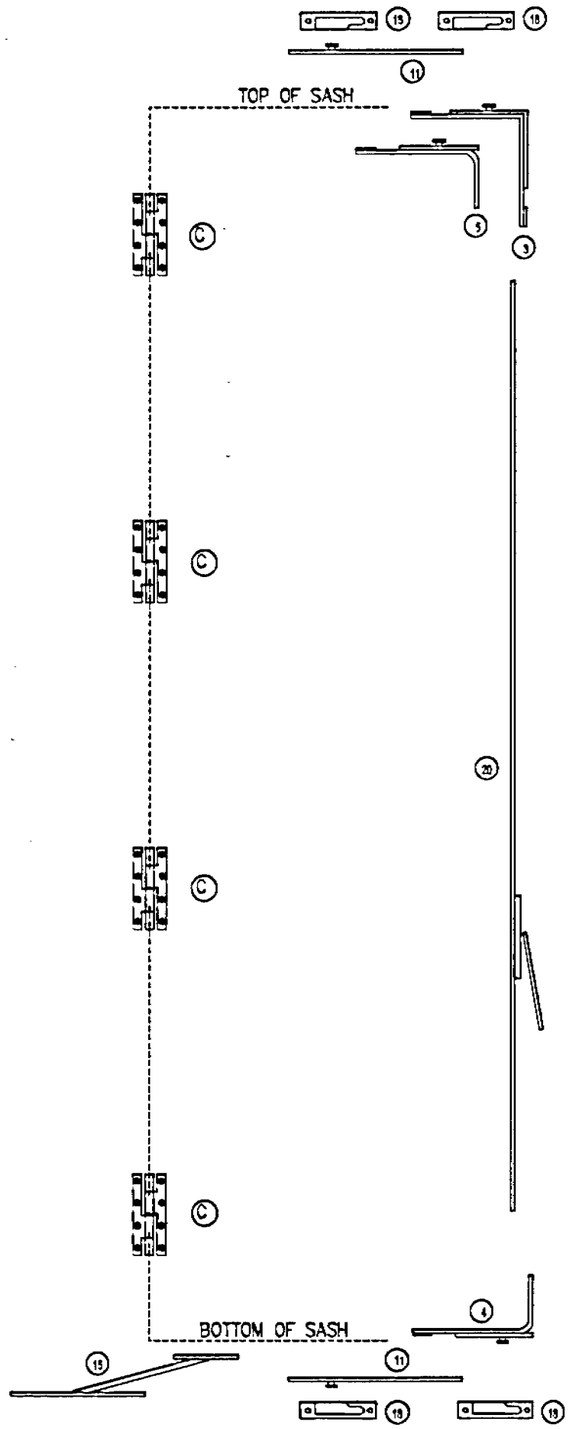
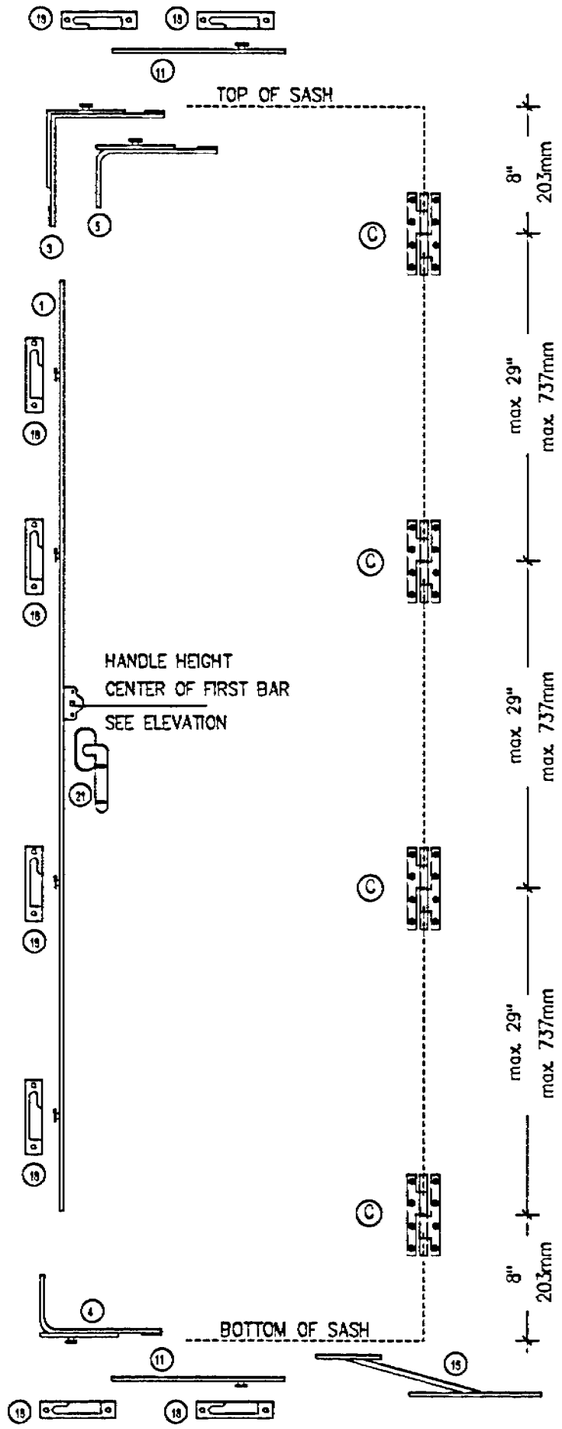
Date: 02/01/2006	Revised: 05/29/2006
	Revised: 06/15/2006
Scale: none	Page 46 of 55





PART #	DESCRIPTION	MATERIAL	LOCATION
		ZINC-NICKEL PLATED STEEL "COASTAL FINISH"	
1	GEAR		ON SASH
3	CORNER TRANSMISSION WITH ONE LOCKING PIN		
4	CORNER TRANSMISSION WITH ONE LOCKING PIN		ON SASH
5	SPECIAL CORNER TRANSMISSION FOR WINDOWS LOWER THEN 13.75"		ON SASH
11	MIDDEL LOCK		ON SASH
15	FRICTION STOP		ON SASH
18	LOCKING PLATE		ON FRAME
20	GEAR FOR INACTIV SASH		ON SASH
21	HANDLE		ON SASH
C	WINDOW HINGES DISTANCE FROM THE CORNER 8"(203mm), MAX. DISTANCE BETWEEN TWO HINGES 29"(737mm). SCREWED WITH 8 Pc STAINLESS STEEL CROSS RECESSED COUNTERSUNK HEAD WOOD SCREWS ALL THREADED DIN 7997-4-60/35mm A2	OIL-RUBBED BRONZE	

CAPS FOR HINGES
IN OIL-RUBBED BRONZE FINISH

PEETZ
WINDOWS AND DOORS

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Peetz Windows and Doors, Inc.
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Weston, FL 33327
eva@peetzwindows.com

www.peetzwindows.com

HARDWARE
WINDOW

GAINOR RESIDENCE
5800 North Bay Road
Miami Beach, Florida

Mr. Greg Shirley
HANDCRAFT CONSTRUCTION
7610 N.W. 6th AVENUE
BOCA RATON, FLORIDA 33487
Tel: (561) 241-9911

Document No.:		
Drawn by: A. SAUER		File: 47.s12
Date: 02/01/2006		Revised: 05/29/2006
		Revised: 06/16/2006
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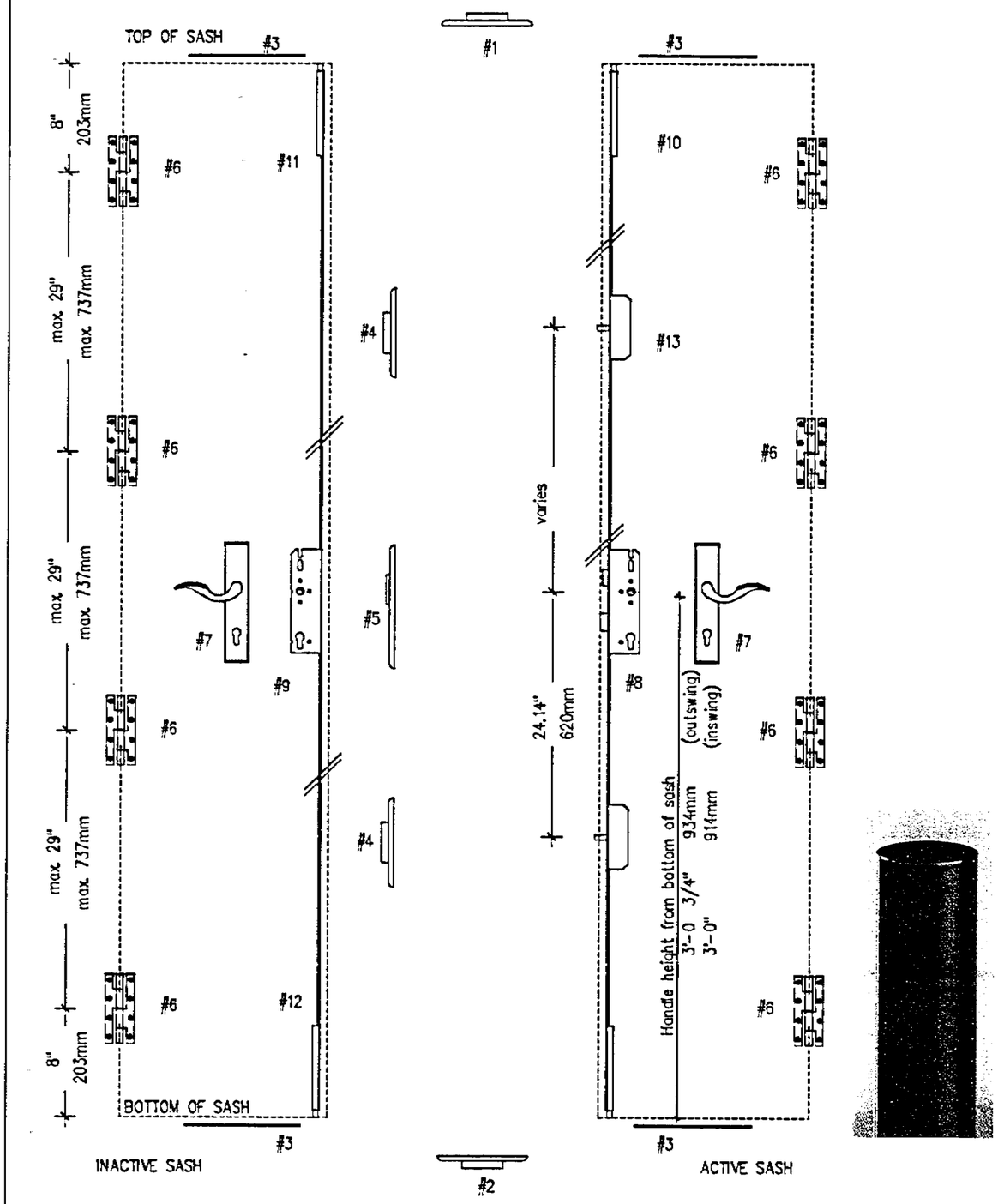

CAPS FOR HINGES
IN OIL-RUBBED BRONZE FINISH

Part #	QTY	DESCRIPTION	MATERIAL	LOCATION
#1	1	DOUBLE BOLT PLATES	STAINLESS STEEL GRADE 300	ON FRAME
#2	1	DOUBLE BOLT PLATES	FACEPLATE BRONZE BOX STAINLESS STEEL GRADE 300	ON FRAME
#3	4	GEAR REINFORCE	STAINLESS STEEL GRADE 300	ON SASH
#4	2	BOLT PLATES	STAINLESS STEEL GRADE 400	ON SASH
#5	1	ADJUSTABLE LATCH/DEATBOLT STRIKE PLATE	STAINLESS STEEL GRADE 400	ON SASH
#6	–	DOOR HINGES	OIL-RUBBED BRONZE	ON SASH AND FRAME
#7	2	HANDLE SET		
#8	1	GEAR	STAINLESS STEEL GRADE 300	ON SASH BACKSET 45mm ACTIV
#9	1	GEAR	STAINLESS STEEL GRADE 300	ON SASH BACKSET 35mm INACTIV
#10	1	TOP EXTENSION	STAINLESS STEEL GRADE 300	ON SASH ACTIV
#11	1	TOP EXTENSION	STAINLESS STEEL GRADE 300	ON SASH INACTIV
#12	1	BOTTOM EXTENSION	STAINLESS STEEL GRADE 300	ON SASH INACTIV
#13	1	MIDDEL EXTENSION	STAINLESS STEEL GRADE 300	ON SASH ACTIV

HARDWARE FOR
OUT- AND INSWING WOOD FRENCH DOORS
MADE BY HOPPE

PEETZ
WINDOWS AND DOORS

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HARDWARE
FRENCH DOOR

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Document No.:	
Drawn by: A. SAUER	File: 48.s12
Date: 02/01/2006	Revised: 05/29/2006
	Revised: 06/16/2006
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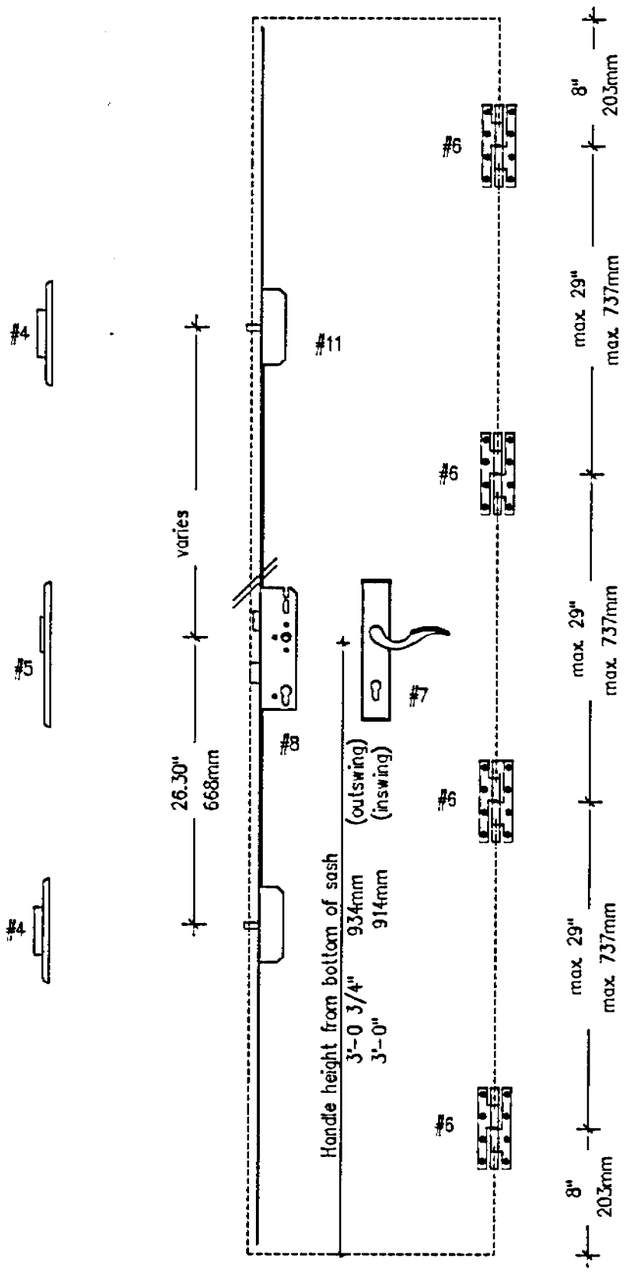

Part #	QTY	DESCRIPTION	MATERIAL	LOCATION
#4	2	BOLT PLATES	STAINLESS STEEL GRADE 400	ON FRAME
#5	1	ADJUSTABLE LATCH/DEATBOLT STRIKE PLATE	STAINLESS STEEL GRADE 400	ON FRAME
#6	–	DOOR HINGES	OIL-RUBBED BRONZE	ON SASH AND FRAME
#7	2	HANDLE SET		
#8	1	GEAR	STAINLESS STEEL GRADE 300	ON SASH BACKSET 45mm ACTIV
#11	1	TOP EXTENSION	STAINLESS STEEL GRADE 300	ON SASH INACTIV

CAPS FOR HINGES
IN OIL-RUBBED BRONZE FINISH

HARDWARE FOR
OUT- AND INSWING WOOD SINGLE DOORS
MADE BY HOPPE

PEETZ
WINDOWS AND DOORS

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HARDWARE
SINGLE DOOR

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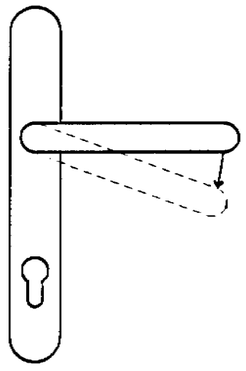
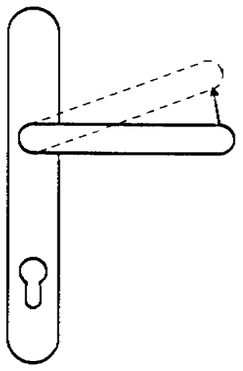
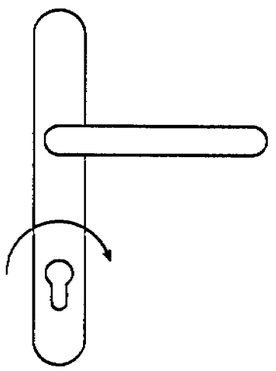
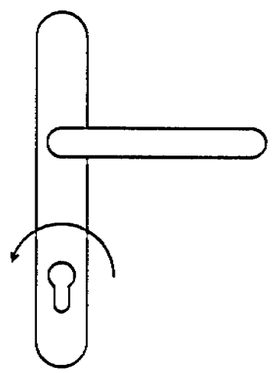
HARDWARE LOCKED WITH HANDLE

FUNCTION	UNLOCK BOLTS, DEADBOLT AND LATCH	LOCK BOLTS AND DEADBOLT	LOCK HANDLE	UNLOCK HANDLE
OPERATION	PUSH HANDLE	LIFT HANDLE	TURN THUMBTURN OR KEY RIGHT	TURN THUMBTURN OR KEY LEFT
SKETCH				

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OPERATION

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Drawn by: A. SAUER	File: 50.s12
Date: 02/01/2006	Revised: 05/29/2006
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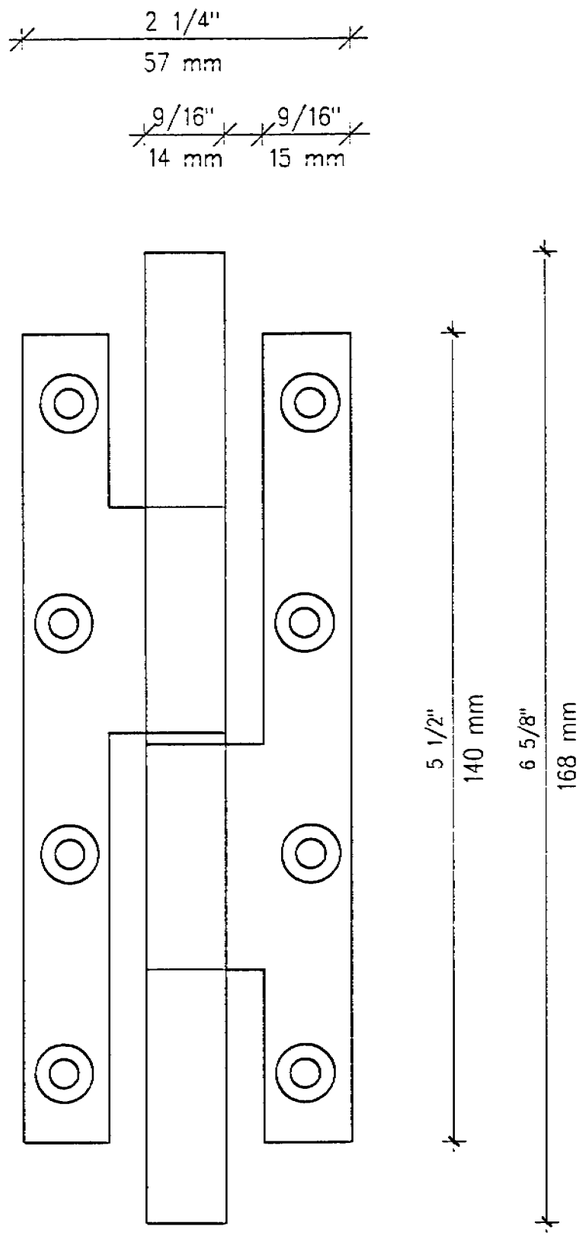
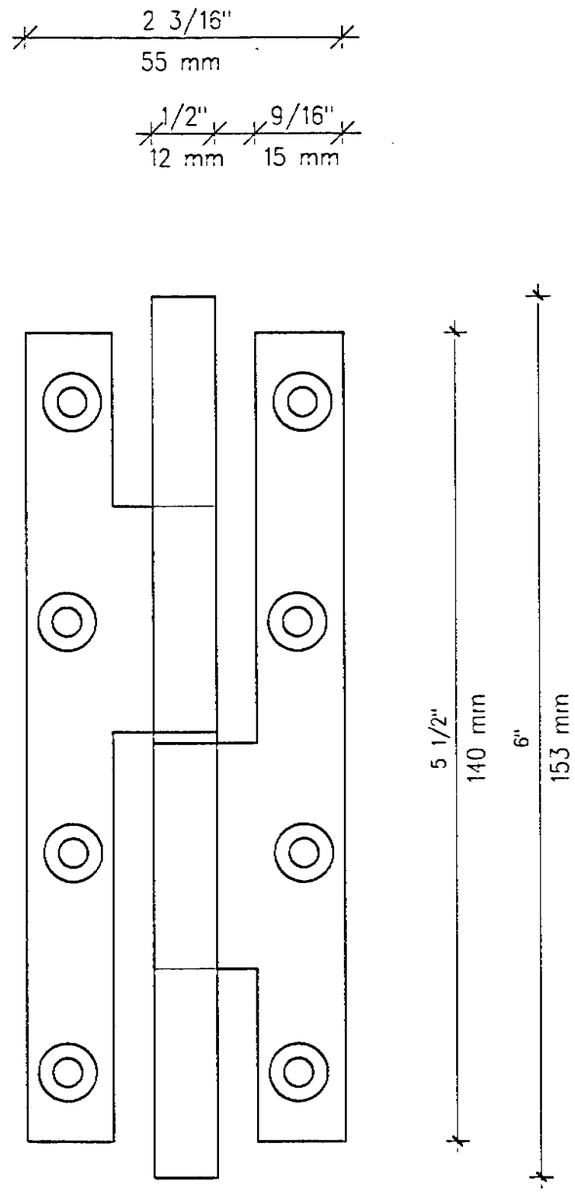


HINGES DOORS



HINGES WINDOWS

Solid brass hinges in oil-rubbed bronze finish.

PEETZ
WINDOWS AND DOORS

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HINGES
WINDOW & DOOR

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Document No.:	
Drawn by: A. SAUER	File: 51.s12
Date: 02/01/2006	Revised: 05/29/2006
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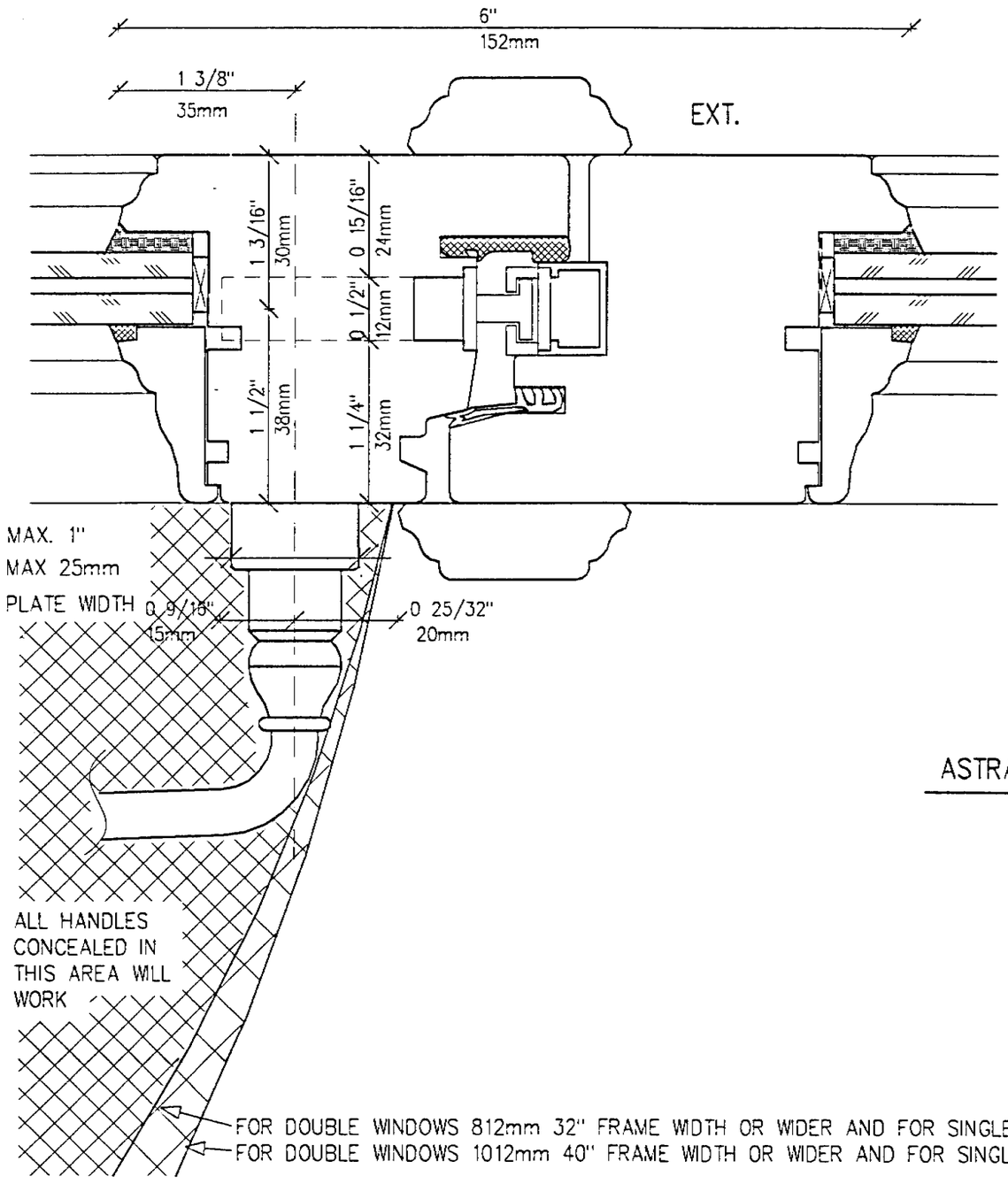
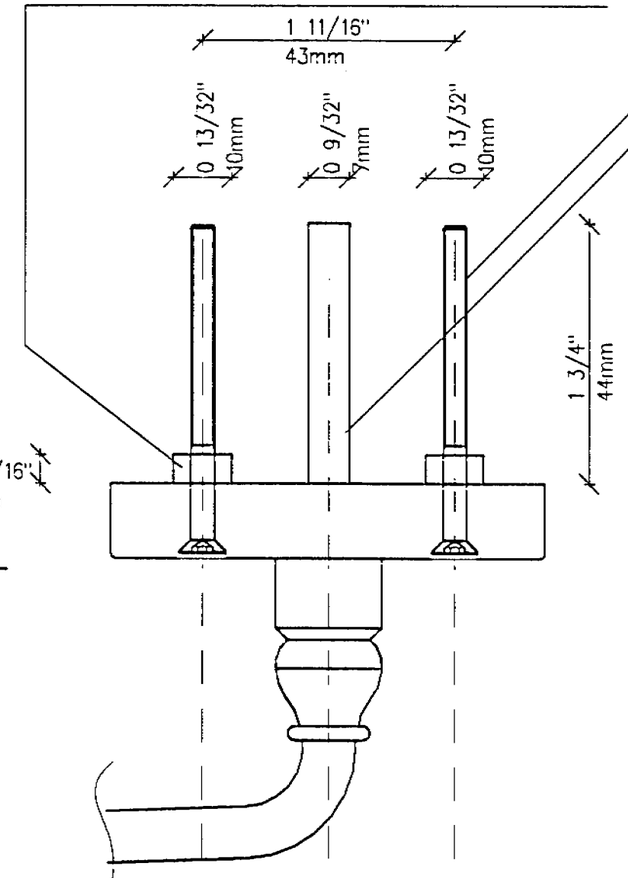


Spindle square 1/4" x 1/4" (7 x 7mm)
length spindle: 1 3/4" plus handle + plate embedments.

Screws M5 length 1 3/4" plus plate

Hole for Base pin ∅ 13/32" 10mm
3/16" 5mm deep, predrilled in sash



ASTRAGAL

FOR DOUBLE WINDOWS 812mm 32" FRAME WIDTH OR WIDER AND FOR SINGLE WINDOWS 450mm 17 3/4" FRAME WIDTH OR WIDER
FOR DOUBLE WINDOWS 1012mm 40" FRAME WIDTH OR WIDER AND FOR SINGLE WINDOWS 550mm 21 3/4" FRAME WIDTH OR WIDER

PEETZ
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MECHANISM DETAILS
OUTSWING CASEMENT

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Document No.:	
Drawn by: A. SAUER	File: 52.s12
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	Revised: 06/16/2006
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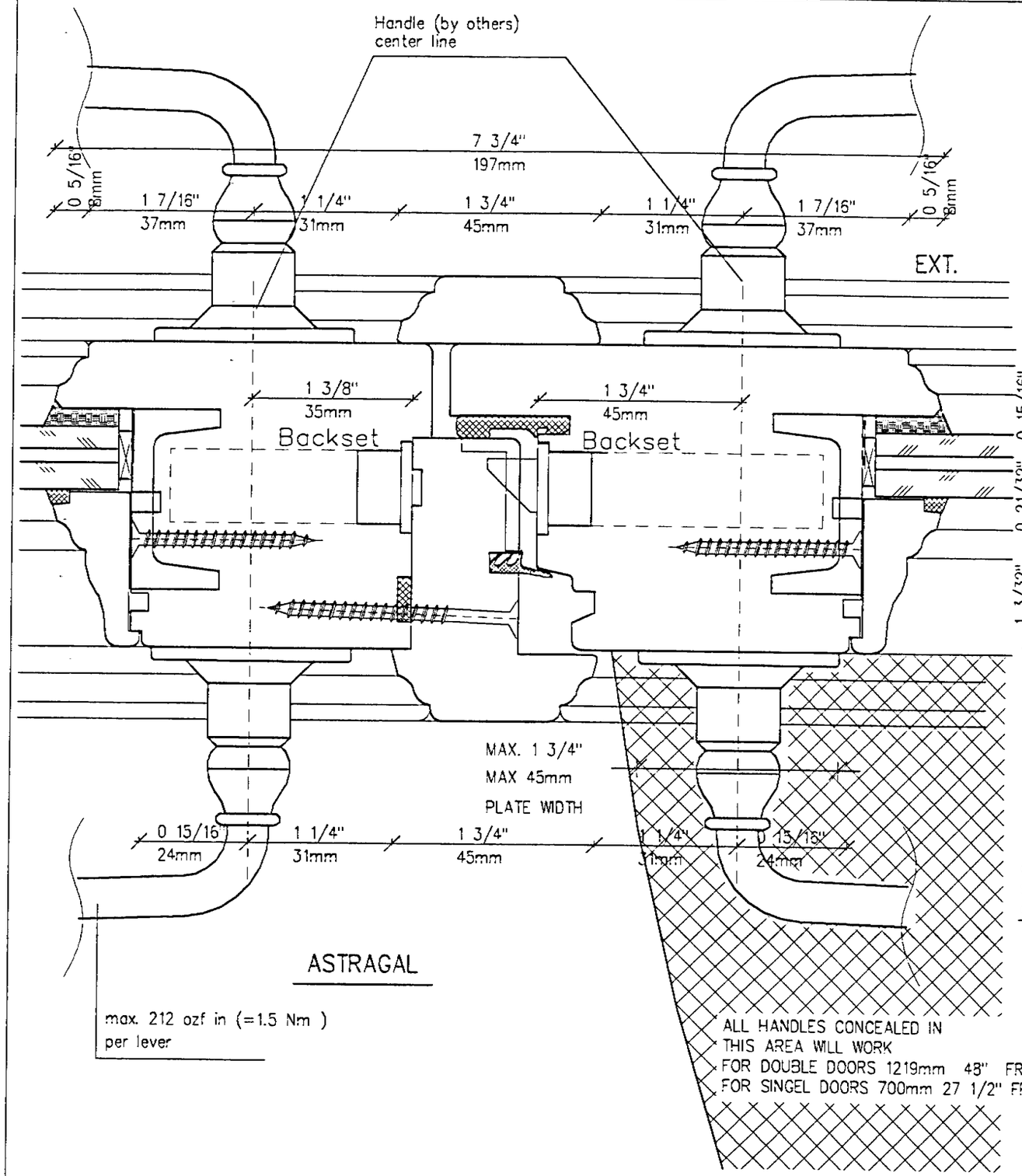

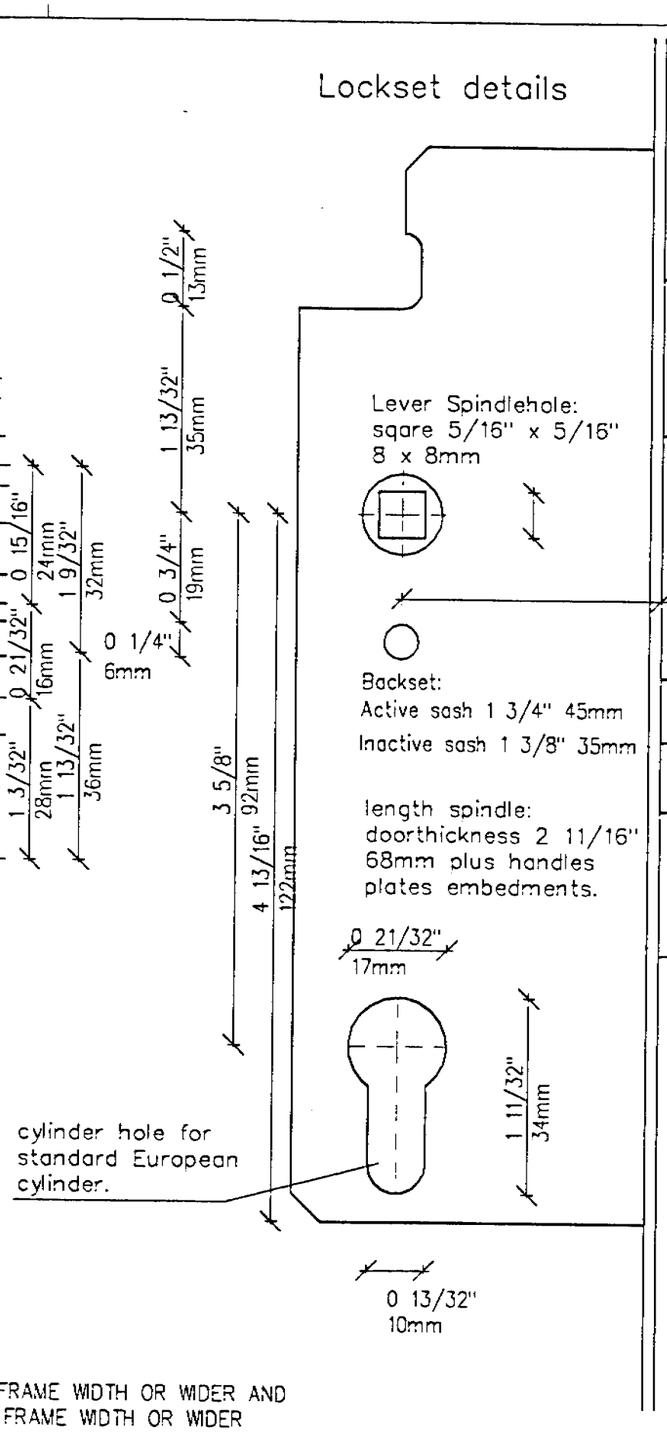

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MECHANISM DETAILS
OUTSWING DOOR

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Drawn by: A. SAUER	File: 53.s12	
Date: 02/01/2006	Revised: 05/29/2006	
	Revised: 05/16/2006	
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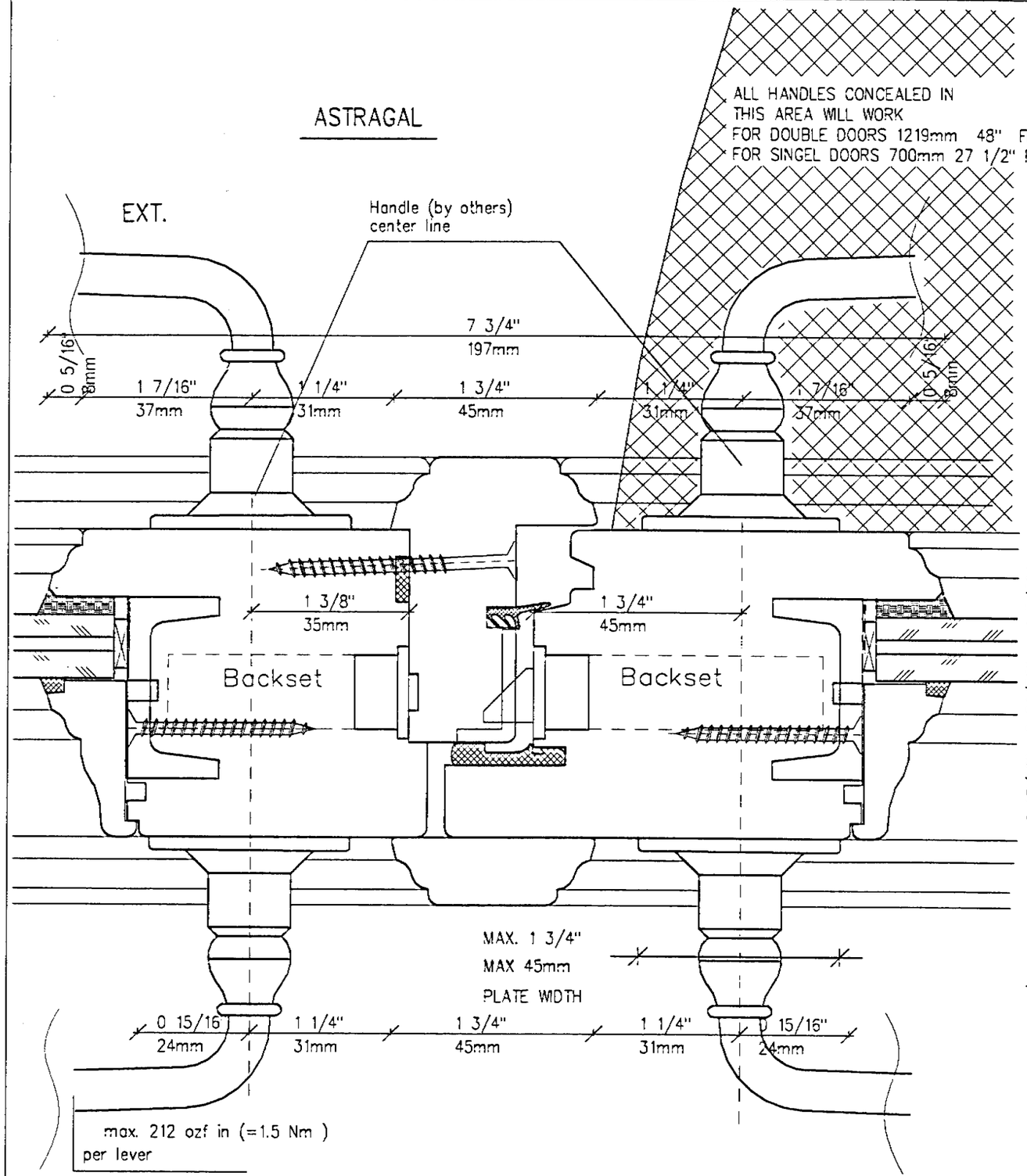
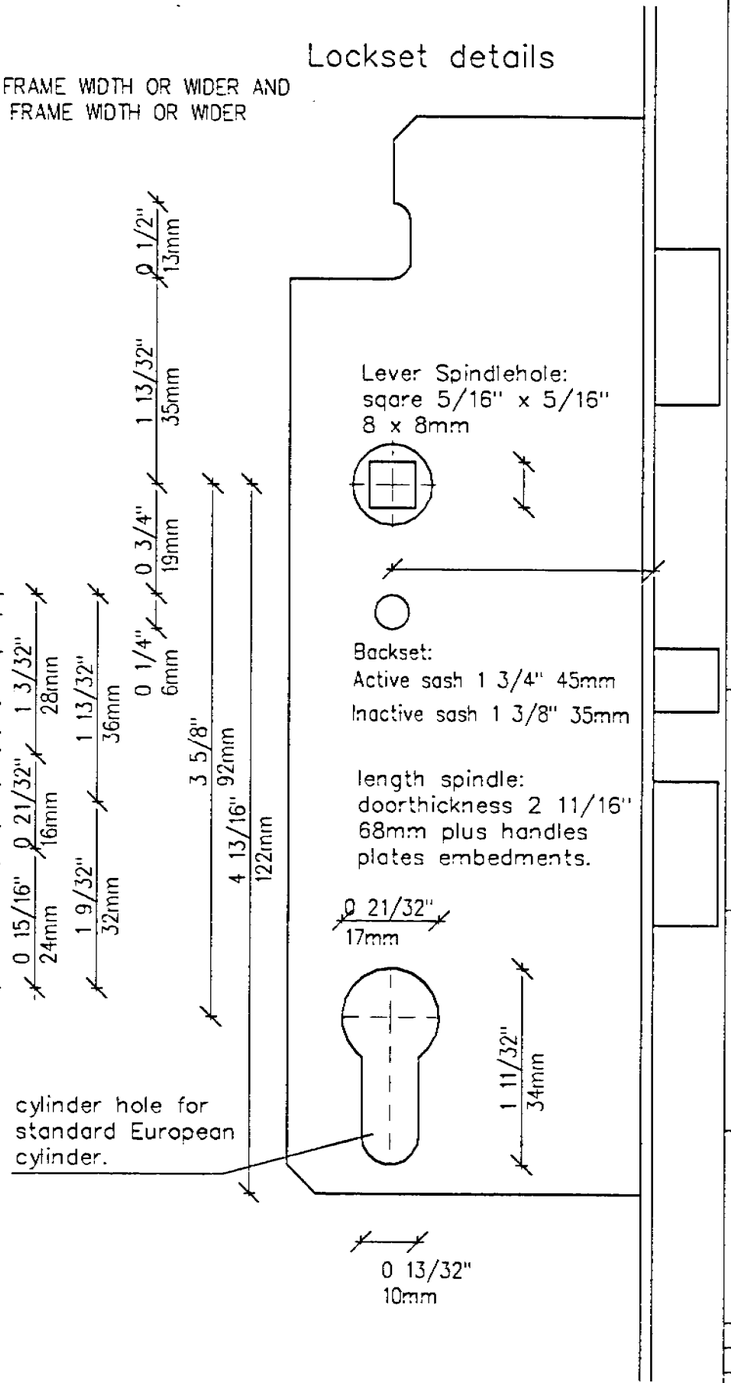
ASTRAGAL

ALL HANDLES CONCEALED IN
THIS AREA WILL WORK
FOR DOUBLE DOORS 1219mm 48" FRAME WIDTH OR WIDER AND
FOR SINGEL DOORS 700mm 27 1/2" FRAME WIDTH OR WIDER



Lockset details



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MECHANISM DETAILS
INSWING DOOR

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Miami Beach, Florida

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Drawn by: A. SAUER	File: 54.s12	
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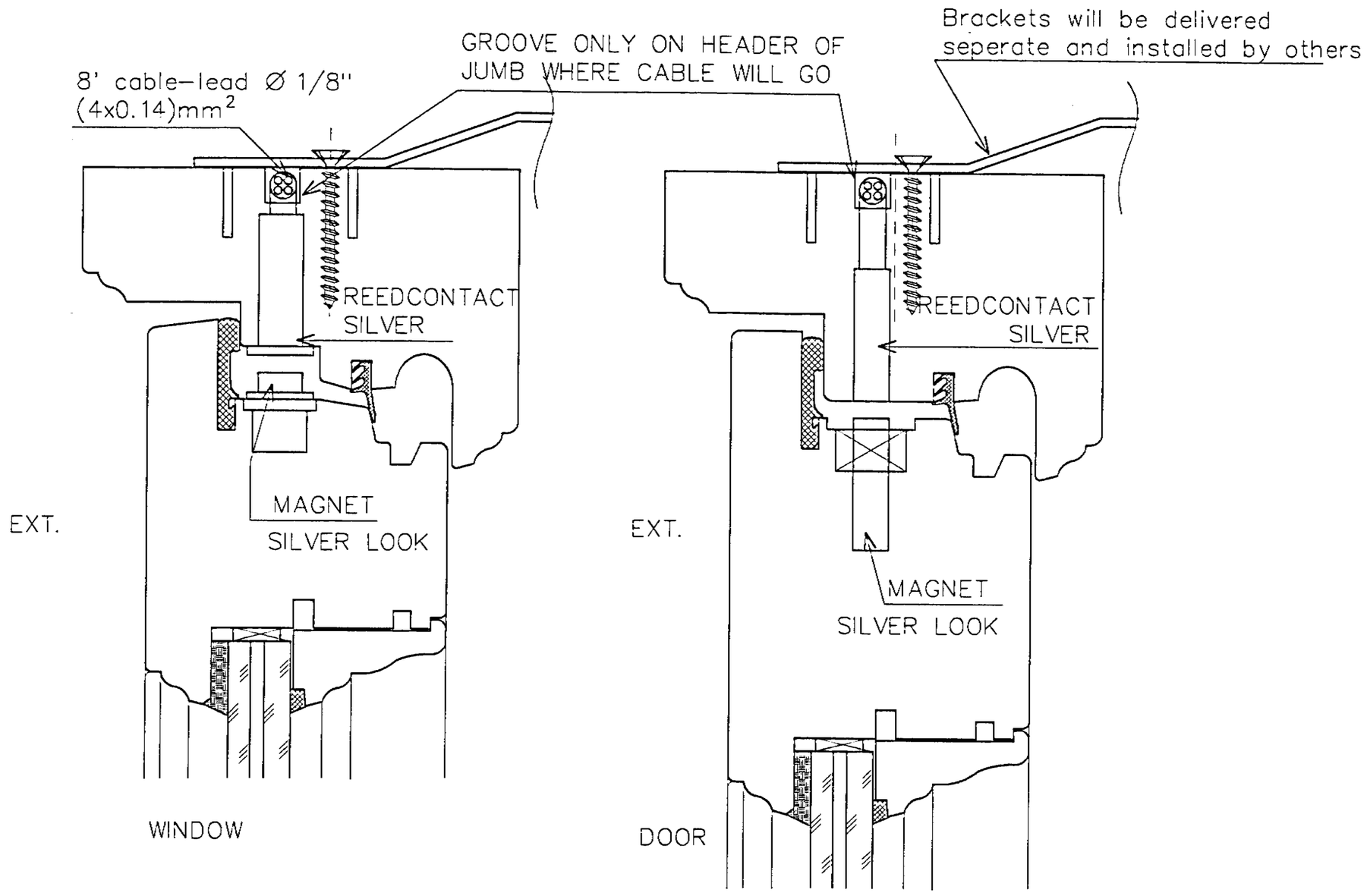
	Revised: 06/16/2006	
Scale: none	Page 54 of 55	



ALARM CONTACTS Factory provided and installed alarm contacts with 8' cable – lead. Cable will follow along the pre – grooved line on the exterior site. Cable will be fixed (from the interior viewsite) on the upper right corner of each operable unit, see sign ⟶ . The Reed – contact will be silver, diameter 5/16", and recessed in the jamb.

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ALARM CONTACTS
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Drawn by: A. SAUER	File: 55.s12
Date: 02/01/2006	Revised: 05/29/2006
	Revised: 06/16/2006
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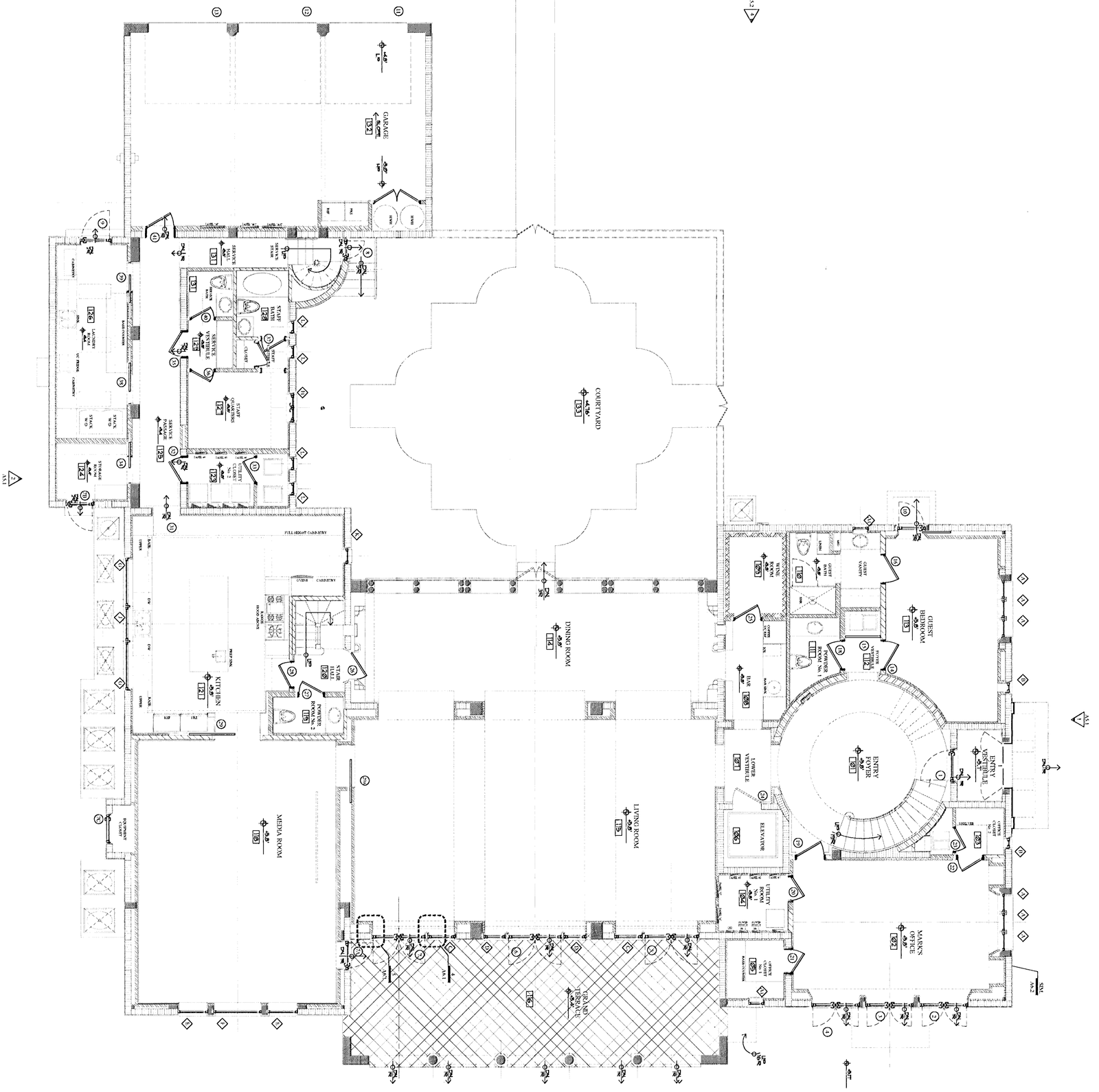


WINDOW & DOOR PERMIT SET 1.09.07

A4.1

A8.1

BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PROTECTION:
ENGINEERING:
POOL WORKS:
STRUCTURAL:
LANDSCAPE:

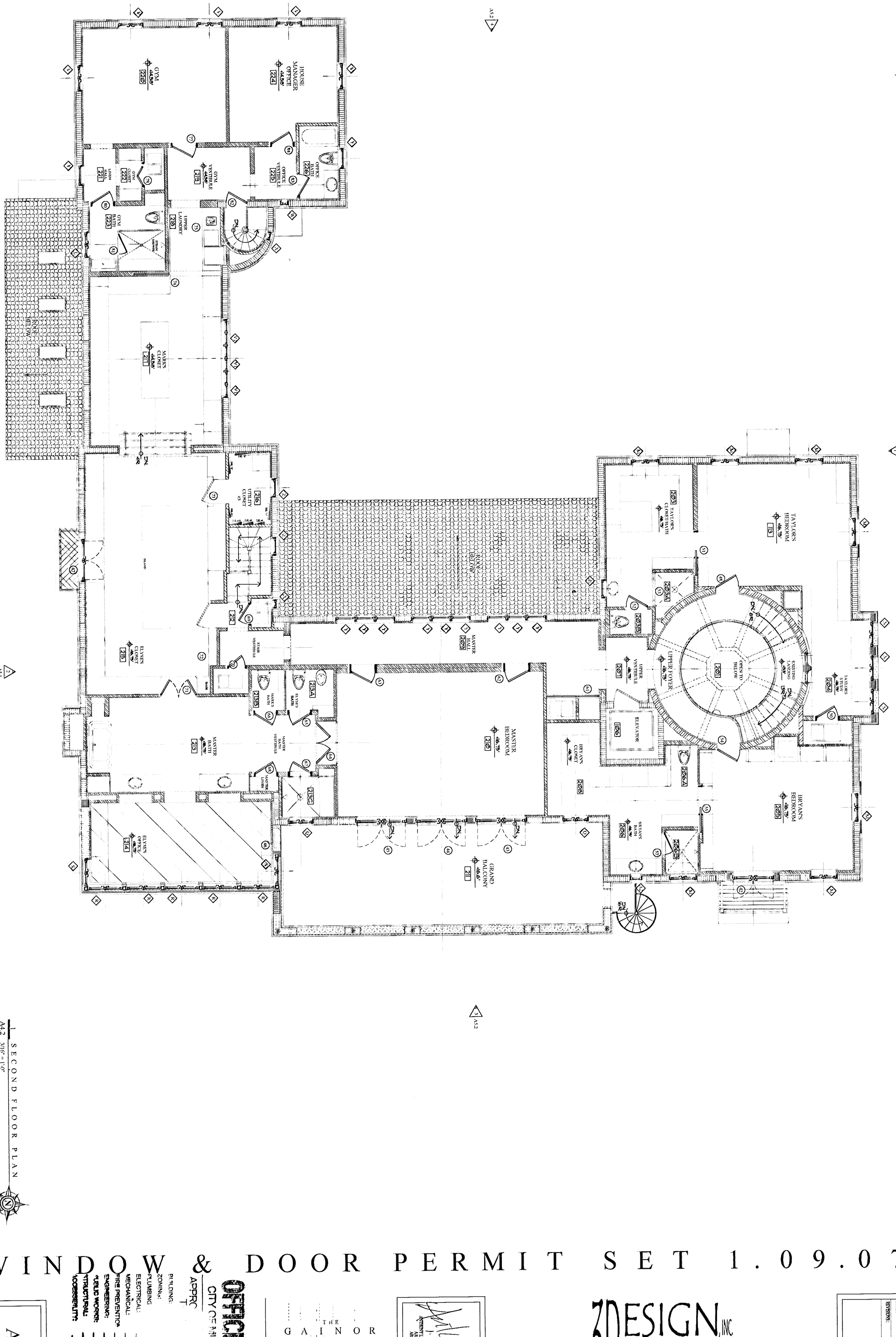
CONTRACTOR:
APPROVED:

THE GAINOR RESIDENCE
5806 NORTH BAY ROAD
MIAMI BEACH, FLORIDA

ANTHONY LEON, ARCHITECT
AR0016752
STATE OF FLORIDA

3DESIGN, INC.
ANTHONY LEON
AR0016752
ARCHITECTURE
1234 WASHINGTON AVE. SUITE#207 MIAMI BEACH, FL 33139 T.305.531.5208 F.305.531.4515

SHEET TITLE:	FIRST FLOOR PLAN
DRAWN:	DJP
DATE:	1.9.07
REVISIONS:	DATE



WINDOW & DOOR PERMIT SET 1.09.07

OFFICE COPY

CITY OF MIAMI BEACH

APPROVED

PERMIT BY

OWING:

BUILDING:

ZONING:

PLUMBING

ELECTRICAL:

MECHANICAL:

FIRE PREVENTION

ENGINEERING:

PUBLIC WORKS:

STRUCTURAL:

ACCESSIBILITY:

THE GAINOR RESIDENCE

5800 NORTH BAY ROAD

MIAMI BEACH, FLORIDA

ANTHONY LEON, ARCHITECT
AR0001569
STATE OF FLORIDA
SEAL

1.9.07

3DESIGN, INC.

ANTHONY LEON

AR0016752

ARCHITECTURE

1234 WASHINGTON AVE. SUITE#207 MIAMI BEACH, FL 33139 T.305.531.5208 F.305.531.4515

SHEET TITLE: SECOND FLOOR PLAN

DRAWN: EF

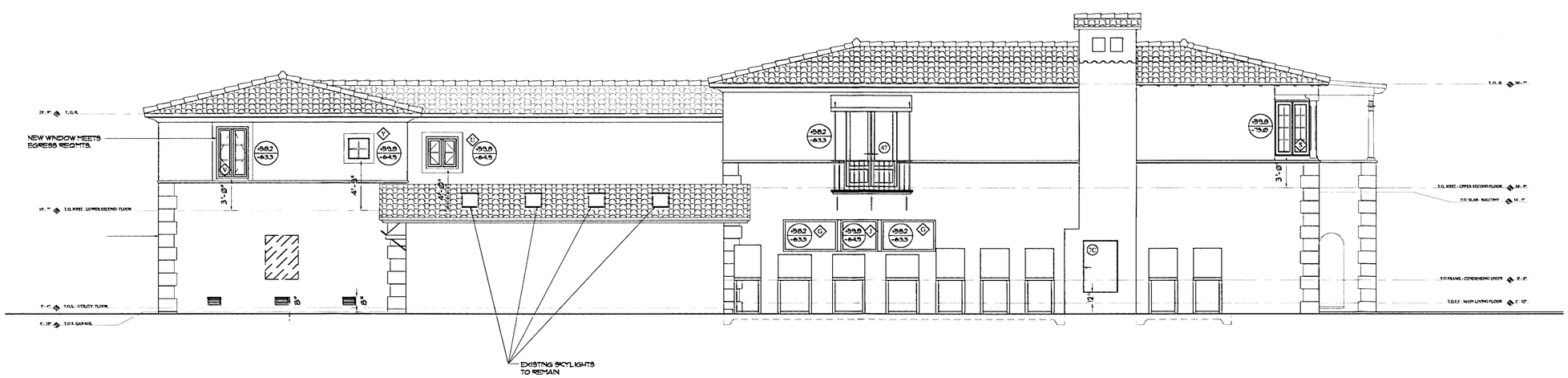
DATE: 1.9.07

REVISIONS: DATE:

A4.2

A8.1

1 SOUTH ELEVATION
A5.1 3/16" = 1'-0"



2 NORTH ELEVATION
A5.1 3/16" = 1'-0"

NOTE: ALL ELEVATIONS SHOWN ARE NGVD

WINDOW & DOOR PERMIT SET 1.09.07

SHEET TITLE:	ELEVATIONS
DRAWN:	BJP
DATE:	1.9.07
REVISIONS:	DATE:

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ARCHITECTURE
AA0001552
1234 WASHINGTON AVE. SUITE# 207 MIAMI BEACH, FL 33139 T. 305.531.5208 F. 305.531.4515

ANTHONY LEON, ARCHITECT
AA0001550
AR0014572
SEAL

THE GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI BEACH, FLORIDA

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CITY OF MIAMI BEACH
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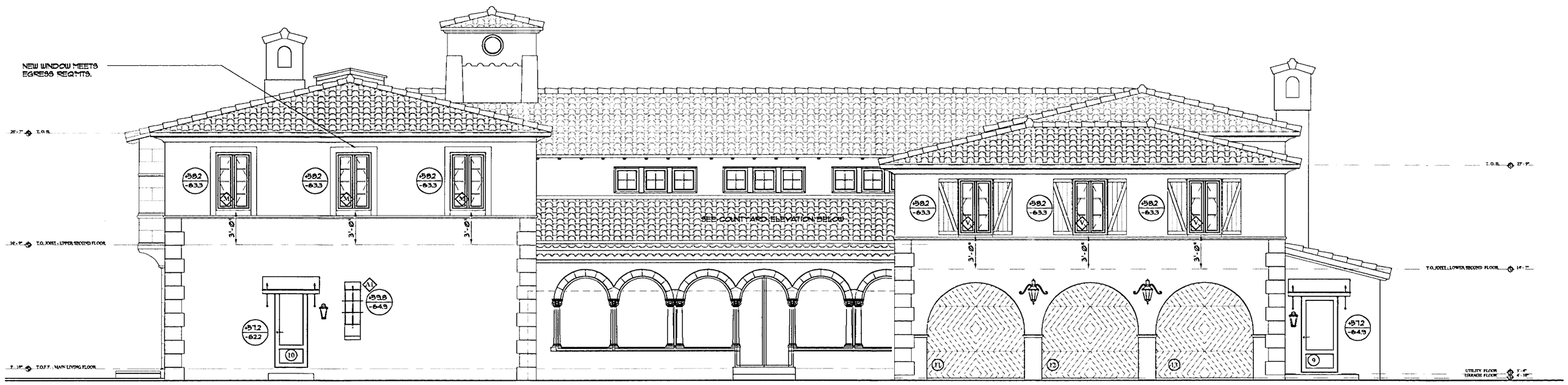
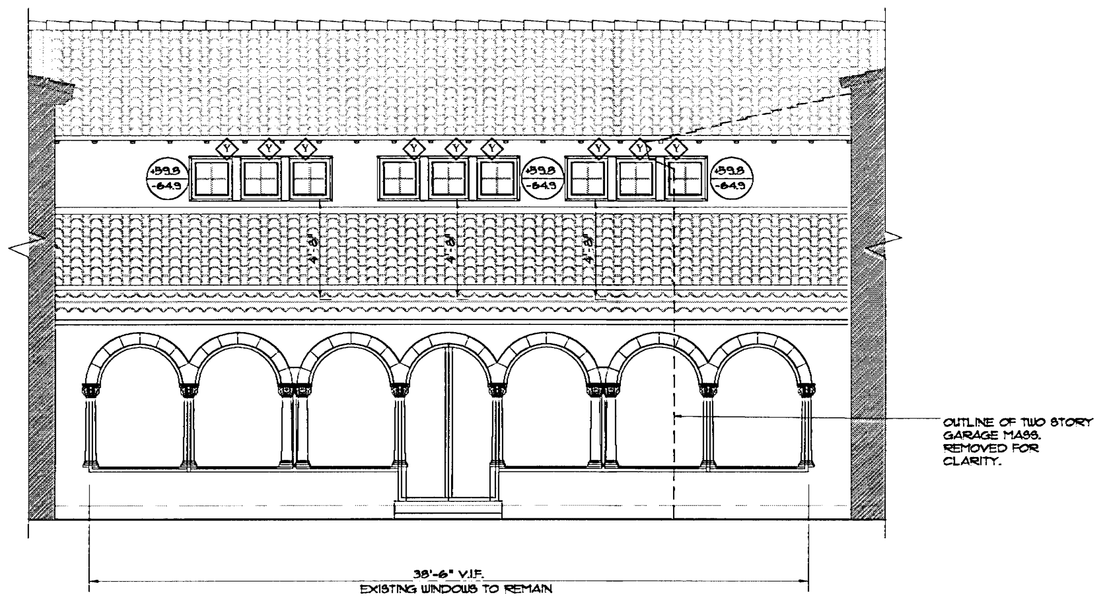
T...
BUILDING:
ZONING:
PLUMBING
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:

A5.1

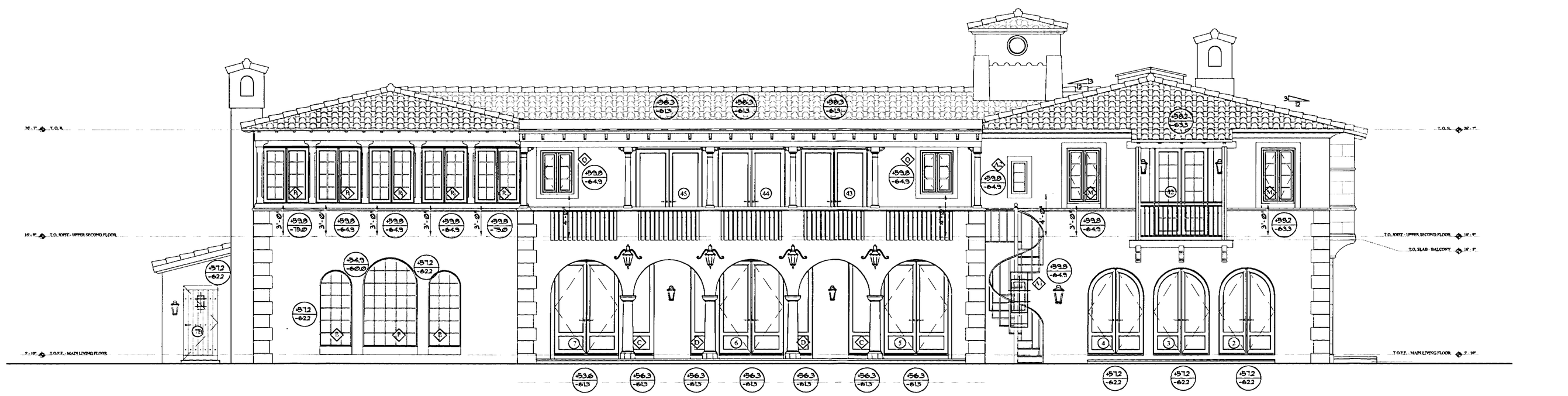
A8.1



3 EAST ELEVATION
A5.2 3/16" = 1'-0"



3A EAST COURTYARD ELEVATION
A5.2 3/16" = 1'-0"



4 WEST ELEVATION
A5.2 3/16" = 1'-0"

NOTE: ALL ELEVATIONS SHOWN ARE NGVD

WINDOW & DOOR PERMIT SET 1.09.07

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SHEET TITLE:	ELEVATIONS
DRAWN:	
DATE:	1.9.07
REVISIONS:	DATE:

ANTHONY LEON, ARCHITECT
AR0006752
SEAL

THE GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI BEACH, FLORIDA

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED
BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:

A5.2

A8.1

DOOR SCHEDULE

1. ALL ROUGH OPENINGS AND UNIT SIZES TO BE SITE VERIFIED BY THE GENERAL CONTRACTOR
2. PROVIDE SHOP DRAWINGS PRIOR TO FABRICATION TO ARCHITECT FOR APPROVAL
3. PROVIDE DOOR FINISH SAMPLE TO ARCHITECT PRIOR TO ANY WORK FOR APPROVAL
4. PROVIDE CASING FINISH SAMPLE TO ARCHITECT PRIOR TO ANY WORK FOR APPROVAL

No	LOCATION	SIZE: WIDTH	HEIGHT	THK	TYPE	MATERIAL	FINISH	HDW SET	CASING INTERIOR	EXTERIOR	MANUF	SIDELIGHTS QTY	W	H	THK	TRANSOMS	TRANSOMS	COMMENTS
EXTERIOR - FIRST FLOOR																		
1	Entry Vestibule	42"	82"	1 3/4"	A	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / In-swing
2	Mark's Office	(2) 31.5"	91.5" top of arch	1 3/4"	B	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing / Arch top
3	Mark's Office	(2) 31.5"	91.5" top of arch	1 3/4"	B	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing / Arch top
4	Mark's Office	(2) 31.5"	91.5" top of arch	1 3/4"	B	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing / Arch top
5	Living Room	(2) 36"	104"	1 3/4"	C	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	1	26"	104"	1 3/4"	-	-	Impact resistant / Out-swing / Fixed Sidelights
6	Living Room	(2) 36"	104"	1 3/4"	C	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	2	26"	104"	1 3/4"	-	-	Impact resistant / Out-swing / Fixed Sidelights
7	Living Room	(2) 36"	104"	1 3/4"	C	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	1	26"	104"	1 3/4"	-	-	Impact resistant / Out-swing / Fixed Sidelights
7A	Breakfast Room	36"	92.625"	1 3/4"	D1	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing / Archtop
7B	Laundry Room	36"	82"	1 3/4"	J	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing
7C	Equipment Closet	36"	60"	1 3/4"	-	STEEL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	N/A	STUCCO RETURN	TBD	-				-	-	Weatherproof / Out-swing
8	Service Stair	26"	82"	1 3/4"	D	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing
9	Laundry Room	40"	92"	1 3/4"	D	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing
10	Guest Bedroom	36"	84"	1 3/4"	D	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	PEETZ	-				-	-	Impact resistant / Out-swing
GARAGE DOORS																		
11	New Garage #1	102" v.i.f.	100" top of arch	2 1/4"	M	SOLID WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	3	MATCH EXISTING	STUCCO RETURN	DESIGNER DOORS	-				-	-	Impact resistant / Overhead
12	New Garage #2	102" v.i.f.	100" top of arch	2 1/4"	M	SOLID WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	3	MATCH EXISTING	STUCCO RETURN	DESIGNER DOORS	-				-	-	Impact resistant / Overhead
13	New Garage #3	102" v.i.f.	100" top of arch	2 1/4"	M	SOLID WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	3	MATCH EXISTING	STUCCO RETURN	DESIGNER DOORS	-				-	-	Impact resistant / Overhead
INTERIOR - FIRST FLOOR																		
14	Guest Bedroom	36"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
15	AHU Closet #1	48"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size
16	Guest Bathroom	36"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
18	Powder Room #1	36"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
19	Mark's Office	36"	87"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
20	Utility Closet #1	30"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
21	Office Closet #1	30"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
22	Office Closet #2	32"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size
23	AHU Closet #2	30"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
24	Elevator	36"	90"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Elevator door
25	Wine Room	34.5"	96"	1 3/4"	D	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
26	Dining Room	36"	88"	1 3/4"	K	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Archtop
27	Powder Room #2	32"	84"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
28	Kitchen	32"	84"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	(2) 5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
29	Family Room	(2) 30"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Double Swing
29A	Family Room	(2) 30"	96"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	7	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket/ Electric doors
30	Pantry	32"	84"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
31	Kitchen	34" v.i.f.	84"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
32	Mechanical Room	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size
33	AHU Closet #3	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
34	Walk-in Refridgerator	32"	92"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket
35	Service Quarters	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size
36	Service Closet	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
37	Service Bath	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
38	Laundry Room	36"	92"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	7	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket
39	Laundry Room	36"	92"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	7	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket
40	Staff Bath	32"	80"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
41	Garage	36"	84"	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / (2) hr. Rated
EXTERIOR - SECOND FLOOR																		
42	Bryan's Bedroom	(2) 30"	97.625"	1 3/4"	E	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / Out-Swing
43	Master Bedroom	(2) 37.5"	96"	1 3/4"	F	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / Out-Swing
44	Master Bedroom	(2) 37.5"	96"	1 3/4"	F	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / Out-Swing
45	Master Bedroom	(2) 37.5"	96"	1 3/4"	F	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	2	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / Out-Swing
46	Master Sitting	34"	98.625"	1 3/4"	G	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / Out-Swing
47	Master Closet	(2) 30"	96"	1 3/4"	F	WD/ALUM/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	1	MATCH EXISTING	STUCCO RETURN	T.B.D.	-				-	-	Impact resistant / In-Swing
INTERIOR - SECOND FLOOR																		
48	Taylor's Bedroom	32"	84'	1 3/4"	K	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Archtop
50	AHU Closet #4	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
51	Taylor's Closet	39"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket
52	Taylor's Private Bath	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
53	Taylor's Shower	32"	96'	1/2"	L	GLASS	AS PER ARCH. APPROVED - MANUF. SAMPLE	9	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing, Glass Shower Door
54	Bryan's Bedroom	32"	84'	1 3/4"	K	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Archtop
55	Byran's Bathroom	40"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket
57	Bryan's Shower	32"	96'	1/2"	L	GLASS	AS PER ARCH. APPROVED - MANUF. SAMPLE	9	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing, Glass Shower door
60	Elevator	36"	80'	1 3/4"	H	WD/GL	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing, Elevator Door
61	AHU Closet #5	80"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	9	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	By-pass Slider
62	Master Bedroom	36"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
63	Master Bedroom	36"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
64	Master Bath	(2) 30" v.i.f.	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	(2) 4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Double Swing
65	Mark's Bath	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
66	Elysa's Bath	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
67	Master Shower	32"	96'	1/2"	L	GLASS	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing, Glass Shower Door
68	Master Linen Closet	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
69	AHU #6 Closet	30" v.i.f.	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
70	AHU #7 Closet	30"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	9	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
71	Master Closet	(2) 30"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
72	Master Closet	36"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size / Self-closing
73	Electrical Room #2	36"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing / Refer to Millwork drawing for exact size
74	Mark's Closet	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Pocket / Self-closing
77	Gym	36"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
78	AHU #8 Closet	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Bi-fold
80	Gym Bath	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
81	Gym Shower	32"	84"	1/2"	L	GLASS	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing , Glass Shower Door
82	Service Stair	30" v.i.f.	79' v.i.f.	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
83	Office Bath	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	5	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing
84	House Manager	32"	96'	1 3/4"	H	WOOD	AS PER ARCH. APPROVED - MANUF. SAMPLE	4	MATCH EXISTING	MATCH EXISTING	CUSTOM	-				-	-	Swing

WINDOW SCHEDULE

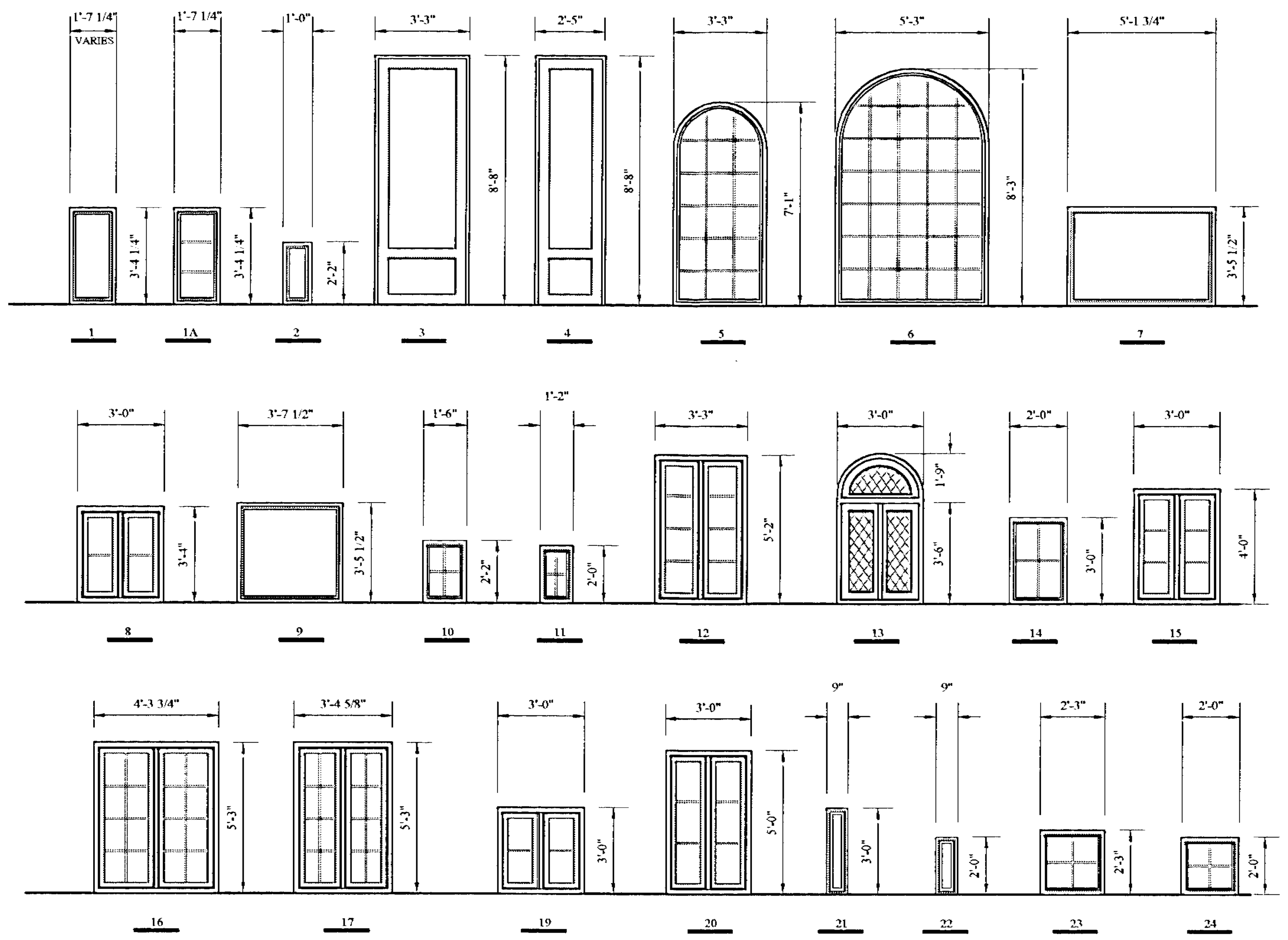
1. ALL ROUGH OPENINGS AND UNIT SIZES TO BE SITE VERIFIED BY THE GENERAL CONTRACTOR
2. PROVIDE SHOP DRAWINGS PRIOR TO FABRICATION TO ARCHITECT FOR APPROVAL
3. PROVIDE DOOR FINISH SAMPLE TO ARCHITECT PRIOR TO ANY WORK FOR APPROVAL
4. PROVIDE CASING FINISH SAMPLE TO ARCHITECT PRIOR TO ANY WORK FOR APPROVAL

MARK	QUANTITY	SIZE: WIDTH	HEIGHT	TYPE	MATERIAL	MANUF	CASING INTERIOR	EXTERIOR	DETAILS: JAMB	HEAD	THRESH	COMMENTS
A	6	22.625"	40.25"	1	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
A1	2	19.25"	40.25"	1	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
A2	1	19.25"	40.25"	1A	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
B	2	12"	26"	2	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
C	2	36"	104"	3	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed / Sidelight
D	2	29"	104"	4	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed / Sidelight
E	2	[illegible]	85"	5	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed / Arch top
F	1	63"	99"	6	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed / Arch top
G	2	61.75"	41.5"	7	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
H	1	36"	40"	8	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
J	1	43.5"	41.5"	9	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
K	1	18"	26"	10	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
L	4	14"	24"	11	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
M	7	39"	62"	12	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
N	3	38"	63"	13	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing/ Archtop
P	1	24"	36"	14	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
Q	2	36"	48"	15	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
R	5	51.75"	63"	16	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
S	2	40.625"	63"	17	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
U	1	36"	36"	19	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
V	5	36"	60"	20	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Casement-outswing
V1	3	36"	60"	20	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
W	1	9"	36"	21	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
X	2	9"	24"	22	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
Y	11	27"	27"	23	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed
Z	2	24"	24"	24	Wood/Aluminum clad	PEETZ WINDOWS	MATCH EXISTING	STUCCO RETURN	T.B.D.	T.B.D.	T.B.D.	Impact resistant / Fixed

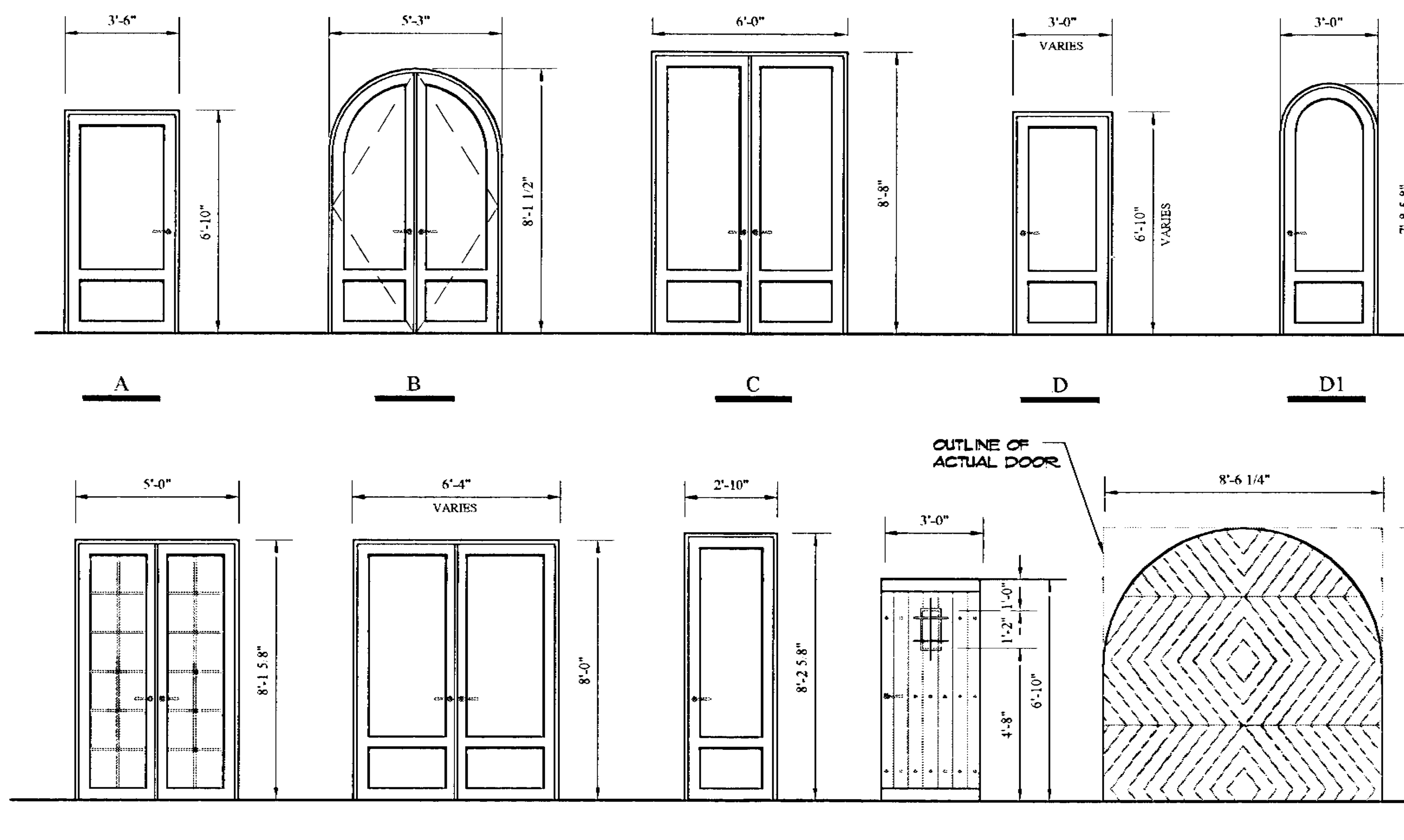
HARDWARE SETS

NOTE: INSTALL ALL FINISH HARDWARE IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. UPON COMPLETION OF INSTALLATION, ADJUST ALL HARDWARE AS REQUIRED TO ASSURE SMOOTH AND PROPER OPERATION

No	DESCRIPTION
1	(3) 4X4 BUTT HINGES
	(1) ENTRY SET W/ LOCK
	(1) INDIVIDUAL DEAD BOLT
2	(6) MANUFACTURER HINGES
	(1) ENTRY SET W/ LOCK
	(1) INDIVIDUAL DEAD BOLT
	(1) DUMMY SET
	(2) MANUFACTURER FLUSH BOLTS
3	(1) OVERHEAD SUSPENDED ROLL UP W/ MOTOR LIGHT & LOCK
4	(3) BUTT HINGES
	(1) PRIVACY SET
5	(3) MANUFACTURER BUTT HINGES
	(1) PASSAGE SET
6	(1) POCKET SUSPENDED TRACK
	(2) PRIVACY PULLS W/ LOCK
	(1) FLUSH PULL
7	(1) POCKET SUSPENDED TRACK
	(1) FLUSH PULL
8	(1) TOP & BOTTOM SLIDER TRACK
	(2) PULLS

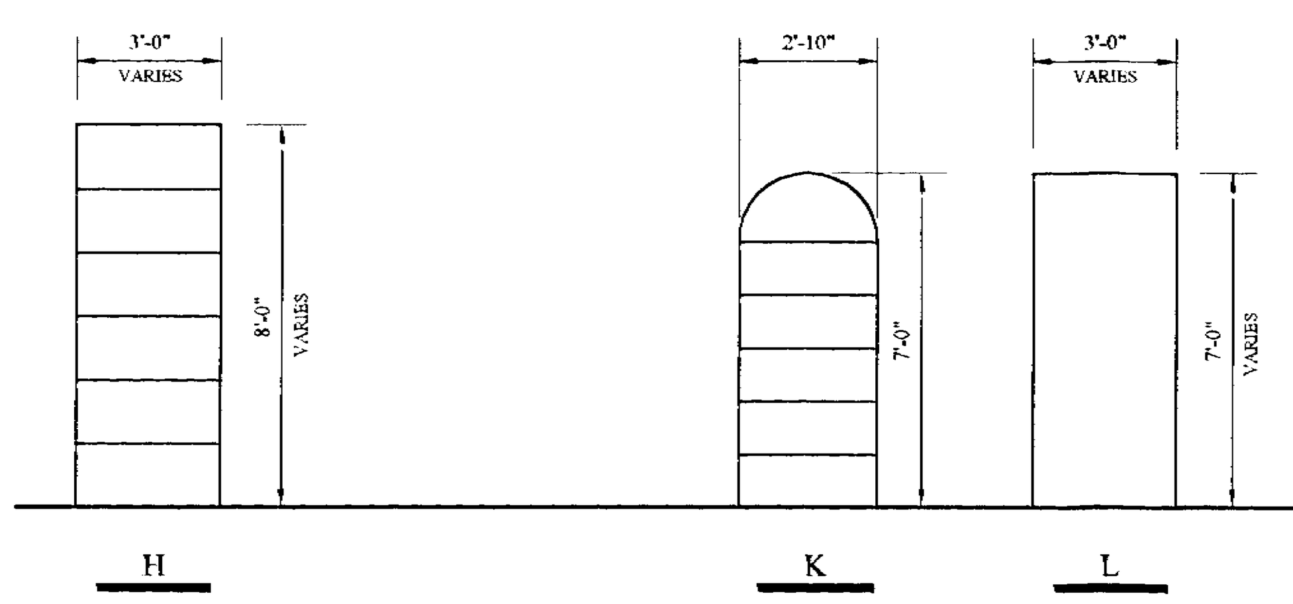


1 WINDOW TYPES
A8.1



M GARAGE DOOR

2 EXTERIOR DOOR TYPES
A8.1



3 INTERIOR DOOR TYPES
A8.1

SHEET NOTES

1. GLAZING IN SLIDING AND SWINGING DOORS AND IN SHOWER TO TUB ENCLOSURES INCLUDING ANY GLAZING W/ IN 60" A.F.F SURFACE IN WALLS SURROUNDING ANY TUB OR SHOWER ENCLOSURE, SHALL BE FRAMELESS W/ 1/4" TEMPERED, CLASS II SAFETY GLAZING. ALL ENCLOSURES TO HAVE PIVOT HINGES AND TRANSPARENT WATER BLOCKS.
2. CLOSET DOOR LATCH SHALL BE SUCH THAT CHILDREN CAN OPEN THE DOOR FROM INSIDE THE CLOSET (TYP.)
3. BATHROOM DOORS DESIGNED TO ALLOW OPENING FROM THE OUTSIDE DURING AN EMERGENCY WHEN LOCKED (TYP.)
4. SECONDARY MEANS OF EGRESS AS PER N.F.P.A. 21-2.2.3.
5. ALL INTERIOR DOORS TO BE SOLID, STAIN GRADE WOOD, PRE HUNG AS SPECIFIED IN SCHEDULES. WOOD SPECIES AND STAIN COLOR AS SELECTED BY OWNER.

WINDOW & DOOR PERMIT SET 1.09.07

SHEET TITLE:	DOOR & WINDOW SCHEDULE/ NOTES
DRAWN:	BJF
DATE:	1.9.07
REVISIONS:	DATE:

3DESIGN, INC
ANTHONY LEON
AR0016752
ARCHITECTURE
1234 WASHINGTON AVE. SUITE#207 MIAMI BEACH, FL 33139 T.305.531.5208 F.305.531.4515

ANTHONY LEON, ARCHITECT
AA0003569
AR0016752
SEAL

THE GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI BEACH, FLORIDA

OFFICE COPY
CITY OF MIAMI BEACH
APPROV... PERMIT ... TH... ...NG:
BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:

A8.1

A8.1

B07013600

5800 N Bay Rd.

CITY OF MIAMI BEACH
BUILDING DEPARTMENT
1700 CONVENTION CENTER DRIVE
2ND FLOOR - CITY HALL
MIAMI BEACH, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I, (we) have been retained by: 3Design Inc. to perform special inspector services under the Florida Building Code at the 5800 N BAY RD project on the below listed structures as of 1,10,08 (date). I am a professional engineer licensed in the State of Florida.

Process Number: _____ **Master Permit (IF APPLICABLE):** _____

- ○ Special Inspector for Pilings, FBC 1822.1.20
- ○ Special Inspector for Soil Compaction, FBC 1820.3.1
- ○ Special Inspector for Precast Attachments, FBC 1927.12.2 (By P.E. or R.A.)
- ● Special Inspector for Reinforced Masonry, FBC 2122.4
- ○ Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A.)
- ○ Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R.A.)
- ○ Special Inspector for _____

NOTE: Only the marked boxes apply.

The following individual's) employed by this firm or me are authorized representatives to perform inspections:

1. SAM. LYUBKIN 2. _____
3. _____ 4. _____

***NOTE: FBC 2001 HVZ sections 1927.12.2, 2218.2, 2319.17.4.2 requires either a Registered professional Engineer or Registered Architect to perform the actual inspections.**

I, (we) will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, (we) understand that a Special Inspection Log for each building must be displayed in a convenient location on the site for reference by the City of Miami Beach Building Department Inspector. All mandatory inspections, as required of the Florida Building Code, Inspection performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. Further, upon completion of the work under each building permit, I will submit to the Building Inspector at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.

Signed and Sealed
34306 FL.
License Number

Date: 1-10-2008

Architect/Engineer Signature: _____
Architect/Engineer Name Printed: SAM. LYUBKIN, P.E.
Address: 2632 Hollywood Blvd. #103
Phone Number: (954) 922-8845
Owner/Agent Signature: _____
Owner/Agent Name Printed: _____
Building Department Accepted By: _____ 01/14/08

In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the Public Records of this County, and there may be additional permits required from other governmental entities such as water management's districts, state agencies, or federal agencies.

The City of Miami Beach assumes no responsibility for accuracy of or results from these plans which are approved subject to compliance with all Federal, State, and Local Laws, Rules, and Regulations.

GEOTECHNICAL
ENVIRONMENTAL
HYDROGEOLOGY
ASBESTOS

TESTING LABORATORIES
DRILLING SERVICES
INSPECTION SERVICES
ROOFING

DYNATECH ENGINEERING CORP.

Miami, April 5, 2005

Mr. Thomas F. Weber
THOMAS WEBER INC.
KEY EXECUTIVE BUILDING
104th Crandon Boulevard
Key Biscayne, FL 33149

Re: Gainor Residence @
5800 North Bay Road
Miami Beach, FL

Dear Mr. Weber:

Pursuant to your request; DYNATECH ENGINEERING CORP., D.E.C. completed a Preliminary Subsoil Investigation on April 5, 2005 at the above referenced project. The purpose of our investigation was to verify subsoil conditions relative to foundation design of the proposed additions.

A total of (2) standard penetration boring tests were performed according to ASTM-D 1586 down to an average depth of 35' below existing ground surface.

The following graph was developed as a general condition for the subject site: (Refer to field boring logs for exact locations and soil description):

Depth From	To	Description
0'- 0"	1'- 0"	Topsoil and grass
1'- 0"	4'- 0"	Silty beach sand
4'-0"	5'-0"	Peat
5'-0"	15'-0"	Gray silty beach sand
15'-0"	18'-0"	Tan medium sand with rock fragment
18'-0"	35'-0"	Tan sandy lime rock

Groundwater table elevation was measured immediately at the completion of each boring and was found at an average depth of 5' below existing ground surface. Fluctuation in water level should be anticipated due to seasonal variations and run off as well as varying ground elevations construction dewatering pumping activities in the area.

750 West 84 Street, Hialeah, FL 33014-3618 • Phone (305) 828-7499 • Fax (305) 828-9598

Based on our understanding of the proposed structure and our field boring logs; it is evident that the deep foundation system are needed to support the proposed addition without detrimental settlement to the structure.

Deep foundation systems shall consist of one of the following alternatives:

Alternatives Pile Foundation	Approximate Pile Depth	Size	Pile Capacity in Tons Compression	Pile Capacity in Tons Tension	Allowable Lateral Capacity in Tons
Pin Piles	To Refusal	3 Inch	5 Tons	2 Tons	1 Ton
Pin Piles	To Refusal	4 Inch	8 Tons	3 Tons	1 Ton
Type A of B Helical Piles	25' BLS	4 Inch	10 Tons	2 Tons	1 Ton
Auger Cast Piles	25' BLS	12 Inch	25 Tons	7 Tons	2 Tons
Auger Cast Piles	25' BLS	14 Inch	35 Tons	10 Tons	4 Tons
Precast Concrete Piles	25' BLS	10"x 10"	17 Tons	5 Tons	1 Tons
Precast Concrete Piles	25' BLS	12"x 12"	25 Tons	7 Tons	2 Tons
Precast Concrete Piles	25' BLS	14"x 14"	35 Tons	10 Tons	4 Tons

BLS: Below Existing Land Surface

Estimated Lateral Load for a pile Top Deflection of ¼ inch. The proposed pile length is based on the existing ground elevation at the time of drilling. Pile length may vary depending on proposed grade beam elevation and soil profile.

In the case of the Pin, Helical, or Precast piles a minimum of 4 piles shall be driven to determine production pile length. All work shall be performed in accordance with the applicable building code.

In case of existing structures in the vicinity of the pile driving operation, care shall be taken not to create excessive vibration. Vibration levels shall be monitored to verify compliance with county regulations. Steps must be taken to prevent excessive vibrations. The minimum center to center of piles or adjacent foundations shall be not less than twice the average diameter for round piles or 1 – ¾ times the diagonal dimensions of rectangular piles, but in no case less than 30 inches.

Regardless of the thoroughness of a geotechnical exploration there is always the possibility that conditions may be different from those of the test locations; therefore, DYNATECH ENGINEERING CORP., does not guarantee any subsoil conditions between the bore test holes. The data from the soil boring is for foundation analysis only. It is not to be used for excavating or back filling estimates. As a mutual protection to clients, the public and ourselves, all reports are submitted as the confidential property of clients, and authorization for publication of statements, conclusions or extracts from or regarding our reports is reserved pending our written approval. The discovery of any site or subsurface conditions during construction which deviate from the information obtained from our subsoil investigation is always likely and should be reported to us for our evaluation.

It has been a pleasure working with you and look forward to do so in the near future. Please feel free to contact us if we may be of further service to you.

Sincerely yours,

Wissam Naamani, P.E.
DYNATECH ENGINEERING CORP.
Florida Reg. No. 39584
Special Inspector No. 757
WN/sk

DYNATECH ENGINEERING CORP.

750 WEST 84TH STREET
HIALEAH, FL 33014
(305) 828-7499

TEST BORING REPORT

CLIENT : Thomas Weber, Inc.
PROJECT : Gainor Residence @
ADDRESS : 5800 North Bay Road, Miami Beach, FL.
LOCATION : See attached plan

DATE: 04-04-05
HOLE NO.: B-1
DRILLER: ASLD

DEPTH	DESCRIPTION OF MATERIALS	SAMPLE NO.	HAMMER BLOWS ON SAMPLER	"N"
1		2	Hand	H
2				
3	TOPSOIL AND ROOTS AND	4	Hand	H
4	0'- 0" to 1'-0" GRASS			
5		6	2 1	3
6	1'-0" to 2'- 0" TAN BEACH SAND		2 1	
7		8	2 2	4
8	2'-0" to 4'-0"		2 2	
9	GRAY SILTY BEACH SAND	10	2 3	5
10			2 1	
11	4'-0" to 5'-0" PEAT	12	1 2	4
12			2 1	
13	5'-0" to 15'-0" GRAY SILTY BEACH SAND	14	2 2	4
14			2 2	
15	TAN MEDIUM SAND WITH	16	A	A
16	15'-0" to 18'-0" ROCK FRAGMENT			
17		18	A	A
18	18'-0" to 35'-0"			
19	TAN SANDY LIME ROCK	20	A	A
20				
21		22	18 22	43
22			21 17	
23		24	A	A
24				
25		26	A	A
26				
27		28	A	A
28				
29		30	20 23	48
30			25 27	
31		32	A	A
32				
33		34	A	A
34				
35		36	A	A
36				
37		38		
38				

Water Level: 5' **Below Surface**

As a mutual protection to clients, the public and ourselves, all reports are submitted as the confidential property of clients, and authorization for publication of statement conclusions or extracts from or regarding our reports is reserved pending on our written approval.

DYNATECH ENGINEERING CORP.

750 WEST 84TH STREET
HIALEAH, FL 33014
(305) 828-7499

TEST BORING REPORT

CLIENT : Thomas Weber, Inc.
PROJECT : Gainor Residence @
ADDRESS : 5800 North Bay Road, Miami Beach, FL.
LOCATION : See attached plan

DATE: 04-04-05
HOLE NO.: B-2
DRILLER: ASLD

DEPTH	DESCRIPTION OF MATERIALS	SAMPLE NO.	HAMMER BLOWS ON SAMPLER	"N"
1		2	Hand	H
2				
3	+0'- 0" to 1'-0" TOPSOIL AND ROOTS AND GRASS	4	Hand	H
4				
5		6	1 2	4
6	1'-0" to 2'- 0" TAN BEACH SAND		2 1	
7		8	1 1	2
8	2'-0" to 4'-0" GRAY SILTY BEACH SAND		1 1	
9		10	2 2	3
10	4'-0" to 5'-0" PEAT		1 1	
11		12	2 2	3
12			1 1	
13	5'-0" to 15'-0" GRAY SILTY BEACH SAND	14	1 1	2
14			1 2	
15		16	A	A
16	15'-0" to 18'-0" TAN MEDIUM SAND WITH ROCK FRAGMENTS			
17		18	A	A
18	18'-0" to 35'-0" TAN SANDY LIME ROCK			
19		20	A	A
20				
21		22	20 23	47
22			24 23	
23		24	A	A
24				
25		26	A	A
26				
27		28	A	A
28				
29		30	24 25	52
30			27 24	
31		32	A	A
32				
33		34	A	A
34				
35		36	A	A
36				
37		38		
38				

Water Level: 5' Below Surface

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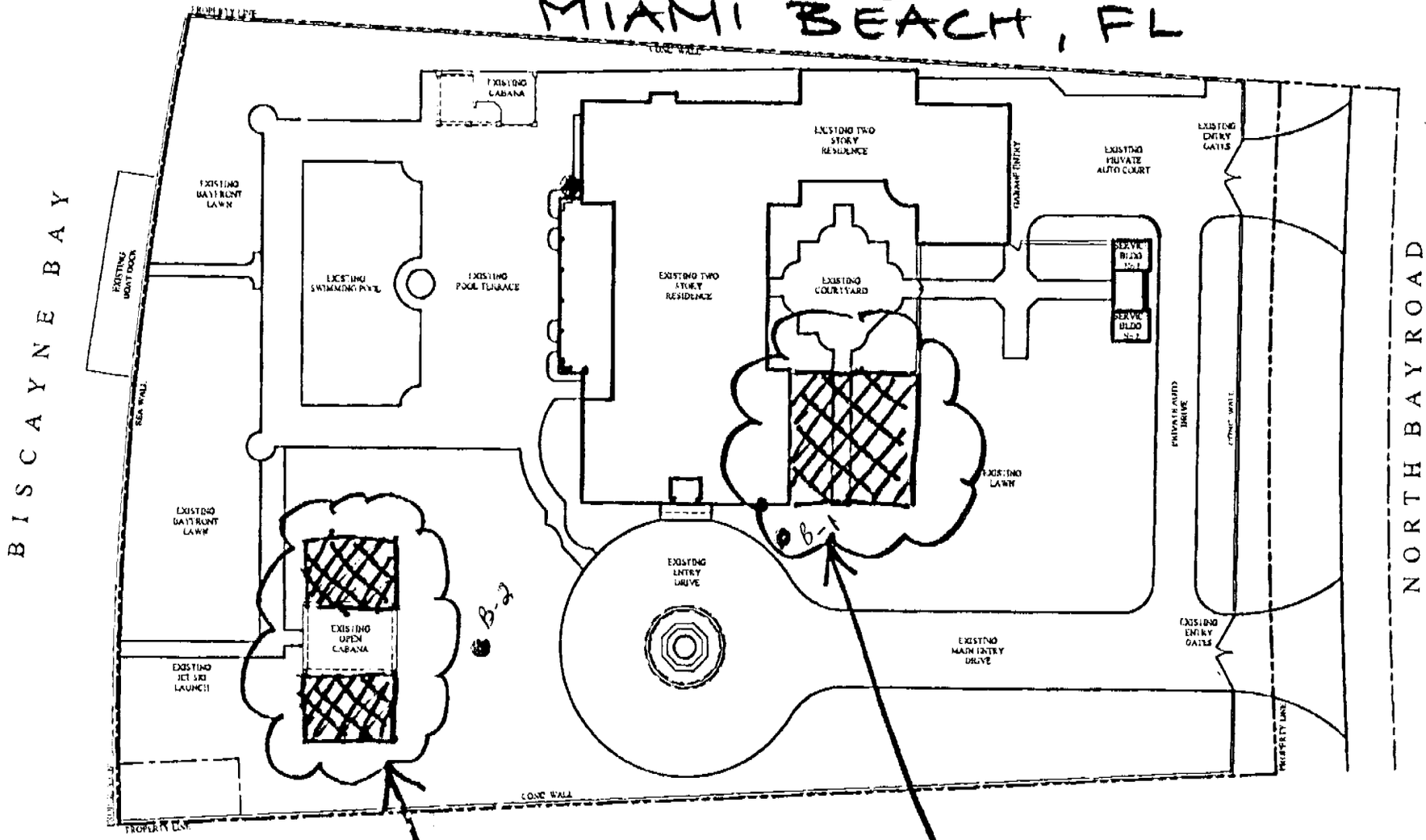
GAINOR RESIDENCE
5800 N. BAY ROAD
MIAMI BEACH, FL



OFFICE BLDG ADDITION (EXISTING CABANA TO BE

TWO STORY HOME ADDITION

05 02:44p Thomas Weber 305.361.9986 p.2

APPENDIX

GENERAL NOTES

Soil borings on unmarked vacant property should be considered preliminary with further boring(s) to be drilled after building pad(s) are staked out.

Soil borings on existing structures that are to be demolished should be considered preliminary and additional borings would need to be performed after the structure(s) has been demolished and proposed new building staked out.

As a mutual protection to clients, the public and ourselves, all reports are submitted as confidential property of clients, and authorization for publication of statements, conclusions, extracts from or regarding our reports is reserved pending our written approval.

KEY CLASSIFICATION & SYMBOLS

Correlation of Penetration Resistance With Relative Density and Consistency

	Cone Penetration Tests (Kg/cm^2)	Standard Penetration (Blows/ft.)	Relative Density
Sands	0-16	0-4	Very Loose
	17-40	5-10	Loose
	41-80	11-20	Firm
	81-120	21-30	Very Firm
	Over 120	31-50	Dense
Silts & Clay	0-3	0-2	Very Loose
	4-9	3-4	Soft
	10-17	5-8	Firm
	18-31	9-15	Stiff
	32-60	16-30	Very Stiff
	Over 60	31-50	Hard

Particle Size

Boulders	> 12 in.
Cobble	3 in. to 1 in.
Gravel	4.76 mm to 3in.
Sand	0.07 mm to 4.67 mm.
Silt	0.005 mm. to 0.074 mm
Clay	< 0.005 mm

Modifiers

5% - 10 %	Slightly Silty or Clayey
10% - 30%	Silty or Clayey
30% - 50%	Very Silty or Very Clayey
0% - 5%	Slightly Trace
6% - 10%	Trace
11% - 20%	Little
21% - 35%	Some
> 35% And	

Rock Hardness Description

Soft	Rock core crumbles when handled
Medium	Can break with your hands.
Moderate Hard	Thin edges or rock core can be broken with fingers
Hard	Thin edges or rock core cannot be broken with fingers
Very Hard	Rock core rings when struck with hammer (cherts)

Plot 9

SAM LYUBKIN, P.E.
CONSULTING ENGINEER, P.E. 34306 FL

Forum Building
2632 Hollywood Boulevard, Suite 103
Hollywood, Florida 33020
(954) 922-8845

2.11.05

7 FT. PRIVACY WALLS FOR GUNOR RESIDENCE AT 5800 NO. BAY ROAD, M.B.

1. WIND PRESSURES:
FBC 2004 (146 MPH) - 7' SIGN. DESIGN

2. DESIGN: USE 4" ϕ HELICAL PILES
W/ 10T - COMPRESSION
2T - TENSION
1T - LATERAL CAPACITIES, SOIL REPORT BY "DYNATECH, DATED, APRIL 5, 2005

In addition to the requirement of this permit, there may be additional restrictions applicable to this property that may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management's districts, state agencies, or federal agencies.
The City of Miami Beach assumes no responsibility for accuracy of or results from these plans which are approved subject to compliance with all Federal, State and Local Laws, Rules, and Regulations.

P2 of 9

WIND02 v2-10

Detailed Wind Load Design (Method 2) per ASCE 7-02

Analysis by: sl **Company Name:** sam lyubkin,p.e.

Description: 5800 No.Bay Road,M.B.

User Input Data		
Structure Type	Other	
Basic Wind Speed (V)	146	mph
Struc Category (I, II, III, or IV)	II	
Exposure (B, C, or D)	C	
Struc Nat Frequency (n1)	1	Hz
Kd (Directonality Factor)	0.85	
Overall Structure Ht (Ht)	7.00	ft
Width Perp. To Wind Dir (B)	10.00	ft
Width Paral. To Wind Dir (L)	7.00	ft

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table 6-2 Values		
Alpha =	9.500	
zg =	900.000	
At =	0.105	
Bt =	1.000	
Bm =	0.650	
Cc =	0.200	
l =	500.00	ft
Epsilon =	0.200	
Zmin =	15.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	1.00
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	15.00	ft
Izm	Cc * (33/z)^0.167	0.2281	
Lzm	l*(zm/33)^Epsilon	427.06	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9653	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.9068	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

WIND02 v2-10

Detailed Wind Load Design (Method 2) per ASCE 7-02

Design Wind Pressure - Open Buildings and Other Structures

Elev.	Kz	Kzt	Qz	Wind Pressure based upon different Cf's (lb/ft^2)					
				Solid Sign At Ground	???	???	???	???	???
ft			lb/ft^2	1.20	0.00	0.00	0.00	0.00	0.00
15	0.85	1.00	39.37	40.16	0.00	0.00	0.00	0.00	0.00

Note: 1) User must select shape factors from the appropriate tables, and enter within this table.

Figure 6-20: Wind Loads for Solid Signs At Ground Level

Ht - Height of Sign =	7.000 ft
W - Width of Sign =	10.000 ft
V - Ratio of Ht / Width	0.70
Cf - Shape Factor =	1.20
Wind Pressure @ Elevation =	7.000 ft
Kz =	0.85
Kzt =	1.00
Qz =	39.37 lb/ft^2
Wind Pressure =	40.16 lb/ft^2

USE 40.2 PSF

Notes: 1) Signs with openings comprising less than 30% of gross area are considered solid signs
2) Signs where distance from ground to bottom edge is less than 0.25 times vertical dimension are considered at ground level

2).

Title :
Dsgnr:
Description :

Job #
Date: 9:34AM, 3 DEC 07

Scope :

Code Ref: ACI 318-02, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Rev: 580000
User: KW-0602881, Ver 5.8.0, 1-Dec-2003
(c)1983-2003 ENERCALC Engineering Software

Cantilevered Retaining Wall Design

Description 5800 No.Bay Road : 7ft Wall

Criteria

Retained Height	=	1.00 ft
Wall height above soil	=	7.00 ft
Slope Behind Wall	=	0.00 : 1
Height of Soil over Toe	=	12.00 in
Soil Density	=	110.00 pcf
Wind on Stem	=	40.2 psf

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Heel Active Pressure	=	35.0
Toe Active Pressure	=	0.0
Passive Pressure	=	250.0
Water height over heel	=	0.0 ft
Footing||Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in

Footing Strengths & Dimensions

f'c = 3,000 psi	Fy =	60,000 psi
Min. As %	=	0.0014
Toe Width	=	1.25 ft
Heel Width	=	1.92
Total Footing Width	=	3.17
Footing Thickness	=	16.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft

Cover @ Top = 3.00 in @ Btm.= 3.00 in

Design Summary

Total Bearing Load	=	1,608 lbs
...resultant ecc.	=	12.50 in
Soil Pressure @ Toe	=	1,972 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,715 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	10.3 psi OK
Footing Shear @ Heel	=	3.9 psi OK
Allowable	=	93.1 psi
Wall Stability Ratios		
Overturning	=	1.51 OK
Sliding	=	3.09 OK
Sliding Calcs (Vertical Component Used)		
Lateral Sliding Force	=	376.7 lbs
less 100% Passive Force	= -	680.6 lbs
less 100% Friction Force	= -	482.4 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 : 1 Stability	=	0.0 lbs OK

Stem Construction

Top Stem

Stem OK

Design height	ft =	0.00
Wall Material Above "Ht"	=	Masonry
Thickness	=	8.00
Rebar Size	=	# 6
Rebar Spacing	=	16.00
Rebar Placed at	=	Center
Design Data		
fb/FB + fa/Fa	=	0.923
Total Force @ Section	lbs =	298.9
Moment....Actual	ft-# =	1,272.1
Moment.....Allowable	=	1,378.5
Shear.....Actual	psi =	7.6
Shear.....Allowable	psi =	38.7
Bar Develop ABOVE Ht.	in =	36.00
Bar Lap/Hook BELOW Ht.	in =	6.85
Wall Weight	=	84.0
Rebar Depth 'd'	in =	3.81
Masonry Data		
f'm	psi =	1,500
Fs	psi =	24,000
Solid Grouting	=	Yes
Special Inspection	=	Yes
Modular Ratio 'n'	=	25.78
Short Term Factor	=	1.000
Equiv. Solid Thick.	in =	7.60
Masonry Block Type = Normal Weight		
Concrete Data		
f'c	psi =	
Fy	psi =	

Other Acceptable Sizes & Spacings
Toe: Not req'd, Mu < S * Fr
Heel: Not req'd, Mu < S * Fr
Key: No key defined

Footing Design Results

		Toe	Heel
Factored Pressure	=	2,715	0 psf
Mu' : Upward	=	1,936	0 ft-#
Mu' : Downward	=	436	494 ft-#
Mu: Design	=	1,500	494 ft-#
Actual 1-Way Shear	=	10.34	3.93 psi
Allow 1-Way Shear	=	93.11	93.11 psi
Toe Reinforcing	=	# 4 @ 18.00 in	
Heel Reinforcing	=	# 4 @ 18.00 in	
Key Reinforcing	=	None Spec'd	

Title :
Dsgnr:
Description :

Job #
Date: 9:34AM, 3 DEC 07

Scope :

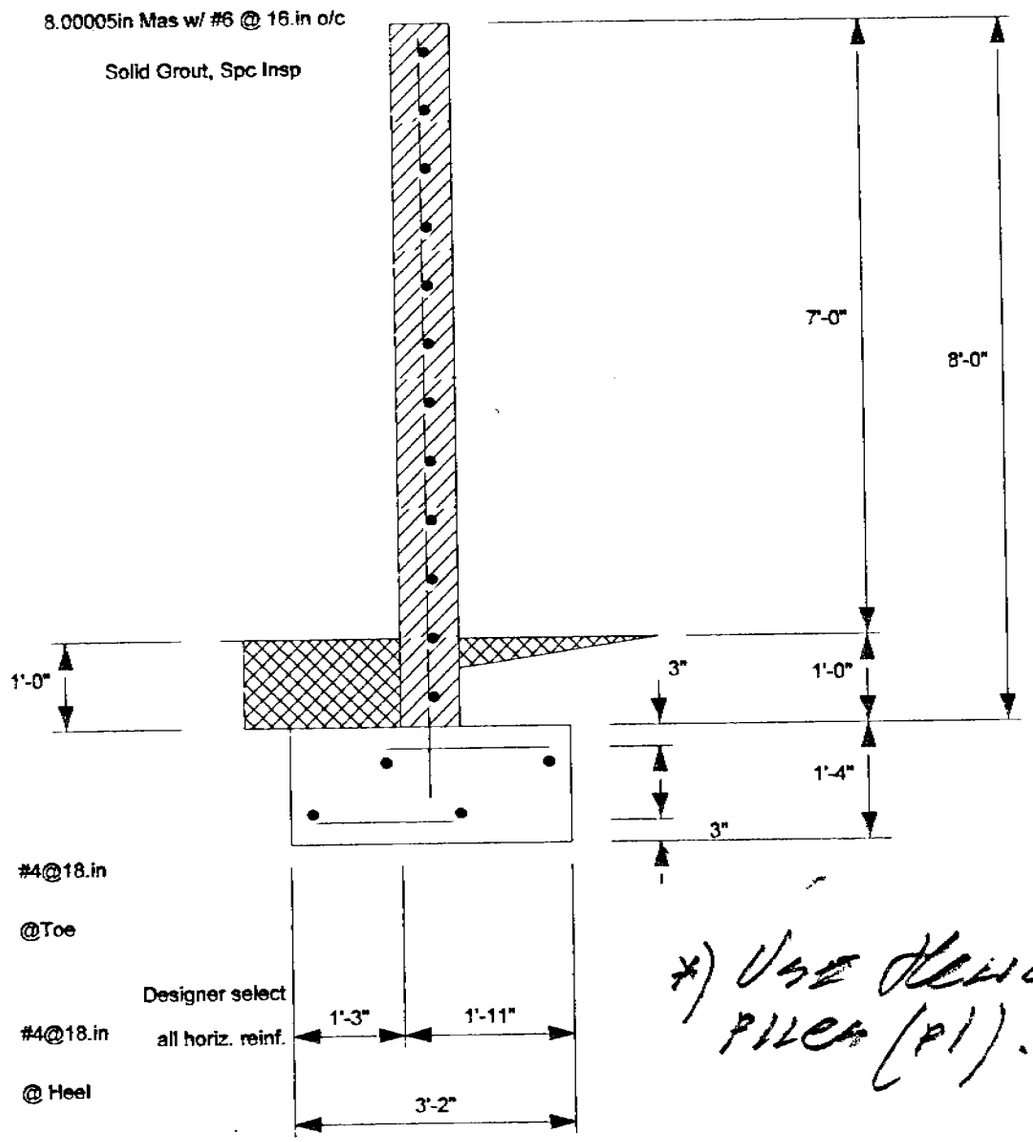
Code Ref: ACI 318-02, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Description 5800 No.Bay Road : 7ft Wall

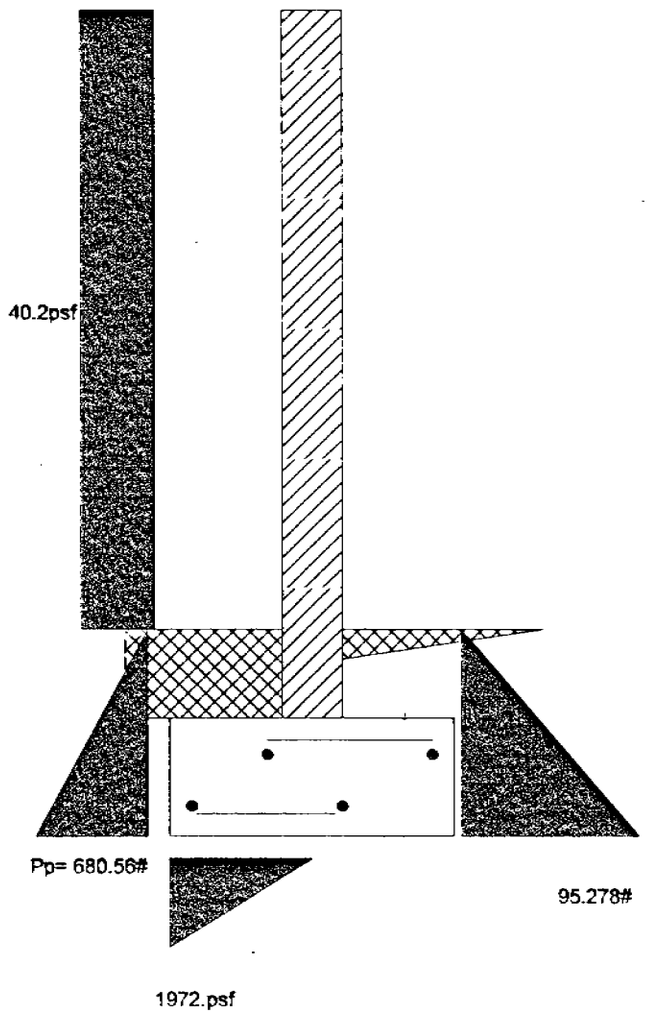
Summary of Overturning & Resisting Forces & Moments

Item		.....OVERTURNING..... Force lbs	Distance ft	Moment ft-#		.....RESISTING..... Force lbs	Distance ft	Moment ft-#
Heel Active Pressure	=	95.3	0.78	74.1	Soil Over Heel	= 137.9	2.54	350.6
Toe Active Pressure	=				Sloped Soil Over Heel	=		
Surcharge Over Toe	=				Surcharge Over Heel	=		
Adjacent Footing Load	=				Adjacent Footing Load	=		
Added Lateral Load	=				Axial Dead Load on Stem	=	0.00	
Load @ Stem Above Soil	=	281.4	5.83	1,641.5	Soil Over Toe	= 137.5	0.63	85.9
SeismicLoad	=				Surcharge Over Toe	=		
					Stem Weight(s)	= 672.0	1.58	1,064.0
Total	=	376.7	O.T.M. =	1,715.6	Earth @ Stem Transitions	=		
					Footing Weight	= 634.0	1.59	1,004.9
Resisting/Overturning Ratio		=	1.51		Key Weight	=		
					Vert. Component	= 26.6	3.17	84.2
Vertical Loads used for Soil Pressure		=	1,607.9 lbs		Total	= 1,607.9 lbs	R.M.=	2,589.6

Vertical component of active pressure used for soil pressure

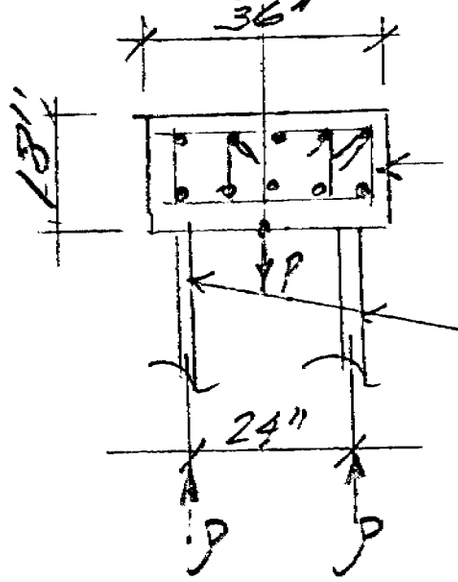


*) Use Helical Piles (PI).


40.2psf
Pp= 680.56#
95.278#
1972.psf

P80=9

℄ OF 'GB' & WALL



18"x36" WIDE GRADE BEAM W/5#6 T&B, #4@8"℄ TIES (2 SETS)

4" ϕ Helical Piles

TRY PILES W/ MAX. SPACING = 9'-0"

MAX. V = (40.2×7)×9 ≈ 2.55K.

$M_{MAX}\ M = [(40.2\times7)\times(3.5+1)]\times9 \approx 11.4$ K-FT.

$S_M = \frac{11.4}{2} = 5.7$K. ↕

$P_{DL} = [(\frac{150\times18\times36}{144\ FT^2}) + (\frac{100\times28}{12\ SOIL}) + (\frac{60\times8}{WALL})]\times9 \approx$

≈ 12.5K ↓

- Σ Load ↓ = (12.5) + (5.7) = 18.2K < (20K×2) 40K (2 PILES)
- Σ Load ↓ PILE = (12.5/2) + (5.7) ≈ 12K < 20 K - OK

Σ Load $\uparrow = (5.7) - \left(\frac{12.5}{2}\right) = -0.6k - ok$

OVERTURNING: $11.4 - (12.5 \times 1.5) =$

$= -7.35k - ok.$

$\left(\frac{12.5 \times 1.5}{11.4}\right) = 1.64 > 1.0 - ok.$

CONTACT :
LISA VAZQUEZ
305.318.3310

DRAWN BY:

REVISIONS:

GENERAL NOTES

1. THESE DOCUMENTS, AS INSTRUMENTS OF SERVICE, ARE THE PROPERTY OF 3DESIGN, Inc. AND MAY NOT BE USED OR REPRODUCED IN ANY MANNER WITHOUT EXPRESSED WRITTEN CONSENT
2. IT IS NOT THE INTENT OF THESE PLANS TO SHOW EVERY MINOR DETAIL OF CONSTRUCTION. THE GENERAL CONTRACTOR IS EXPECTED TO FURNISH AND INSTALL ALL ITEMS REQUIRED TO COMPLETE ALL BUILDING SYSTEMS AND PROVIDE ALL NECESSARY APPURTENANCES FOR EQUIPMENT TO BE PLACED IN PROPER WORKING ORDER WITH QUALITY CRAFTMANSHIP WITHOUT INCREASING THE CONTRACT SUM OR CONTRACT COMPLETION DATE.
3. ALL WORK DESCRIBED BY THSE DOCUMENTS MUST BE PERFORMED BY CONSTRUCTION PROFESSIONALS LICENCED & INSURED IN THE STATE OF FLORIDA (F.B.C. REQUIRED). ALL WORK SHALL BE PERFORMED IN ACCORDANCE W/ THE NATIONAL ELECTRIC CODE AND F.B.C.
4. THE GENERAL CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE JOB SITE PRIOR TO COMMENCING THE WORK. IF THERE ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND FIELD CONDITIONS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT PRIOR TO COMMENCING THE WORK FOR CLEAR INSTRUCTION.
5. DO NOT SCALE THE DRAWINGS. REFER TO FIGURED DIMENSIONS.
6. THE CONTRACTOR IS TO ACQUIRE ALL REQUIRED PERMITS FOR THE DEMOLITION, CONSTRUCTION, FINISHING, AND OCCUPANCY OF THE PROJECT. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE F.B.C., LATEST EDITION.
7. ALL WORK DONE UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR SHALL BE IN A NEAT AND WORKMAN-LIKE MANNER IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL GOVERNING AGENCIES HAVING JURISDICTION.
8. THE GENERAL CONTRACTOR IS TO PROVIDE, LOCATE AND BUILD INTO THE WORK ALL SUPPLEMENTARY MATERIALS (INSERTS, ANCHORS, ANGLES, PLATES, OPENINGS, SLEEVES, HANGERS, SLAB DEPRESSIONS, PITCHES, ETC.) AS REQUIRED TO PROPERLY INSTALL, SUPPORT, BRACE, AND SHORE ALL BUILDING COMPONENTS WITHIN THE SCOPE OF THE PROJECT.
9. THE GENERAL CONTRACTOR SHALL REPAIR ALL DAMAGES TO THE EXISTING BUILDING DURING CONSTRUCTION RESULTING FROM SUCH LACK OF CARE AND DUE DILIGENCE AND MAY NOT CLAIM MONETARY DAMAGES OR TIME DELAYS AGAINST THE CONTRACT SUM OR CONTRACT COMPLETION DATE.
10. THE GENERAL CONTRACTOR SHALL COORDINATE AND SCHEDULE THE WORK OF ALL TRADES TO INSURE THAT THE PROJECT IS COMPLETED BY THE CONTRACT COMPLETION DATE.
11. PRIOR TO COMMENCING WORK, THE GENERAL CONTRACTOR SHALL SITE VERIFY THE LOCATION OF ALL EQUIPMENT TO BE REMOVED/RELOCATED. REMOVALS SHALL BE COORDINATED WITH THE OWNER. IF SO DIRECTED, THE G.C. MAY INCLUDE ANY ADDITIONAL COSTS TO THE BID.
12. THE GENERAL CONTRACTOR SHALL PROVIDE AN ONSITE DUMPSTER IN A LOCATION COORDINATED WITH THE OWNER FOR THE DISPOSAL OF REMOVED MATERIAL/CONSTRUCTION DEBRIS. THE DUMPSTER SHALL BE EMPTIED AT APPROPRIATE INTERVALS TO PREVENT OVERFLOW AND UNSIGHTLY CONDITIONS.
13. IT IS THE INTENT OF 3DESIGN, Inc. THAT THIS WORK BE IN CONFORMANCE WITH ALL REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY. THE G.C. SHALL NOTIFY THE ARCHITECT/ENGINEERS OF RECORD IMMEDIATELY IF ANY DISCREPANCIES ARE ENCOUNTERED BETWEEN THE DRAWINGS AND THESE REQUIREMENTS. ANY DISCREPANCIES WILL BE RESOLVED BY ARCHITECT / ENGINEER OF RECORD PRIOR TO PROCEEDING WITH THE WORK.
14. THE GENERAL CONTRACTOR SHALL PROVIDE THE ARCHITECT WITH RED LINE AS BUILT DRAWINGS FOR ALL FIELD CHANGES/ADDITIONS TO THE WORK INCLUDED IN THE WORK.
15. THE GENERAL CONTRACTOR SHALL PROVIDE AN ITEMIZED COST BREAKDOWN OF ALL ITEMS AND PHASES OF CONSTRUCTION AT THE TIME OF BIDDING.
16. 3DESIGN, Inc. IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, SEQUENCES, PROCEDURES, PRECAUTIONS, OR PROGRAMS RELATED TO THIS PROJECT'S CONSTRUCTION.
17. ALL WORK IS TO BE PLUM, LINE, SQUARE, AND ADEQUATELY SUPPORTED. FILL ALL VOIDS BETWEEN COMPONENTS. ALL ITEMS THAT DO NOT MEET 3DESIGN, Inc. SATISFACTION AS TO GOOD TRADE PRACTICES AND QUALITY CRAFTMANSHIP WILL BE REDONE AT THE G.C.'S EXPENSE.
18. THE GENERAL CONTRACTOR IS TO MAINTAIN A SAFE SITE, CLEAR OF DEBRIS AT ALL TIMES.
19. THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS ARE TO GUARANTEE THEIR WORK FOR A MINIMUM PERIOD OF ONE YEAR IN WRITING SUBMITTED WITH THE BID.
20. ALL DETAILS AND SECTIONS SHOWN ON THESE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE IN THE WORK EXCEPT WHERE A DIFFERENT DETAIL IS SHOWN. IT IS THE G.C.'S RESPONSIBILITY TO FORSEE ADDITIONAL CONDITIONS PRIOR TO COMMENCING THE WORK AND NOTIFY THE ARCHITECT IMMEDIATELY.
21. ALL ASSEMBLIES REFERRED TO AS FIRE-RATED SHALL BE A MINIMUM OF ONE HOUR UNLESS OTHERWISE INDICATED. ALL PENETRATIONS THROUGH ANY RATED ASSEMBLY SHALL BE PROVIDED W/ APPROVED PENETRATION RATED DEVICES.
22. THE GENERAL CONTRACTOR SHALL PROVIDE CUSTOM AND MULTI-COLOR PAINT SELECTIONS FOR OWNER'S APPROVAL.
23. ALL HARDWARE, LIGHTING & BATHROOM FIXTURES AND MISC. SPECIFICATIONS NOT SPECIFICALLY CALLED OUT ON THE DRAWINGS SHALL BE PROVIDED BY THE OWNER.
24. THE GENERAL CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR TRUSSES, STORM SHUTTERS, REINFORCING STEEL, WINDOWS, DOORS, CAST CONCRETE, ORNAMENTAL IRON, STEEL CONNECTORS, ORNAMENTAL WOOD, ETC. TO THE ARCHITECT FOR REVIEW PRIOR TO COMMENCING THE WORK.
25. THE GENERAL CONTRACTOR SHALL PROVIDE A TELEPHONE AND TELEPHONE LINE AT THE JOB SITE. THE G.C. IS RESPONSIBLE FOR IT'S USE.

48 HOURS PRIOR TO EXCAVATING
CONTRACTOR SHALL CALL FOR LOCATION
OF UNDERGROUND UTILITIES
SUNSHINE ONE-CALL 1-800-432-4770
CITY OF MIAMI BEACH 305-673-7080

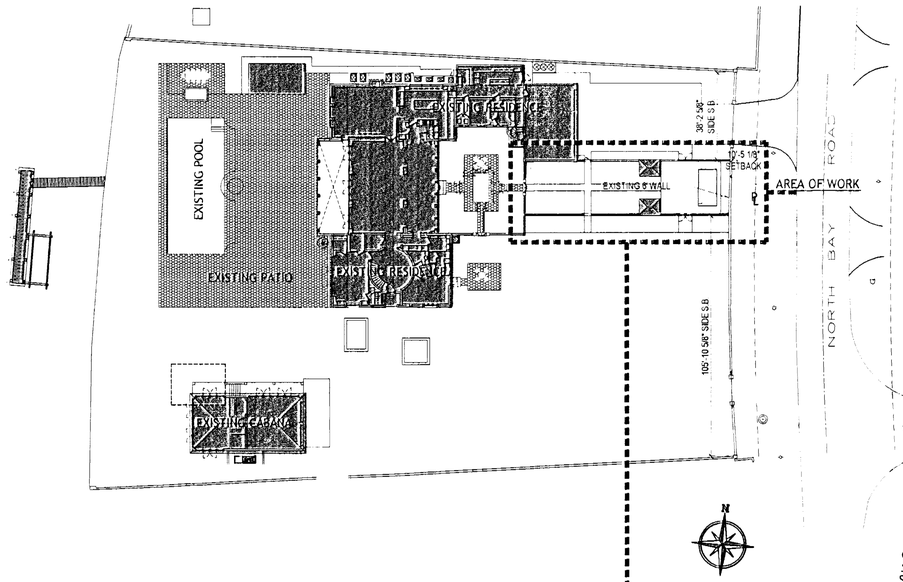
PUBLIC WORKS
PLAN REVIEW NOTICE
Phone 305-673-7080 Fax 305-673-7028

THIS PLAN REVIEW CONSTITUTES APPROVAL FOR OBTAINING BUILDING PERMITS ONLY.

All construction and or use of equipment in the right-of-way and or easements, requirse a separate Public Works Department permit prior to start of construction.

Permit Requirements: Proof of existing sidewalk swale area conditions (pictures) and or posting of sidewalk roadway bonds (Public Works inspection of the right-of-way will be required prior to final sign-off on the C.C., C.O., or the release of bonds.)

Approved Reviewed By: B. Duvall Date: 1/9/08



SITE PLAN
SCALE 1/32" = 1'-0

SCOPE OF WORK:

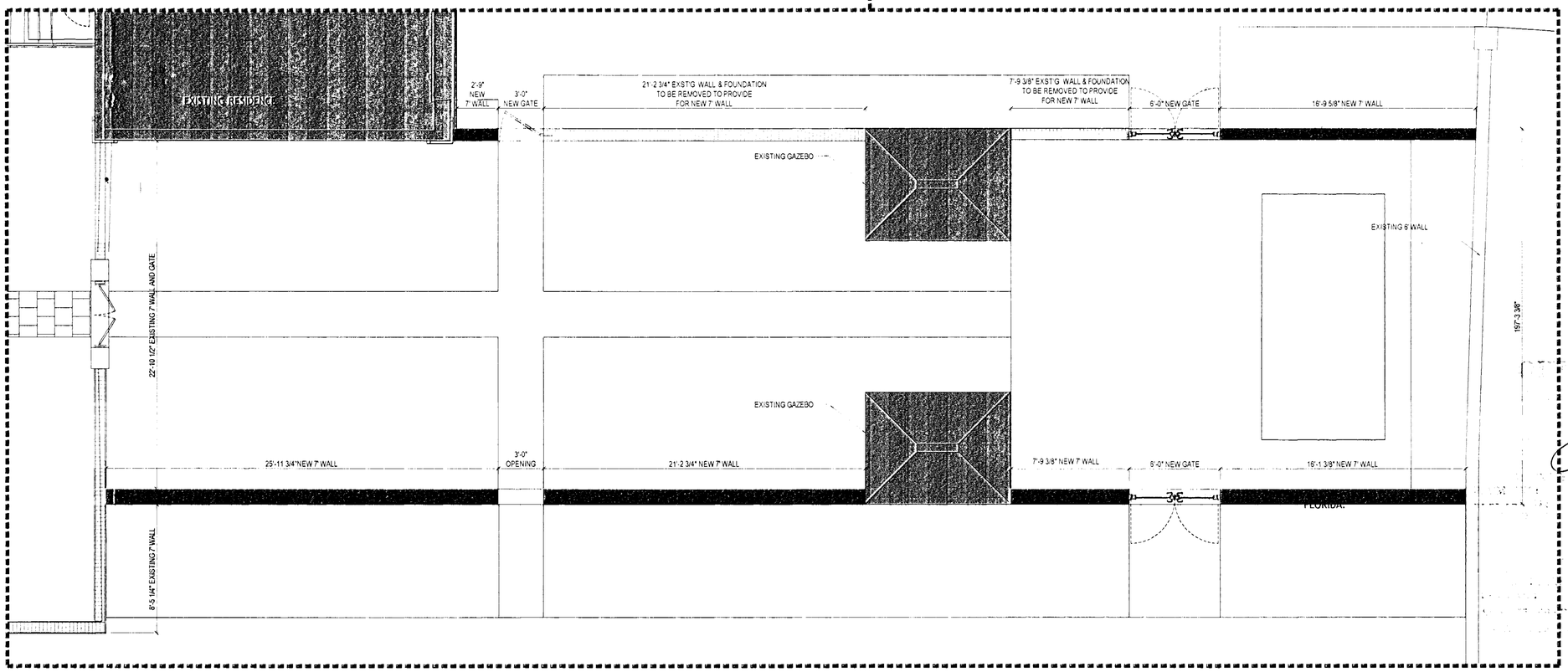
NEW 7' GARDEN WALL

ZONING DATA
LEGAL DESCRIPTION:

LOTS 33 AND 34, BLOCK 1A, OF "LAGORCE-GOLF SUBDIVISION", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 14, AT PAGE 43, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA.

SITE PLAN INFORMATION:

ZONING DESIGNATION:



ENLARGED AREA PLAN
SCALE 1/4" = 1'-0

3DESIGN INC
ANTHONY LEON
AR0008752
ARCHITECTURE
4500 BISCAYNE BLVD. SUITE #30. MIAMI, FL 33137 T.305.458.9577 F.305.458.9579

SEAL

A GARDEN WALL
FOR
THE GAINOR RESIDENCE
FOR
5800 NORTH BAY ROAD
MIAMI BEACH, FL
33141

B. Duvall 1/11/08

B. Duvall 1/9/08 or 1/14/08

THESE PLANS ARE FOR BUILDING DEPARTMENT REVIEW ONLY. THEY ARE NOT TO BE CONSTRUED AS CONSTRUCTION DOCUMENTS UNTIL ALL BUILDING DEPARTMENT APPROVALS ARE OBTAINED.

A1.0

NOVEMBER 21, 2007

STRUCTURAL NOTES:

STRUCTURAL DESIGN SHALL CONFORM WITH ALL LOCAL & FEDERAL ORDINANCES AND BUILDING CODES (FLORIDA BUILDING CODE 2004)

DRAWING DIMENSION & COORDINATION:

DIMENSIONAL INFORMATION SHALL BE BASED ON THE ENTIRE SET OF CONTRACT DOCUMENTS IN TOTAL. COORDINATE ALL THE REQUIREMENTS OF CIVIL, ARCHITECTURAL STRUCTURAL, MECHANICAL, ELECTRICAL, AND INTERIOR DESIGN DRAWINGS TO SUPLEMENT CONTRACT DOCUMENTS WHERE NECESSARY.

REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO PROCEEDING.

WHERE CRITICAL DIMENSIONS CANNOT BE DETERMINED FROM THE PLANS, OR WHERE NEW WORK ADJOINS EXISTING CONSTRUCTION, OR WHERE ONE MATERIAL ADJOINS A PREVIOSLY PLACED MATERIAL WITH A MORE RESTRICTIVE TOLERANCE THAN THE IN-PLACE MATERIAL CONTRACTOR SHALL TAKE FIELD MEASUREMENTS AS REQUIRED TO COMPLETE SHOP DRAWINGS AND INSTALLATION. REPORT ANY DISCREPANCIES EXCEEDING 5% BETWEEN FIELD MEASURED DIMENSION AND SCALED DRAWING DIMENSIONS TO ARCHITECT BEFORE PROCEEDING WITH THE WORK.

WHERE A LINE OF STRUCTURE, OPENING LOCATION, OR DIMENSION IS CRITICAL, AND BASED ON THE REQUIREMENTS OF ANOTHER TRADE OR SUBCONTRACTOR; THAT SUBCONTRACTOR SHALL SUBMIT A SHOP DRAWING WITH THE REQUIRED DIMENSIONAL INFORMATION UPON WHICH THE CONTRACTOR SHALL BASE THE LAYOUT AND CONSTRUCTION. THIS PROCEDURE IS MANDATORY FOR CURTAIN WALL SYSTEMS AND ALL MECHANICAL AND ELECTRICAL OPENINGS.

CONTRACTOR TO VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH THE WORK, AND REPORT ANY DISCREPANCIES TO THE ARCHITECT AND ENGINEER FOR APPROPIATE ACTION

CAST IN PLACE CONCRETE:

TO BE MIXED AND PLACED IN ACCORANCE WITH ACI 301. ALL REINFORCING CONCRETE TO HAVE 28 DAY COMPRESSIVE STRENGTH OF f'c=5000 PSI.(W/C=04) CONTRACTOR SHALL SUBMIT ALL CONCRETE MIX DESING FOR REVIEW. CONCRETE MIX SUBMITTAL SHALL COMPLY WITH SECTION 4.3 OF A.C.I. 318 AND MUST SHOW STRENGTH DATA AT 7, 14 AND 28 DAYS. STANDARD DEVIATION AND MODULOS OF ELASTICITY. CONCRETE COMPRESSION TESTING TO COMPLY WITH LOCAL BUILDING CODES. PROVIDE CONCRETE INSERTS AS PER ARCHITECT OR MECHANICAL ENGINEER'S REQUIREMENTS.

CONCRETE REINFORCEMENT:

WORK SHALL BE IN ACCORDANCE WITH ACI 301, ACI 318-02, CRSI "MANUAL OF STANDARD PRACTICE". BARS SHALL CONFORM TO ASTM SPECIFICATION AG 15(51), GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

CONCRETE COVER REQUIRED AS FOLLOWS:

* CAST AGAINST AND EXPOSED TO EARTH	3"
* FORMED, EXPOSED TO EARTH OR WEATHER	
# 6 AND LARGER	2"
# 5 AND SMALLER	2"
* SLABS AND WALLS, NO WEATHER EXPOSURE	
2 HR. FIRE RATING & LESS	1"
3 HR. FIRE RATING & MORE	1"

CONTRACTION JOINTS IN SLABS IN SLABS & BEAMS MAY OCCUR WITHIN THE MIDDLE 1/3 PORTION OF THE SPAN. CONTRACTION JOINTS SHALL BE KEYED WITH 1-1/2" KEY.

ALL NEW MASONRY CONSTRUCTIOM TO BE IN ACCORDANCE WITH "SPECIFICATION FOR CONCRETE MASONRY CONSTRUCTION" ACI 530. AND ALL LOCAL APPLICABLE BUILDING CODE PROVISIONS. (PROVIDE HORIZ. LADDER REINFORCING @ 16" O.C. EMBED 4" INTO COLS.)

PROVIDE CORNER BAR AT ALL BEAM INTERSECTIONS AND CORNERS. CORNERS BARS SHALL BE SAME SIZE AND QUANTITY AS SCHEDULE BEAM REINFORCING EXTENDING 30" BOTH DIRECTIONS.

STRUCTURAL DESIGN CRITERIA:

THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE 2004, AND OTHER REFERENCED CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT THE TIME OF PERMIT.

WIND LOAD CRITERIA:

BASIC VELOCITY -- 146 MPH, AT A HEIGHT OF 30 FT. (F.B.C. 2004)
EXPOSURE CATEGORY: C
BUILDING CATEGORY: 2
IMPORTANCE FACTOR: 1.0
GCPI = +-0.18

DESIGN LOADS:

SEE PLAN FOR LOADING INFORMATION.

MASONRY WALLS:

1- HORIZONTAL REINFORCING:
PROVIDE STANDARD, TRUSS TYPE GA#9 HORIZONTAL JOINT REINFORCING EVERY 2nd BLOCK COURSE (1'-4" O.C. VERTICALLY) LAPPED 6", PROVIDE SPECIAL HORIZONTAL REINFORCING AT "T" AND "L" INTERSECTION. ANCHOR TO COLUMNS WITH MINIMUM 8" EXTENSION INTO AREA OF POUR.

2- CELL FILLING CONCRETE SHALL BE "PEA ROCK" CONCRETE MIX OR GROUT WITH f'c=3,000 PSI MIN. AT 28 DAYS. (9" SLUMP)

3- PROVIDE CORNER BAR AT ALL BEAM INTERSECTIONS AND CORNERS, CORNERS BARS SHALL BE SAME SIZE AND QUANTITY AS SCHEDULED BEAM REINFORCING EXTENDING 18" IN BOTH DIRECTIONS.

4- INSPECTION REQUIRED.

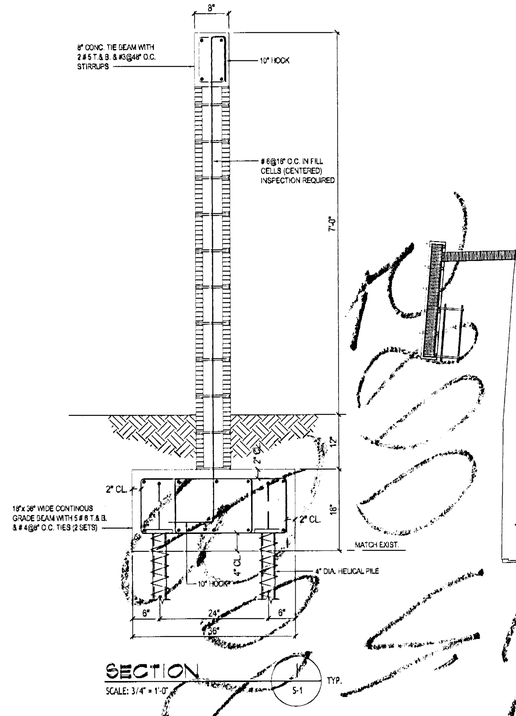
5- HOLLOW LOAD BEARING MASONRY UNITS SHALL CONFORM TO ASTM C-90, TYPE 1, GRADE N, SQUARE END, WITH A MINIMUM AVERAGE COMPRESSIVE STRENGTH ON NET AREA OF f'm=1500 PSI.

6- MORTAR SHALL CONFORM TO ASTM C-270, TYPE "M" WITH A 28 DAY STRENGTH OF 2,500 PSI.

7- LAY ALL MASONRY WITH FULL FACE HEAD JOINTS AND WITH FACE SHELL MORTAR BEDDING.

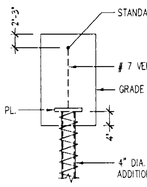
8- MASONRY ANCHORAGE TO SUPERSTRUCTURE SHALL BE PROVIDED IN ACCORDANCE WITH STRUCTURAL DRAWINGS AND DETAILS.

9- THE USE OF ADMIXTURES SHALL NOT BE PERMITTED WITHOUT PRIOR REVIEW OF THE ENGINEER.



SECTION
SCALE: 3/4" = 1'-0"

SITE PLAN
SCALE 1/32" = 1'-0



HELICAL PILE DETAIL

SCALE: 3/4" = 1'-0"

10 TONS COMPRESSION
2 TONS TENSION
1 TON LATERAL

STEEL PIPE PILE (HELICAL PILE) NOTES:

PILES SHALL BE 4" DIAMETER, SCHEDULE 80 PIPES, COATED WITH TWO COATS OF COAL- TAR EPOXY RUST PROOF COATING OR GALVANIZED. PILES SHALL BE SOLID FILLED WITH 4000 PSI GROUT.

PILES SHALL BE DRIVEN INTO GROUT TO DEVELOP TEN (10) TONS SAFE BEARING CAPACITY. THE CAPACITY ACHIEVED SHALL BE CERTIFIED BY INSTALLER'S ENGINEER.

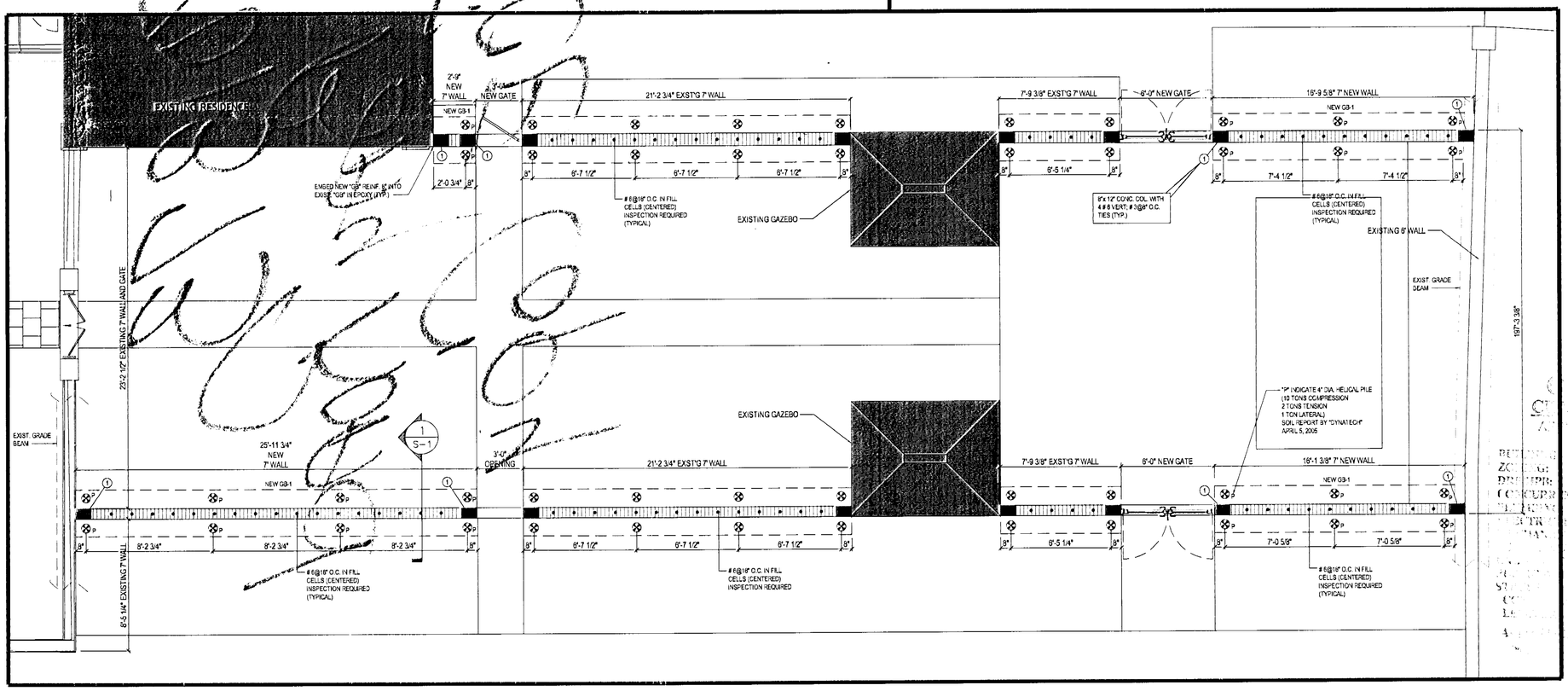
PROVIDE A 8"x 8"x 3/8" STEEL CAP PLATE WITH A 2" DIAMETER HOLE. TACK WELD TO SECURE CAP PLATE TO HELICAL-PILE.

PROVIDE 1 # 7 VERTICAL REBAR, FULL LENGTH OF PILE (VERIFY WITH SOIL ENGINEER), EMBEDDED IN GROUT, CENTERED WITH THE PILE. EXTEND REBAR ABOUT 14" ABOVE THE PILE FOR CONNECTION WITH THE CAST IN PLACE CAP OR GRADE BEAM. PROVIDE A STANDARD HOOK AT THE TOP END OF REBAR.

EMBED PILE 4" INTO CAP OR GRADE BEAM.

IN THE EVENT OF ANY DISCREPANCY OF THE BEARING CAPACITY OF THE PILE WHEN DRIVEN INTO THE GROUND, THE PILE INSTALLER SHALL NOTIFY THE ARCHITECT IMMEDIATELY AND THE DEVIATION SHALL BE REMEDIED PRIOR TO PROCEEDING WITH ANY FURTHER WORK.

PILE INSTALLER SHALL HAVE A MINIMUM 5 YEAR EXPERIENCE IN PIN-PILE INSTALLATION.



ENLARGED AREA PLAN
SCALE 1/4" = 1'-0

DRAWN BY:

REVISIONS:

SEAL

3D DESIGN INC
ANTHONY LEON
AR0006752
ARCHITECTURE
4500 BISCAYNE BLVD. SUITE#204 MIAMI, FL 33137 T.305.438.9377 F.305.438.9379

A GARDEN WALL
FOR
THE GAINOR RESIDENCE
FOR
5800 NORTH BAY ROAD
MIAMI BEACH, FL
33141

THESE PLANS ARE FOR BUILDING DEPARTMENT REVIEW ONLY. THEY ARE NOT TO BE CONSTRUED AS CONSTRUCTION DOCUMENTS UNTIL ALL BUILDING DEPARTMENT APPROVALS ARE OBTAINED.

Sam Lyubkin, P.E.
Consulting Engineer
Forum Building 2632 Hollywood Boulevard
Suite 103 Hollywood, Florida 33020
(954) 922-8845
P.E. #34306
12-18-07

TO THE BEST OF THE ENGINEER'S KNOWLEDGE THE PLANS AND SPECIFICATIONS BY THE ENGINEER FOR THIS BUILDING COMPLY WITH THE MIAMI-DADE APPLICABLE BUILDING CODE

S1.0

NOVEMBER 21, 2007

B0801373

5800 N. Bay Road

OFFICE COPY

B0801373

CITY OF MIAMI BEACH
Building Department
1700 Convention Ctr Drive, 2nd Floor
Miami Beach, Florida 33139

Inspections: (305) 673-7370 **Office: (305) 673-7610**

Bldg Small Work Permit

01-14-2008

Activity Number: B0801373

Status: APPROVED
Site Address: 5800 N BAY RD MBCH
Parcel #: 32150030270

Issued By: BUILSANK
Applied: 01/09/2008
Approved: 01/14/2008
Completed:
To Expire: 07/12/2008

Valuation: $35,000.00

Applicant: FRAME-PRO CONSTRUCTION CORP
17031 SW 150 CT
MIAMI FL 33187
786-346-0932

Property Owner: MARK J GAINOR &W ELYSE S
MARK J GAINOR TRUSTEE
7463 FISHER ISLAND DR 331090717

CONDTION(s):

Description: **NEW 7' GARDEN WALL 100 LFT.**
Inspector Area: C Class Code: R3

==========

DETAIL LIST

Alteration/Repair Fees

Alteration Bulding/Structures - Per Costs:	$0.00	$0.00
Awning, Canopy, Patio Cover - Per Costs:	$0.00	$0.00
Area Under Roof - RADON - Per Sq.Ft.:	0	$0.00
Walk-Thru - Per Valuation:	$0.00	$35.00
Repairs to Building/Structure - Per Costs:	$0.00	$0.00
Roofing or Re-roofing - Per Sq.Ft.:	0	$0.00
Window/Doors - Per # of:	0	$0.00
Signs 36-4 (Writer/Erect) - Per Sq.Ft.:	0	$0.00
Fence and/or Wall - Per Linear Feet:	100	$65.00
Partial Demo (Struct, Sign, Wall) - Per Costs:	$0.00	$0.00
Swimming Pool - Per Gallon:	0	$0.00
Painting - Per Costs:	$0.00	$0.00
Sandblasting - Per Costs:	$0.00	$0.00
Paving - Per Sq.Ft.:	0	$0.00
Concrete Slab - No Paving - Per Sq.Ft.:	0	$0.00
Trees - Per # of:	0	
Hedges - Per Linear Feet:	0	
Groundcover - Per Sq.Ft.:	0	
Landscaping Fee:		$0.00
Other Fees:		$0.00
Penalty Fee (If Applicable):		$0.00

[BSBLDPRM]

Activity Number: B0801373

Fire Safety Fees		
New Building or Addition - Per Sq.Ft.:	0	$0.00
Storage/Industrial Bldg - E & F Occup - Per Sq.Ft.:	0	$0.00
Greenhouse/Argiculture on Premises - Per Sq.Ft.:	0	$0.00
Screen Enclsoure/Trail on Premises - Per Sq.Ft.:	0	$0.00
SS Underground Tanks/App Shelter - Per #:	0	$0.00
Construction not shown Above - Per Costs:	$0.00	$0.00
Alt/Repair Building/Structure - Per Costs:	$0.00	$0.00
Marine Structure Fee		
Dock Area - Per Sq.Ft.:	0	$0.00
Seawall - Per Linear Feet:	0	$0.00
Boat Lifts, Davits, Hoist - Per # of:	0	$0.00
Batter, Mooring, Dock Piles - Per # of:	0	$0.00
Marine Structure Alt/Repair - Per Costs:	$0.00	$0.00
SFBC Compliance Surcharge		
New Const/Add - Res/Mult-Fam/Comm - Per Sq.Ft.:	0	$0.00
New Const/Add - Strg/Ind/Msc - Per Sq.Ft.:	0	$0.00
Cost for Other Construction:		$0.00
Training Fee		
Training Fee:		$35.00
Sanitation Fee:		$105.00
Additional Fees		
1st Reinspection:		$0.00
Continued Reinspections - Per # of:	0	$0.00
Building Joint Inspections - Per # of:	0	$0.00
Change of Contractor Per # of:	0	$0.00
Permit Extension - Per # of:	0	$0.00
Residential Card:		
Commercial Card:		
Permit Card Replacements:		$0.00
Lost Plan Fee - SF:		$0.00
Lost Plan Fee - Other:		$0.00
Overtime Inspection Fees:		$0.00
Total of All Fees:		$261.00
Total of Payments:		$261.00
Balance Due:		$0.00

MIAMIBEACH

BUILDING DEPARTMENT
1700 Convention Center Drive
Miami Beach, FL 33139
Office: 305-673-7610 Fax: 305-673-7857

• Liability Insurance Expired
• Lock on License NEED
* Struc
* Public.
* Zon

WORK PERMIT APPLICATION

FLORIDA BUILDING CODE IN EFFECT

$861.00
$175.00 front fee.

DATE ____/____/____ PERMIT # B0801373

If subsidiary or revision, provide the Master building permit number here B: _____

IS THIS PERMIT ASSOCIATED WITH A VIOLATION? If so; BV# _____

For DEMOLITION provide the year the structure was built _____ **Historic District** ☐Yes ☐N

Type of Property ☐Single Family ☐Commercial ☐Multi-Family/Condo

TYPE OF IMPROVEMENT: ☐Building ☐Electrical ☐Plumbing ☐Mechanical ☐ **REVISION**

Describe NEW 7' GARDEN WALL

Job Value $35,000.00 **Square Feet** _____ **Linear Feet** 100 **Pool Gallonage** _____ **No. of units** _____

Job Address 5800 NO. BAY ROAD

Folio # 3215 00302270 **Unit #** _____

City MIAMI BEACH State FLA Zip _____ Phone _____

Owner/Owner Builder _____ Drivers License No. _____

Address _____

City _____ State _____ Zip _____ Phone _____

Fee Simple Titleholder's Name (if other than owner) _____

Address _____

City _____ **State** _____ **Zip** _____ **Phone** _____

Contractor FRAME-PRO CONSTRUCTION License No. CBC 1507134

Address 17031 SW 150 CT

City MIAMI State FL Zip 33187 Phone _____

Cell# _____ Fax # _____

☐**Architect** ANTHONY LEON License No. AR0016752

Address 4300 BISCAYNE BLVD. SUITE # 604

City MIAMI State FL Zip 33137 Phone 305-438-9377

☐**Engineer** SAM LUVBKIN License No. PE34306

Address 2632 HOLLYWOOD BLVD. #103

City HOLLYWOOD State FLA Zip 33020 Phone 954-922-8845

PLEASE COMPLETE SHADED AREAS

BUILDING WORK PERMIT APPLICATIONS ARE AVAILABLE ON THE MIAMI BEACH WEBSITE AT :WWW.MIAMIBEACHFL.GOV

Bonding company Name_____

Address_____

City_____ State_____ Zip_____ Phone_____

Mortgage Lender's Name_____

Address_____

City_____ State_____ Zip_____ Phone_____

This application is hereby made to obtain a permit to do the work and installations as indicated. I certify that all work will be performed to meet the standards of all laws and construction regulations in this jurisdiction. I understand that **SEPARATE PERMITS** are required for ***Electrical, Mechanical, Plumbing, Signs, Swimming Pools, Spas, Windows, Sliding Glass Doors and Roofing***.

OWNER'S AFFIDAVIT: I certify that all the foregoing information is accurate and that all work will be done in compliance with all applicable laws regulating construction and Zoning.

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies or federal agencies.

*** If the contractor is going to be hired by the tenant, check here.** ☐

Signature of Owner or Agent	Signature of Tenant	Signature of Qualifier
J.R.K. GAINOR		DANILO RAMIREZ
Printed Name of Owner or Agent	Printed Name of Tenant	Printed Name of Qualifier
Date	Date	Date
Signature of Notary Public	Signature of Notary Public	Signature of Notary Public
Identification	Identification	Identification
Sworn to and subscribed before me this 2nd day of December 2007. (Seal)	Sworn to and subscribed before me this ___ day of _____ 20___. (Seal)	Sworn to and subscribed before me this 2nd day of December 2007. (Seal)

EVELYN RODRIGUEZ
MY COMMISSION # DD 648557
EXPIRES: April 7, 2011
Bonded Thru Notary Public Underwriters

EVELYN RODRIGUEZ
MY COMMISSION # DD 648557
EXPIRES: April 7, 2011
Bonded Thru Notary Public Underwriters

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. NOTICE OF COMMENCEMENT SHOULD BE FILED AT: 22 NW 1ST STREET, MIAMI, FL

STATE OF FLORIDA **COUNTY OF DADE**

Print Owner's Name

Owner's Signature

Sworn to and subscribed before me this 2nd day of December 2007, by: _____

(X) Personally Known ( ) Produced Identification - Type of Identification_____

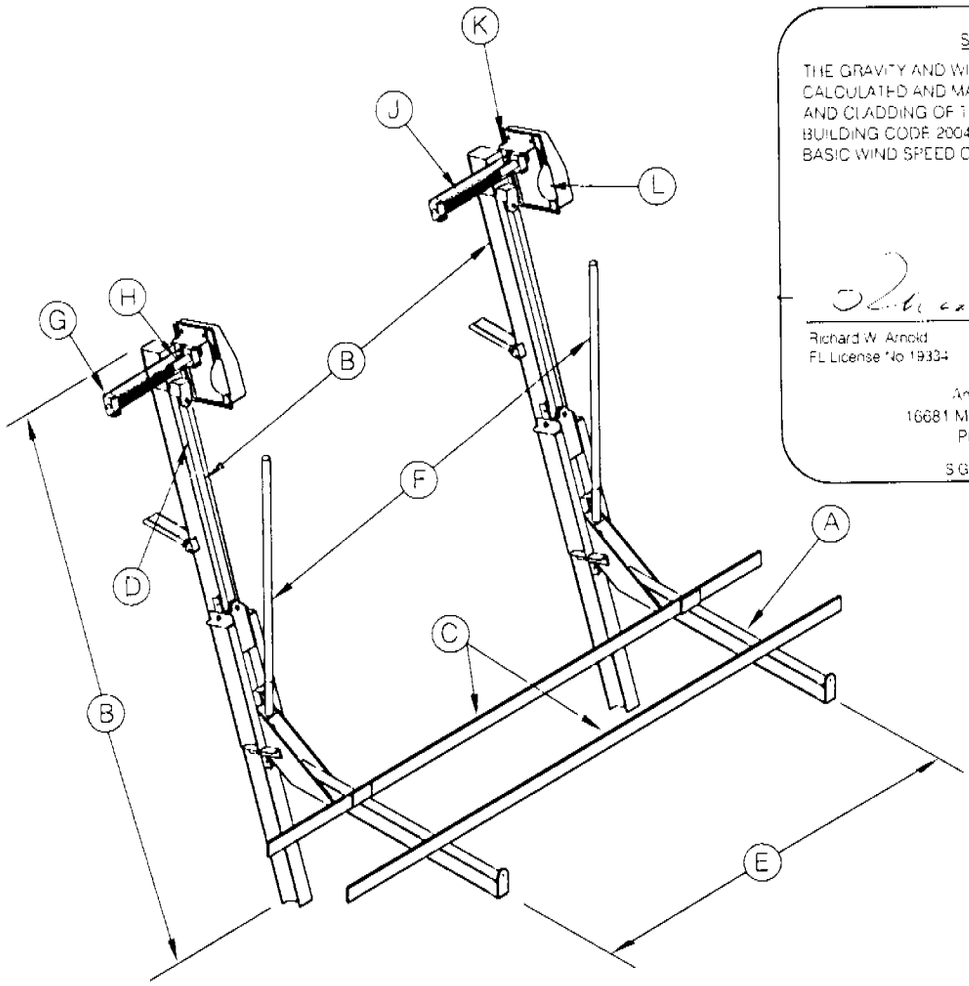
Signature of Notary Public (Seal)

EVELYN RODRIGUEZ
MY COMMISSION # DD 648557
EXPIRES: April 7, 2011
Bonded Thru Notary Public Underwriters

Application Approved By: _____

Permit Clerk

GOLDEN ENGINEERED ELEVATOR LIFT SPECIFICATIONS



STRUCTURAL ENGINEERING REVIEW

THE GRAVITY AND WIND LOADS FOR THIS CONSTRUCTION HAVE BEEN CALCULATED AND MAIN WIND FORCE RESISTING SYSTEM AND COMPONENTS AND CLADDING OF THIS STRUCTURE DESIGN DO COMPLY WITH THE FLORIDA BUILDING CODE 2004, SECT. 1609 FOR WIND PRESSURES GENERATED BY A BASIC WIND SPEED OF 150 MPH

Richard W. Arnold
FL License No 19334

Date

Arnold/Sanders Consulting Engineers, Inc
16681 McGregor Blvd, Suite 102, Fort Myers, FL 33908
Phone: 239-[illegible] Fax: 239-[illegible]

SIGNATURE NOT VALID WITHOUT RAISED SEAL

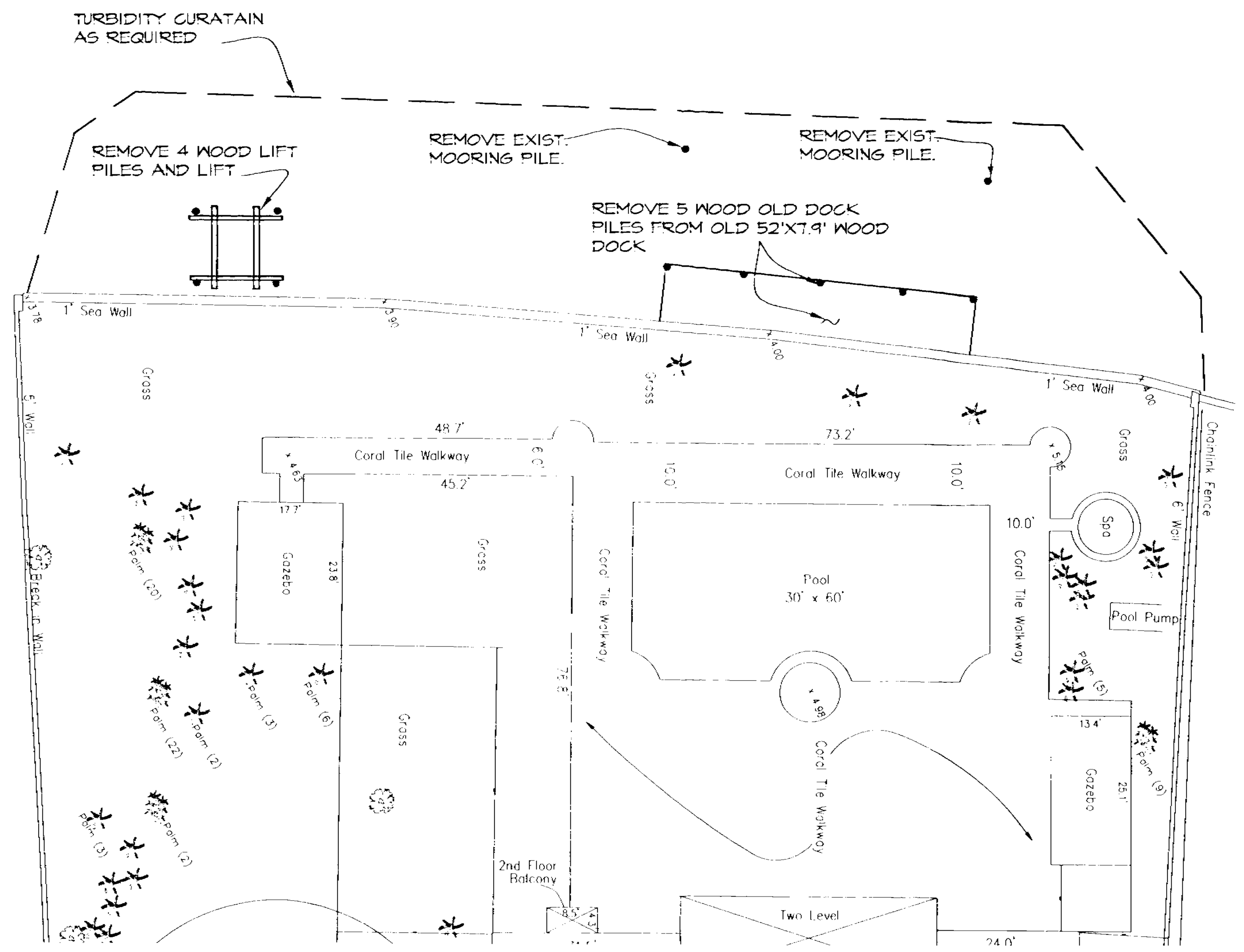
In addition to the requirement of this permit, there may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management's drainage restrictions applicable to this property that may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management's drainage restrictions, state agencies, or federal agencies.

The City of Miami Beach assumes no responsibility for accuracy of results from those plans which are approved subject to compliance with Federal, State, and Local Laws, Rules, and Regulations.

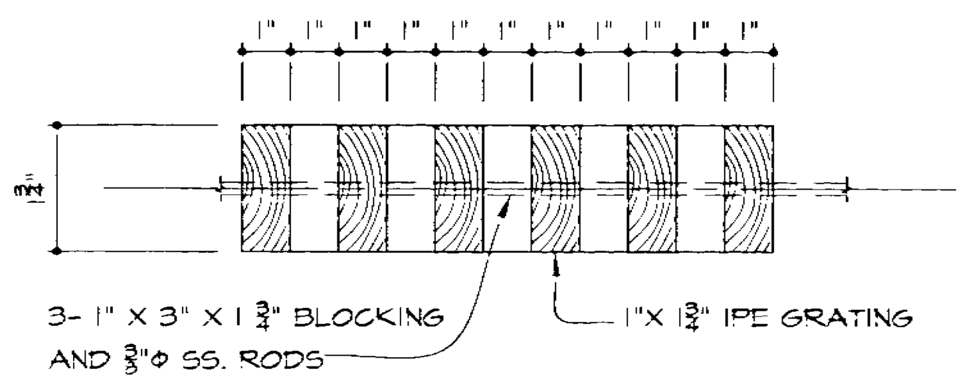
INCLINE MOUNT OR VERTICAL MOUNT

NOTE: THIS STRUCTURE WILL WITHSTAND WIND LOADS ASSOCIATED WITH WIND SPEEDS OF 150 MPH CALCULATED PER SECTION 1609 , FLORIDA BUILDING CODE, 2004, USING ASCE 7-02

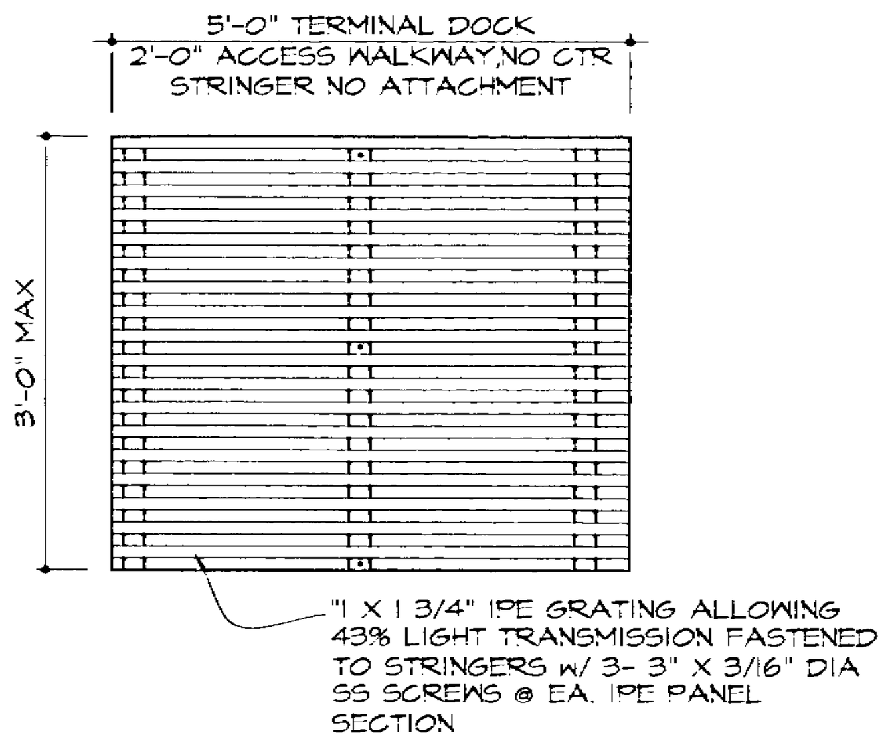
LIFT CAPACITY	(A) CRADLE I BEAM	(B) TRACK I BEAM	(C) BUNK BOARDS FEET	(D) CABLE SIZE	(E) TRACK SPREAD	(F) GUIDE POST HEIGHT	(G) BRGS	(H) DRIVE SHAFT	(J) WINDER DIA	(K) GEAR RATIO	(L) MOTOR HP/VOLTAGE	INCHES OF LIFT PER MIN
3,000#	2 - 6 H x 19 4 W x 29 x 7 LG @ 4.03#/FT	2 - 8 H x 23 5 W x 35 x 25 LG @ 6.18#/FT	2 - 2 x 8 x 144 ROUGH SAWN CARPETED	2 - 5/16" Ø x 20' S S 2 PART	7 THRU 10'	80"	4 - 2" EXTRUDED 6061 T6 ALUM	2 - 1/2" Ø SCHEDULE 80 ALUM PIPE	2 - 2 1/2" Ø SCHEDULE 80 ALUM PIPE	96:1	2 - 3/4 HP 120V/20A 240V/10A	13-1/2"
5,000# & 7,000#	2 - 8 H x 23 5 W x 35 x 8 LG @ 6.18#/FT	2 - 8 H x 25 5 W x 41 x 25 LG @ 7.02#/FT		2 - 5/16" Ø x 30' S S 2 PART							5,000# 2 - 3/4 HP 7,000# 2 - 1 HP	
10,000#	2 - 9 H x 27 5.5 W x 44 x 8 LG @ 8.36#/FT	2 - 9 H x 27 5.5 W x 44 x 25 LG @ 8.36#/FT		2 - 5/16" Ø x 30' S S 2 PART	8 THRU 11						2 - 3/4 HP 120V/20A 240V/10A	
12,000#	2 - 10 H x 25 6 W x 41 x 8 LG @ 8.65#/FT	2 - 10 H x 25 6 W x 41 x 25 LG @ 8.65#/FT		2 - 3/8" Ø x 38' S S 3 PART								9"
15,000#	2 - 10 H x 29 6 W x 50 x 9 LG @ 10.3#/FT	2 - 10 H x 29 6 W x 50 x 25 LG @ 10.3#/FT	2 - 3 x 10 x 192 ROUGH SAWN CARPETED		8 THRU 14	120"					2 - 1 HP 120V/20A 240V/10A	
17,000#	2 - 12 H x 29 7 W x 47 x 10' LG @ 11.7#/FT	2 - 12 H x 29 7 W x 47 x 10' LG @ 11.7#/FT		2 - 3/8" Ø x 50' S S 4 PART								6.75"
20,000#	2 - 12 H x 31 7 W x 62 x 10' LG @ 14.3#/FT	2 - 12 H x 31 7 W x 62 x 25 LG @ 14.3#/FT		2 - 3/8" Ø x 50' S S 4 PART								



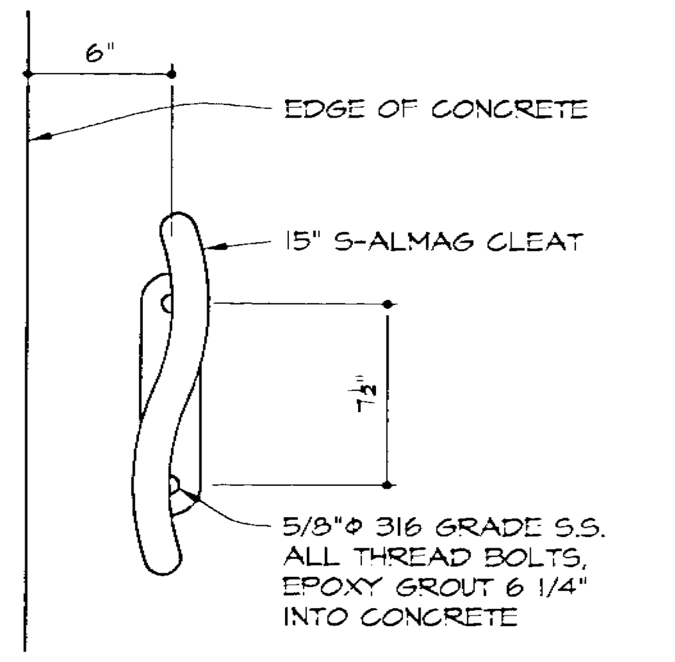
△ SITE PLAN 1" = 20'-0"
EXISTING CONDITIONS



IPE GRATING DETAIL 3" = 1'-0"



△ IPE GRATING DETAIL 1" = 1'-0"



TYP. 15" CLEAT DETAIL
1½" = 1'-0"

GENERAL NOTES △

PRECAST CONCRETE PILING

1. PRECAST CONCRETE PILES (W/ 5000 P.S.I. MIN. CONCRETE) FOR 12" x 12" PILES W/ (4) 7/16"Φ 270 K.S.I. ASTM A416 STRANDS, 14"X14" PILES FOR (8) 1/2"Φ 270 K.S.I. ASTM A416 STRANDS. EXTEND PILE REINF 12" MIN INTO PILE CAPS. LOW-LAX STRANDS W/ 2 1/2" MIN. CONCRETE COVER TO TIES. DRIVEN TO A MINIMUM BEARING CAPACITY OF (12"X12") 25 TONS, (14"X14") 35 TONS WITH 12' MIN. PENETRATION INTO FIRM MATERIAL BELOW SILT LAYER.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

CONCRETE AND REINFORCING STEEL

1. ALL CONCRETE (EXCEPT PRECAST PILES) SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI (.40 W/C RATIO) AT THE END OF 28 DAYS. FOUR (4) CONCRETE CYLINDERS SHALL BE TAKEN FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF AND SHALL BE TESTED AT 3, 7 AND 28 DAYS. SLUMP SHALL NOT EXCEED 5" (± 1").
2. ALL REINFORCEMENT SHALL BE 60000 PSI MINIMUM YIELD NEW BILLET STEEL IN ACCORDANCE WITH ASTM A615 GRADE 60. ALL BAR LAPS SHALL BE A MINIMUM OF 48 BAR DIAMETERS. PLACING OF REINFORCEMENT SHALL CONFORM TO THE LATEST ACI AND MANUAL OF STANDARD OF PRACTICE CODES.
3. ALL CONCRETE SHALL BE PLACED WITHIN 90 MINUTES FROM BATCH TIME, AND VIBRATED AS REQUIRED BY THE ACI MANUAL OF CONCRETE PRACTICE. TEMPERATURE OF CONCRETE AT THE TIME OF PLACEMENT SHALL BE BETWEEN 75° F. AND 100° F.
4. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" (EXCEPT STAIR NOSINGS), OR AS SHOWN ON THE PLANS.

WOOD

1. WOOD GRATING SHALL BE IPE 1" X 1 3/4" , ALLOWING 43% LIGHT TRANSMISSION, AS PLAN & SECTIONS. (F_b=2000 PSI MIN.)
2. PRIMARY WOOD FRAMING MEMBERS SHALL BE NO. 2 PRESSURE TREATED SOUTHERN PINE OR BETTER. (2X12 & 4X12 F_b=975 PSI MIN, 3X10 F_b=1050 PSI MIN) 6X6 = #1 SOUTHERN PINE (F_b= 1350 PSI)

WOOD PILING

1. WOOD PILES SHALL BE 12" DIAMETER SOUTH AMERICAN GREENHEART PILES AS SHOWN ON THE PLANS, DRIVEN TO 12 FEET MINIMUM PENTRATION INTO FIRM MATERIAL

BOLTS

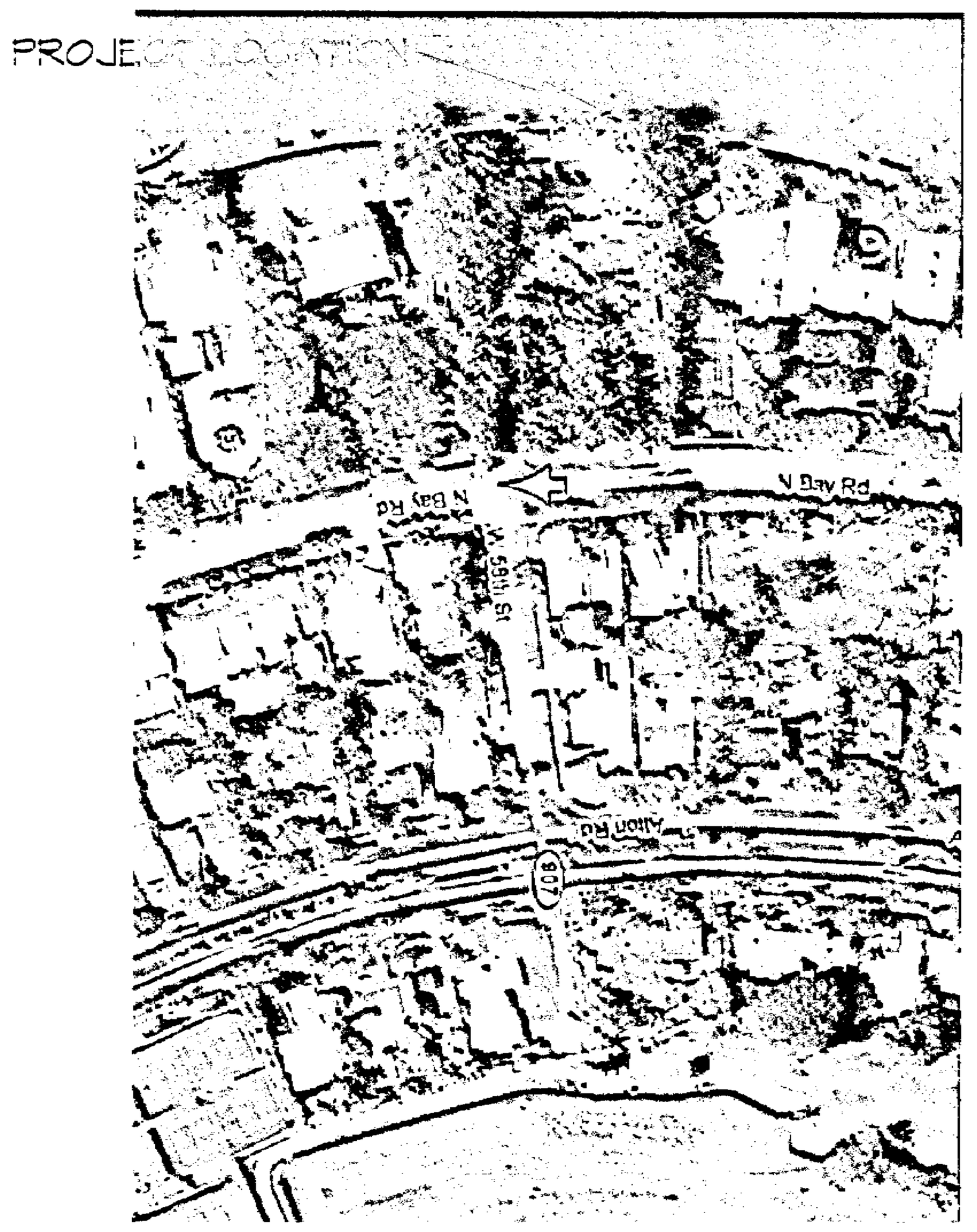
1. ALL BOLTS, WASHERS AND NUTS SHALL BE TYPE 304 STAINLESS STEEL.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

GENERAL

1. ELEVATIONS SHOWN REFER TO THE NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929.
2. ALL DIMENSIONS ON PLANS ARE SUBJECT TO VERIFICATION IN THE FIELD.
3. IT IS THE INTENT OF THESE PLANS TO BE IN ACCORDANCE WITH APPLICABLE CODES AND AUTHORITIES HAVING JURISDICTION. ANY DISCREPANCIES BETWEEN THESE PLANS AND APPLICABLE CODES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH WORK.
4. IT IS THE INTENT OF THESE PLANS AND THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH LOCAL, STATE, AND FEDERAL ENVIRONMENTAL PERMITS ISSUED FOR THIS PROJECT. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE AND GOVERN HIMSELF BY ALL PROVISIONS OF THESE PERMITS.
5. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 2004 EDITION.

DESIGN LOADS

1. DOCK LL ....60 PSF
2. DOCK DESIGN FOR WIND LOADING WITHOUT BOAT MOORING TO DOCK IS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004 EDITION AND SECTION 6 OF ASCE 7-98. WIND SPEED 146 MPH, EXPOSURE CATEGORY "C" FOR COMPONENTS AND CLADDING, EXPOSURE CATEGORY "C" FOR MWFRS. IMPORTANCE FACTOR, I = 1.0. INTERNAL PRESSURE COEFFICIENT = 0.18
3. 50 MPH WIND SPEED APPLIED TO A 55' VESSEL WITH A 42" DRAFT. AT SLIP #1



LOCATION PLAN NTS

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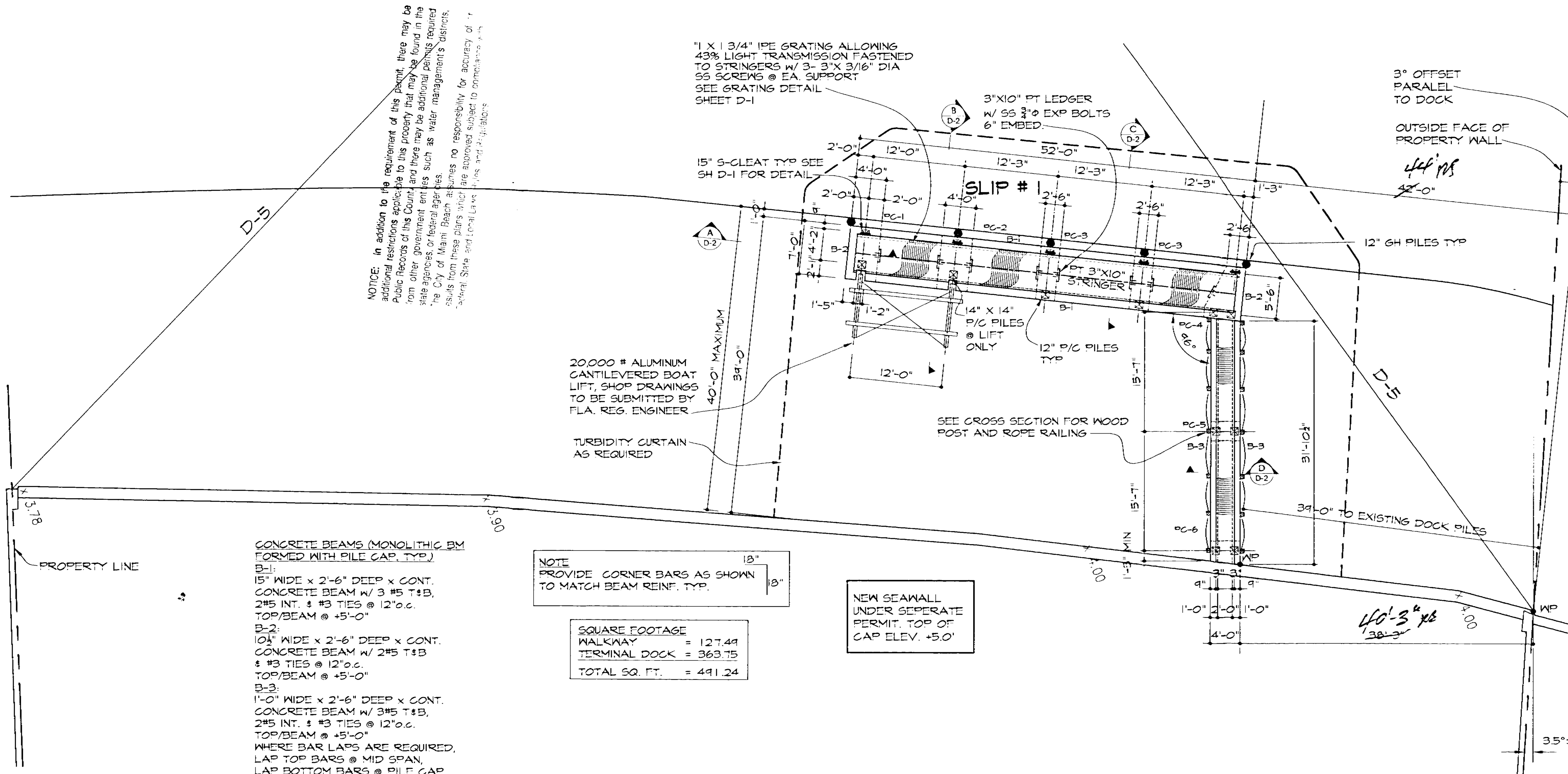
ENVIRONMENTAL RESOURCES



CONCRETE BEAMS (MONOLITHIC BM FORMED WITH PILE CAP. TYP.)
B-1:
15" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 3 #5 T&B, 2#5 INT. & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
B-2:
10½" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 2#5 T&B & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
B-3:
1'-0" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 3#5 T&B, 2#5 INT. & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
WHERE BAR LAPS ARE REQUIRED, LAP TOP BARS @ MID SPAN, LAP BOTTOM BARS @ PILE CAP

NOTE
PROVIDE CORNER BARS AS SHOWN TO MATCH BEAM REINF. TYP. (18" / 18")

SQUARE FOOTAGE	
WALKWAY	= 127.49
TERMINAL DOCK	= 363.75
TOTAL SQ. FT.	= 491.24

NEW SEAWALL UNDER SEPERATE PERMIT. TOP OF CAP ELEV. +5.0'

△ NEW PROPOSED DOCK PLAN 1" = 10'-0"

ROBERT E. SAMARA P.E.,P.A.
Consulting Engineers
PE # 19649
7901 S.W. 67th Avenue., Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
△ 11-27-07 REV PER BMS & PILE CAPS
△ 12-17-07 MOVED DOCK & REV DETAILS

DATE: 11-07-07
DWN BY: A.B.
JOB NO: 07-70
SHEET: D-1
OF 2 SHEETS

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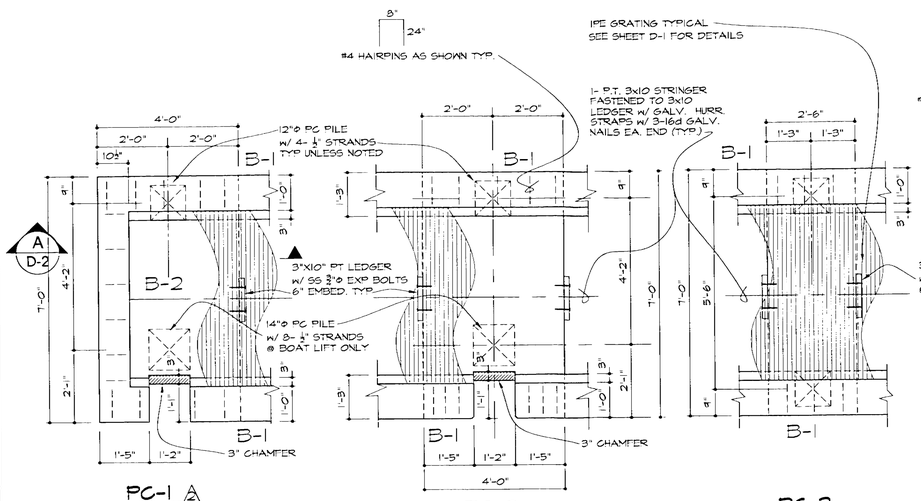
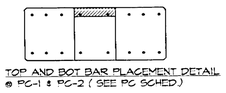
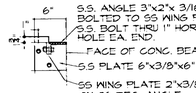
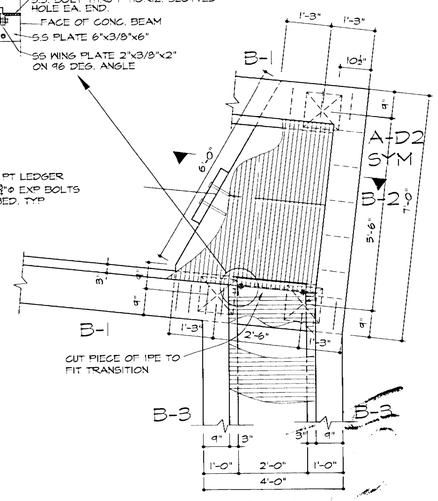
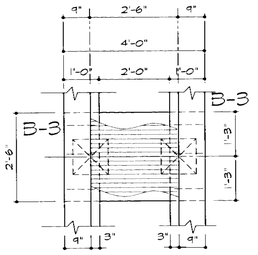
Environmental Resources Regulation Division

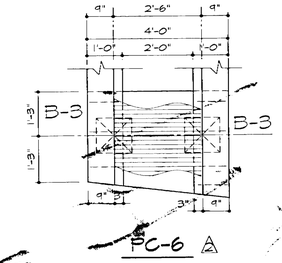


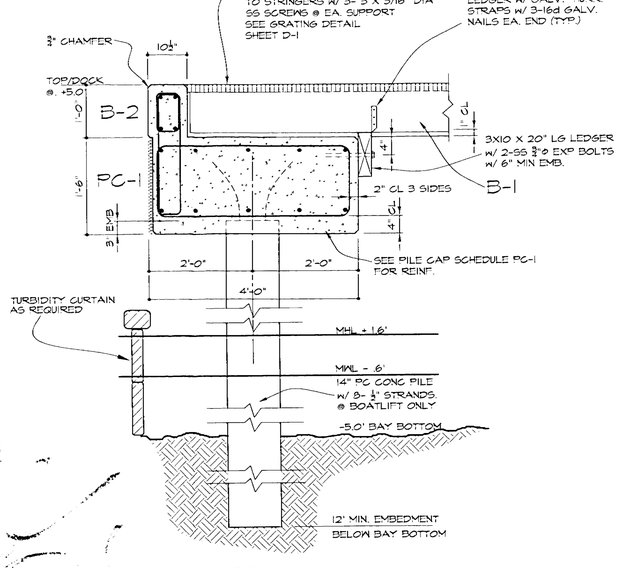

TOP AND BOT BAR PLACEMENT DETAIL
@ PC-1 & PC-2 (SEE PC SCHED.)







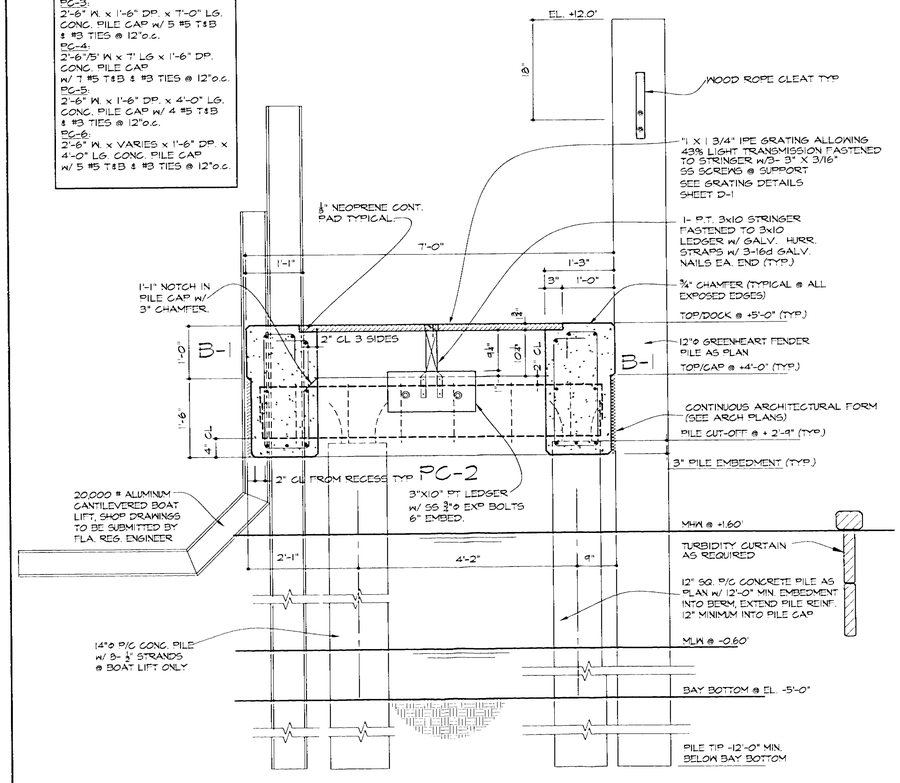




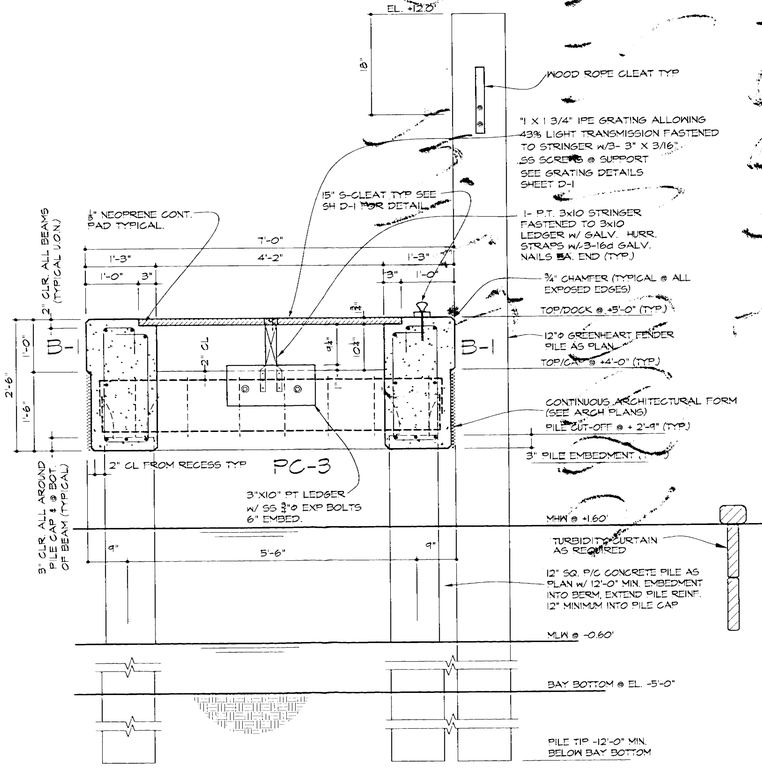
SECTION A-D2 ¾" = 1'-0"

PILE CAP DETAILS 1/2" = 1'-0"

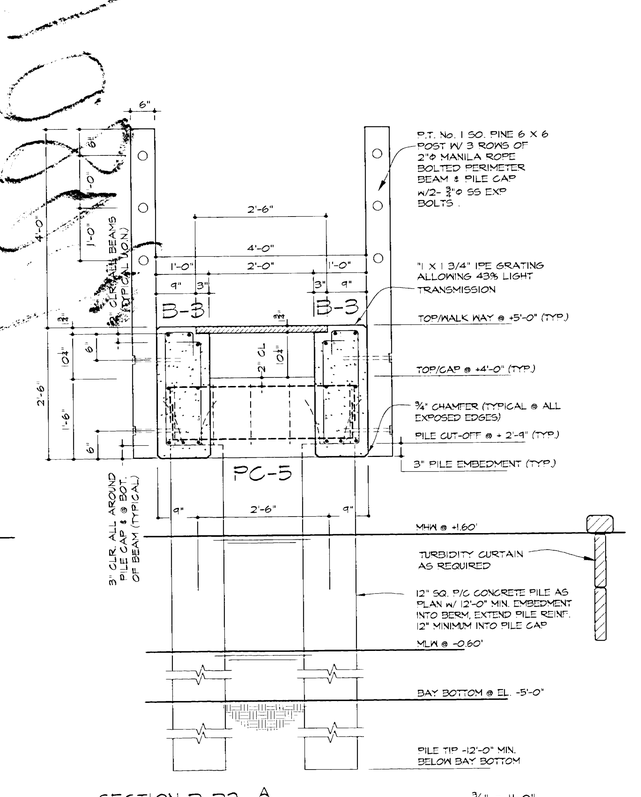
CONCRETE PILE CAPS:
PC-1:
4'-0" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 8 #5 T&B SEE BAR PLACEMENT DETAIL.
PC-2:
4'-0" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 8 #5 T&B & #3 TIES @ 12"o.c. SEE BAR PLACEMENT DETAIL.
PC-3:
2'-6" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 5 #5 T&B & #3 TIES @ 12"o.c.
PC-4:
2'-6"/5' W x 7' LG x 1'-6" DP. CONC. PILE CAP w/ 7 #5 T&B & #3 TIES @ 12"o.c.
PC-5
2'-6" W. x 1'-6" DP. x 4'-0" LG. CONC. PILE CAP w/ 4 #5 T&B & #3 TIES @ 12"o.c.
PC-6:
2'-6" W. x VARIES x 1'-6" DP. x 4'-0" LG. CONC. PILE CAP w/ 5 #5 T&B & #3 TIES @ 12"o.c.



SECTION B-D2 ¾" = 1'-0"



SECTION C-D2 ¾" = 1'-0"



SECTION D-D2 A ¾" = 1'-0"

ROBERT E. SAMARA P.E., P.A.
Consulting Engineers
PE# 19649
7901 S.W. 67th Avenue, Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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ENVIRONMENTAL RESOURCES REGULATION DIVISION

DERM COAST:
PRELIMINARY APPR.
NAME
DATE 1/5/08

JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
A 11-27-07
REV PER BMS
& PILE CAPS
A 12-17-07
MOVED DOCK
& ADJ PILE CAPS

DATE: 11-07-07

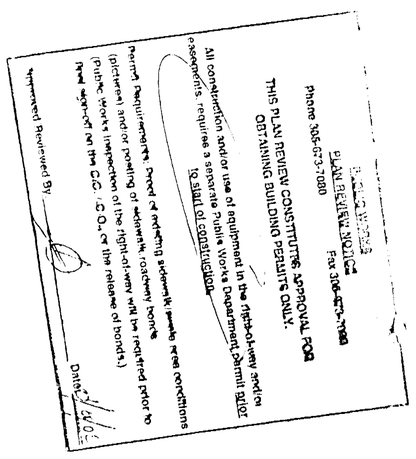
DWN BY: A.B.

JOB NO: 07-70

SHEET: D-2
OF 2 SHEETS

RECEIVED
DEC 2 8 2007
DERM
Environmental Resources Regulation Division

48 HOURS PRIOR TO EXCAVATING
CONTRACTOR SHALL CALL FOR LOCATION
OF UNDERGROUND UTILITIES
SUNSHINE ONE-CALL 1-800-432-4770
CITY OF MIAMI BEACH 305-673-7080

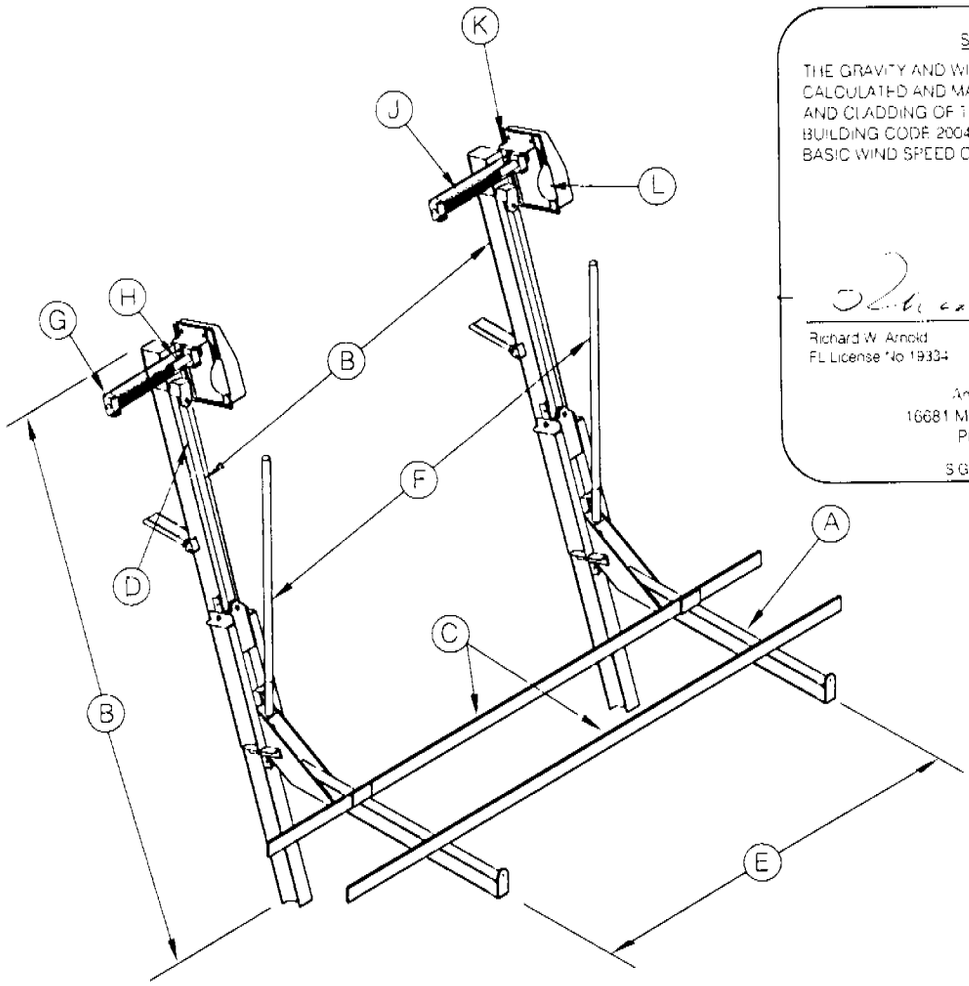


B0801356

5800 N. Bay Rd.

Office Copy

GOLDEN ENGINEERED ELEVATOR LIFT SPECIFICATIONS



STRUCTURAL ENGINEERING REVIEW

THE GRAVITY AND WIND LOADS FOR THIS CONSTRUCTION HAVE BEEN CALCULATED AND MAIN WIND FORCE RESISTING SYSTEM AND COMPONENTS AND CLADDING OF THIS STRUCTURE DESIGN DO COMPLY WITH THE FLORIDA BUILDING CODE 2004, SECT. 1609 FOR WIND PRESSURES GENERATED BY A BASIC WIND SPEED OF 150 MPH

Richard W. Arnold
FL License No 19334

Date

Arnold/Sanders Consulting Engineers, Inc
16681 McGregor Blvd, Suite 102, Fort Myers, FL 33908
Phone: 239-[illegible] Fax: 239-[illegible]

SIGNATURE NOT VALID WITHOUT RAISED SEAL

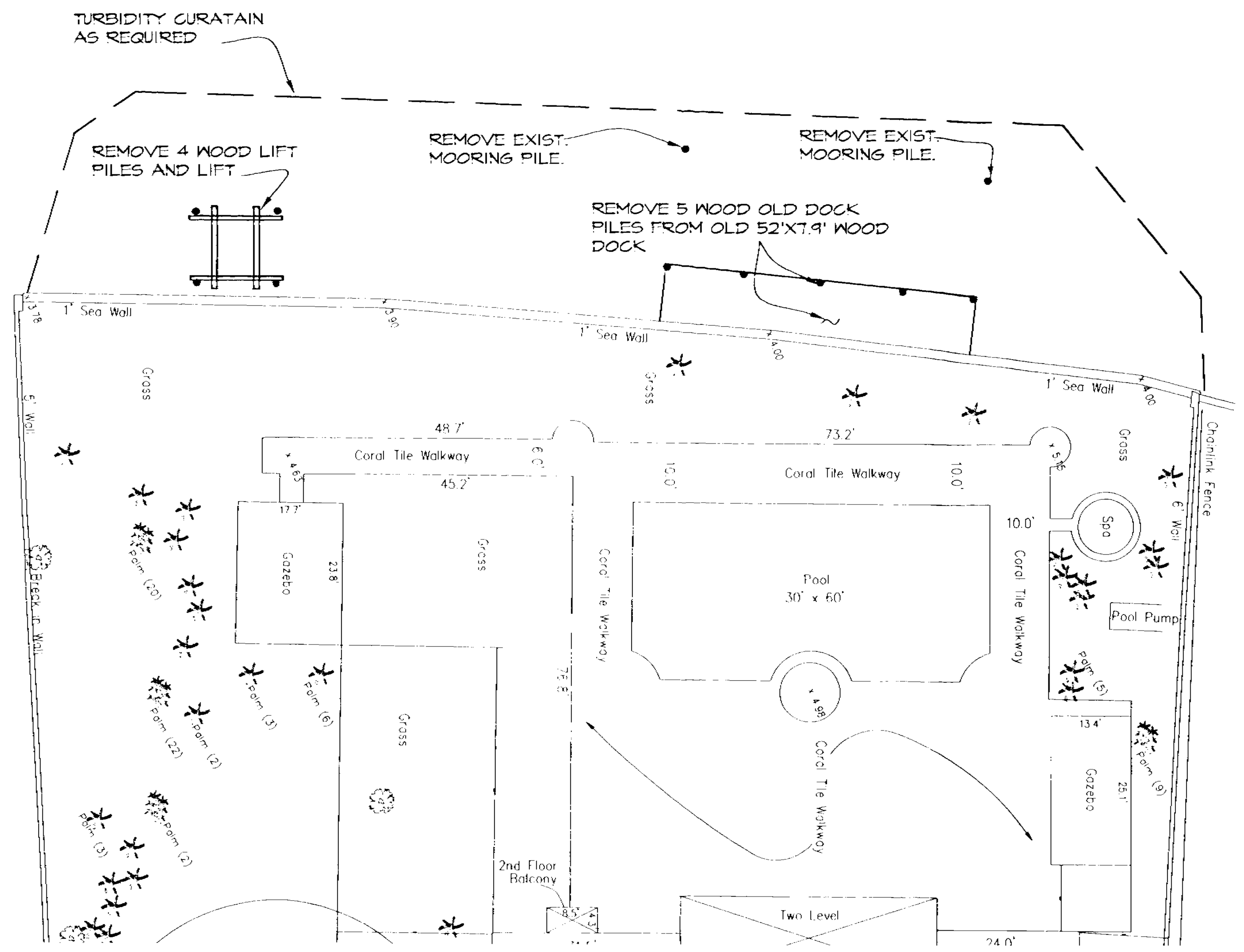
In addition to the requirement of this permit, there may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management's drainage restrictions applicable to this property that may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management's drainage restrictions, state agencies, or federal agencies.

The City of Miami Beach assumes no responsibility for accuracy of results from those plans which are approved subject to compliance with Federal, State, and Local Laws, Rules, and Regulations.

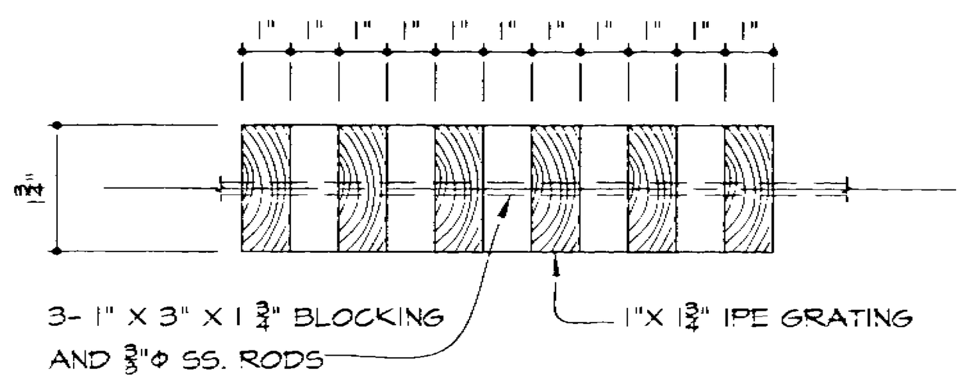
INCLINE MOUNT OR VERTICAL MOUNT

NOTE: THIS STRUCTURE WILL WITHSTAND WIND LOADS ASSOCIATED WITH WIND SPEEDS OF 150 MPH CALCULATED PER SECTION 1609 , FLORIDA BUILDING CODE, 2004, USING ASCE 7-02

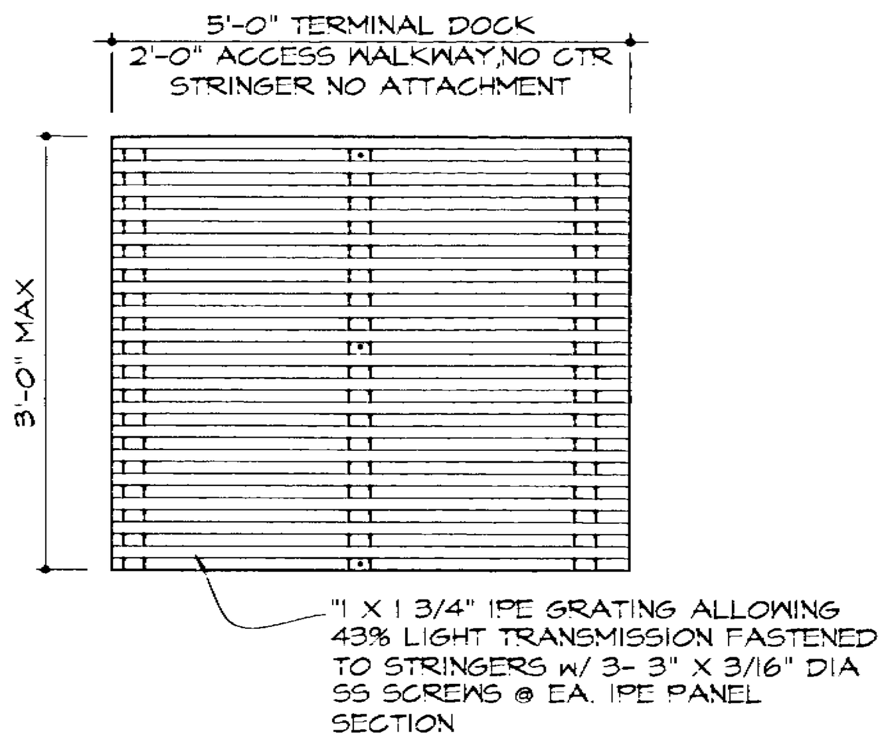
LIFT CAPACITY	(A) CRADLE I BEAM	(B) TRACK I BEAM	(C) BUNK BOARDS FEET	(D) CABLE SIZE	(E) TRACK SPREAD	(F) GUIDE POST HEIGHT	(G) BRGS	(H) DRIVE SHAFT	(J) WINDER DIA	(K) GEAR RATIO	(L) MOTOR HP/VOLTAGE	INCHES OF LIFT PER MIN
3,000#	2 - 6 H x 19 4 W x 29 x 7 LG @ 4.03#/FT	2 - 8 H x 23 5 W x 35 x 25 LG @ 6.18#/FT	2 - 2 x 8 x 144 ROUGH SAWN CARPETED	2 - 5/16" Ø x 20' S S 2 PART	7 THRU 10'	80"	4 - 2" EXTRUDED 6061 T6 ALUM	2 - 1/2" Ø SCHEDULE 80 ALUM PIPE	2 - 2 1/2" Ø SCHEDULE 80 ALUM PIPE	96:1	2 - 3/4 HP 120V/20A 240V/10A	13-1/2"
5,000# & 7,000#	2 - 8 H x 23 5 W x 35 x 8 LG @ 6.18#/FT	2 - 8 H x 25 5 W x 41 x 25 LG @ 7.02#/FT		2 - 5/16" Ø x 30' S S 2 PART							5,000# 2 - 3/4 HP 7,000# 2 - 1 HP	
10,000#	2 - 9 H x 27 5.5 W x 44 x 8 LG @ 8.36#/FT	2 - 9 H x 27 5.5 W x 44 x 25 LG @ 8.36#/FT		2 - 5/16" Ø x 30' S S 2 PART	8 THRU 11						2 - 3/4 HP 120V/20A 240V/10A	
12,000#	2 - 10 H x 25 6 W x 41 x 8 LG @ 8.65#/FT	2 - 10 H x 25 6 W x 41 x 25 LG @ 8.65#/FT		2 - 3/8" Ø x 38' S S 3 PART								9"
15,000#	2 - 10 H x 29 6 W x 50 x 9 LG @ 10.3#/FT	2 - 10 H x 29 6 W x 50 x 25 LG @ 10.3#/FT	2 - 3 x 10 x 192 ROUGH SAWN CARPETED		8 THRU 14	120"					2 - 1 HP 120V/20A 240V/10A	
17,000#	2 - 12 H x 29 7 W x 47 x 10' LG @ 11.7#/FT	2 - 12 H x 29 7 W x 47 x 10' LG @ 11.7#/FT		2 - 3/8" Ø x 50' S S 4 PART								6.75"
20,000#	2 - 12 H x 31 7 W x 62 x 10' LG @ 14.3#/FT	2 - 12 H x 31 7 W x 62 x 25 LG @ 14.3#/FT		2 - 3/8" Ø x 50' S S 4 PART								



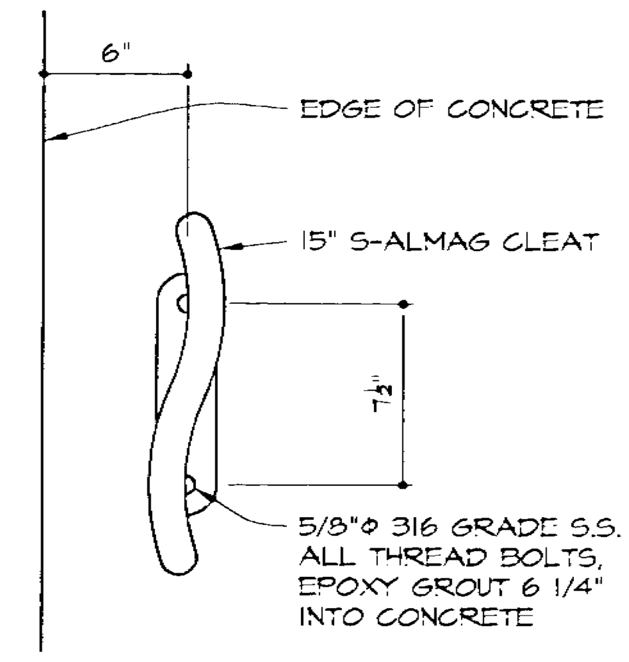
△ SITE PLAN 1" = 20'-0"
EXISTING CONDITIONS



IPE GRATING DETAIL 3" = 1'-0"



△ IPE GRATING DETAIL 1" = 1'-0"



TYP. 15" CLEAT DETAIL
1½" = 1'-0"

GENERAL NOTES △

PRECAST CONCRETE PILING

1. PRECAST CONCRETE PILES (W/ 5000 P.S.I. MIN. CONCRETE) FOR 12" x 12" PILES W/ (4) 7/16"ø 270 K.S.I. ASTM A416 STRANDS, 14"X14" PILES FOR (8) 1/2"ø 270 K.S.I. ASTM A416 STRANDS. EXTEND PILE REINF 12" MIN INTO PILE CAPS. LOW-LAX STRANDS W/ 2 1/2" MIN. CONCRETE COVER TO TIES. DRIVEN TO A MINIMUM BEARING CAPACITY OF (12"X12") 25 TONS, (14"X14") 35 TONS WITH 12' MIN. PENETRATION INTO FIRM MATERIAL BELOW SILT LAYER.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

CONCRETE AND REINFORCING STEEL

1. ALL CONCRETE (EXCEPT PRECAST PILES) SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI (.40 W/C RATIO) AT THE END OF 28 DAYS. FOUR (4) CONCRETE CYLINDERS SHALL BE TAKEN FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF AND SHALL BE TESTED AT 3, 7 AND 28 DAYS. SLUMP SHALL NOT EXCEED 5" (± 1").
2. ALL REINFORCEMENT SHALL BE 60000 PSI MINIMUM YIELD NEW BILLET STEEL IN ACCORDANCE WITH ASTM A615 GRADE 60. ALL BAR LAPS SHALL BE A MINIMUM OF 48 BAR DIAMETERS. PLACING OF REINFORCEMENT SHALL CONFORM TO THE LATEST ACI AND MANUAL OF STANDARD OF PRACTICE CODES.
3. ALL CONCRETE SHALL BE PLACED WITHIN 90 MINUTES FROM BATCH TIME, AND VIBRATED AS REQUIRED BY THE ACI MANUAL OF CONCRETE PRACTICE. TEMPERATURE OF CONCRETE AT THE TIME OF PLACEMENT SHALL BE BETWEEN 75° F. AND 100° F.
4. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" (EXCEPT STAIR NOSINGS), OR AS SHOWN ON THE PLANS.

WOOD

1. WOOD GRATING SHALL BE IPE 1" X 1 3/4", ALLOWING 43% LIGHT TRANSMISSION, AS PLAN & SECTIONS. (F_b=2000 PSI MIN.)
2. PRIMARY WOOD FRAMING MEMBERS SHALL BE NO. 2 PRESSURE TREATED SOUTHERN PINE OR BETTER. (2X12 & 4X12 F_b=975 PSI MIN, 3X10 F_b=1050 PSI MIN) 6X6 = #1 SOUTHERN PINE (F_b= 1350 PSI)

WOOD PILING

1. WOOD PILES SHALL BE 12" DIAMETER SOUTH AMERICAN GREENHEART PILES AS SHOWN ON THE PLANS, DRIVEN TO 12 FEET MINIMUM PENTRATION INTO FIRM MATERIAL

BOLTS

1. ALL BOLTS, WASHERS AND NUTS SHALL BE TYPE 304 STAINLESS STEEL.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

GENERAL

1. ELEVATIONS SHOWN REFER TO THE NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929.
2. ALL DIMENSIONS ON PLANS ARE SUBJECT TO VERIFICATION IN THE FIELD.
3. IT IS THE INTENT OF THESE PLANS TO BE IN ACCORDANCE WITH APPLICABLE CODES AND AUTHORITIES HAVING JURISDICTION. ANY DISCREPANCIES BETWEEN THESE PLANS AND APPLICABLE CODES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH WORK.
4. IT IS THE INTENT OF THESE PLANS AND THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH LOCAL, STATE, AND FEDERAL ENVIRONMENTAL PERMITS ISSUED FOR THIS PROJECT. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE AND GOVERN HIMSELF BY ALL PROVISIONS OF THESE PERMITS.
5. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 2004 EDITION.

DESIGN LOADS

1. DOCK LL ....60 PSF

2. DOCK DESIGN FOR WIND LOADING WITHOUT BOAT MOORING TO DOCK IS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004 EDITION AND SECTION 6 OF ASCE 7-98. WIND SPEED 146 MPH, EXPOSURE CATEGORY "C" FOR COMPONENTS AND CLADDING, EXPOSURE CATEGORY "C" FOR MWFRS. IMPORTANCE FACTOR, I = 1.0. INTERNAL PRESSURE COEFFICIENT = 0.18
3. 50 MPH WIND SPEED APPLIED TO A 55' VESSEL WITH A 42" DRAFT. AT SLIP #1

LOCATION PLAN NTS

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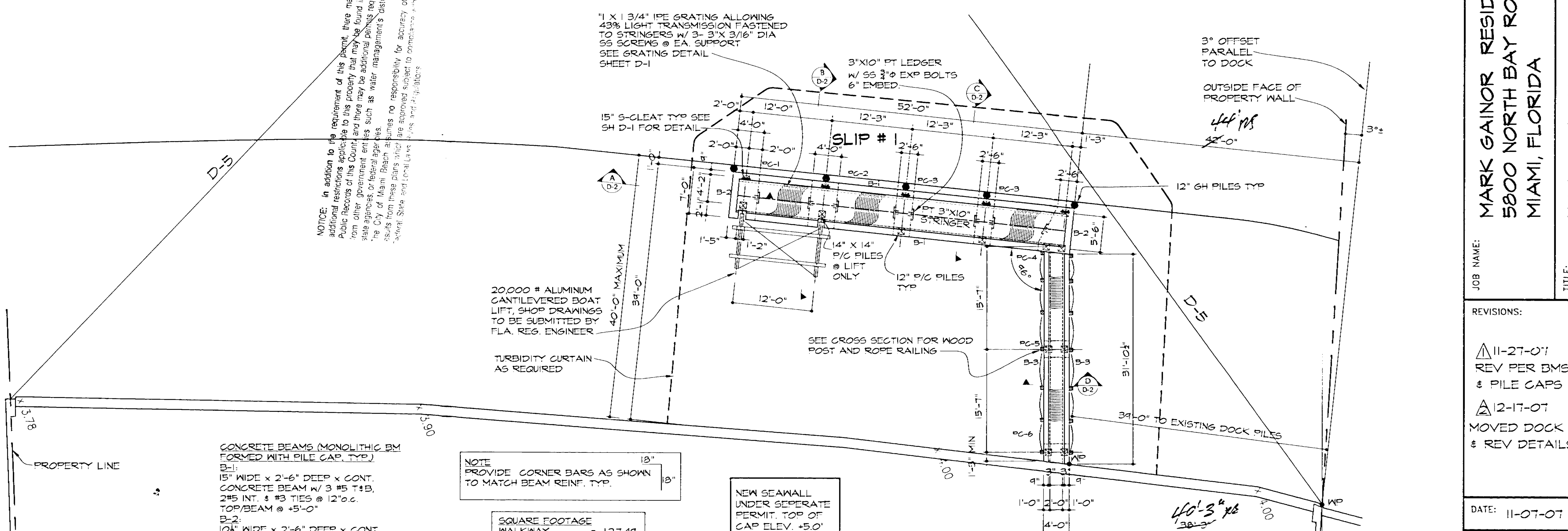
ENVIRONMENTAL RESOURCES



CONCRETE BEAMS (MONOLITHIC BM FORMED WITH PILE CAP. TYP.)
B-1:
15" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 3 #5 T&B, 2#5 INT. & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
B-2:
10½" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 2#5 T&B & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
B-3:
1'-0" WIDE x 2'-6" DEEP x CONT. CONCRETE BEAM W/ 3#5 T&B, 2#5 INT. & #3 TIES @ 12"o.c. TOP/BEAM @ +5'-0"
WHERE BAR LAPS ARE REQUIRED, LAP TOP BARS @ MID SPAN, LAP BOTTOM BARS @ PILE CAP

NOTE
PROVIDE CORNER BARS AS SHOWN TO MATCH BEAM REINF. TYP. (18" / 18")

SQUARE FOOTAGE	
WALKWAY	= 127.49
TERMINAL DOCK	= 363.75
TOTAL SQ. FT.	= 491.24

NEW SEAWALL UNDER SEPERATE PERMIT. TOP OF CAP ELEV. +5.0'

△ NEW PROPOSED DOCK PLAN 1" = 10'-0"

ROBERT E. SAMARA P.E., P.A.
Consulting Engineers
PE # 19649
7901 S.W. 67th Avenue, Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
△ 11-27-07 REV PER BMS & PILE CAPS
△ 12-17-07 MOVED DOCK & REV DETAILS

DATE: 11-07-07
DWN BY: A.B.
JOB NO: 07-70
SHEET: D-1
OF 2 SHEETS

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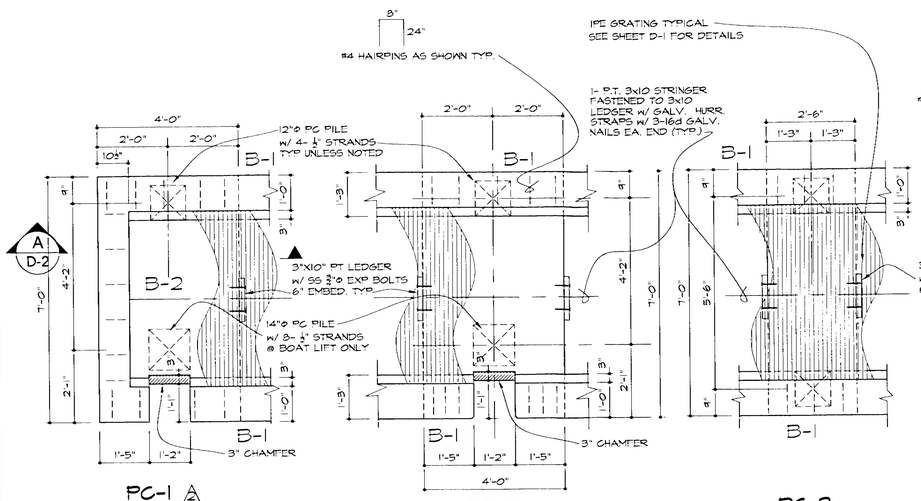
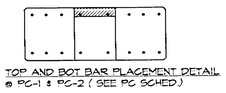
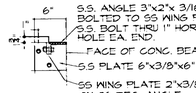
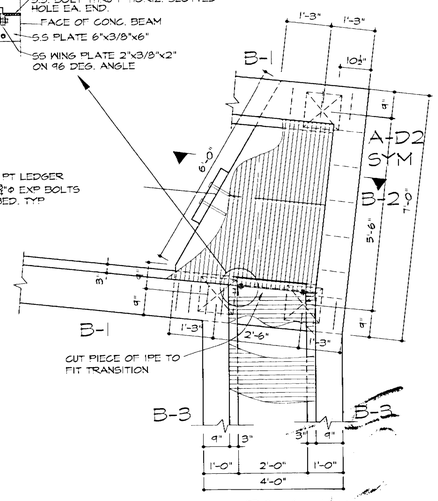
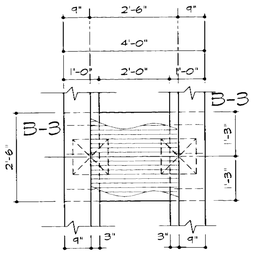
DERM
Environmental Resources Regulation Division

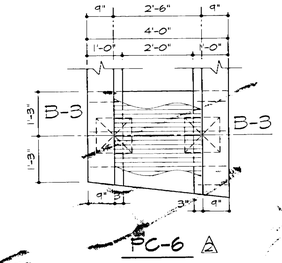


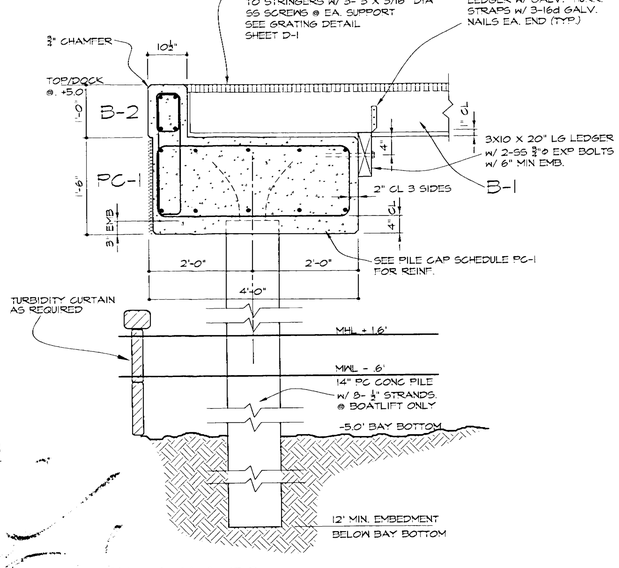

TOP AND BOT BAR PLACEMENT DETAIL
@ PC-1 & PC-2 (SEE PC SCHED.)







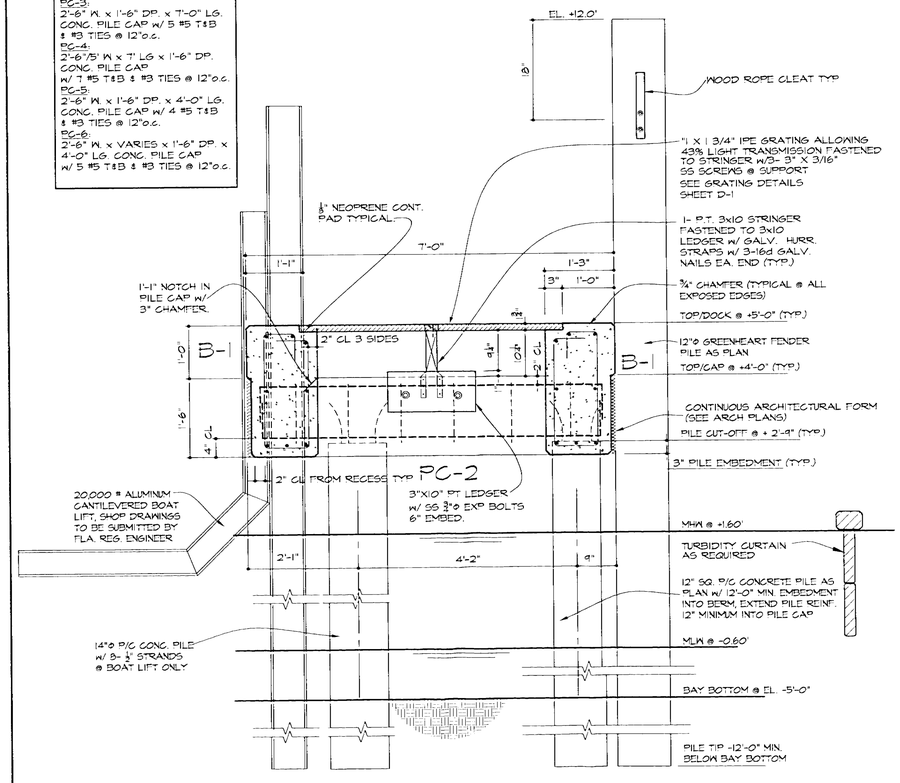




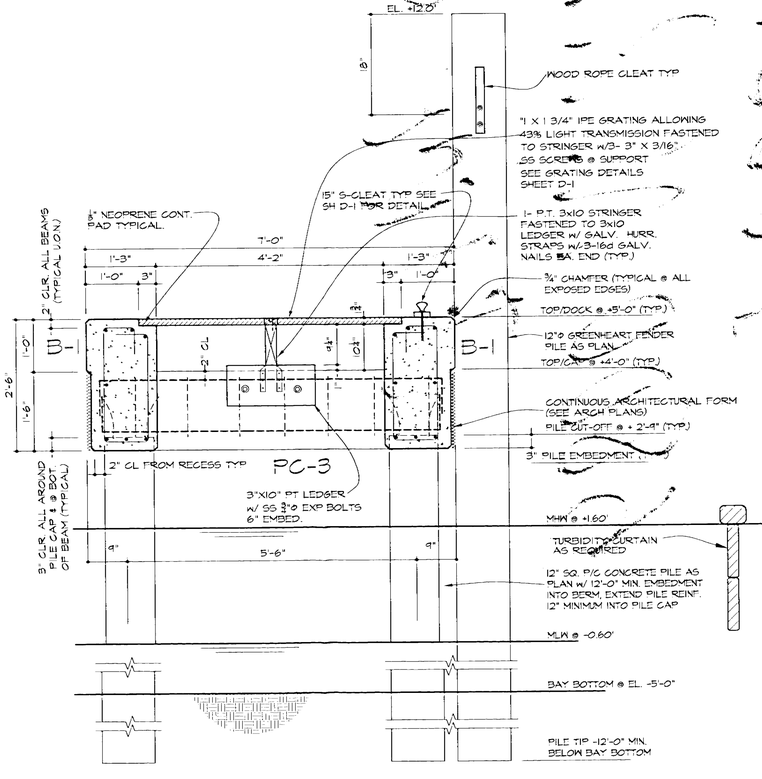
SECTION A-D2 ¾" = 1'-0"

PILE CAP DETAILS 1/2" = 1'-0"

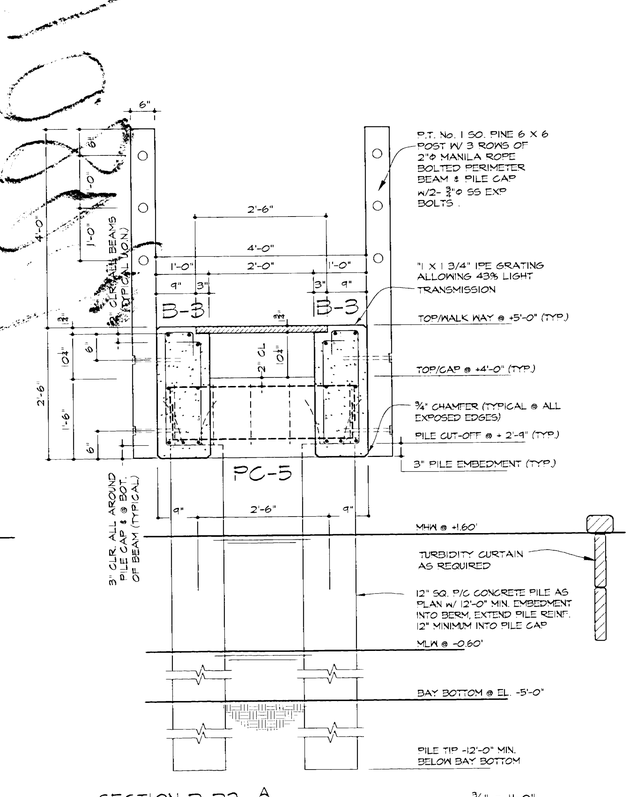
CONCRETE PILE CAPS:
PC-1:
4'-0" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 8 #5 T&B SEE BAR PLACEMENT DETAIL.
PC-2:
4'-0" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 8 #5 T&B & #3 TIES @ 12"o.c. SEE BAR PLACEMENT DETAIL.
PC-3:
2'-6" W. x 1'-6" DP. x 7'-0" LG. CONC. PILE CAP w/ 5 #5 T&B & #3 TIES @ 12"o.c.
PC-4:
2'-6"/5' W x 7' LG x 1'-6" DP. CONC. PILE CAP w/ 7 #5 T&B & #3 TIES @ 12"o.c.
PC-5
2'-6" W. x 1'-6" DP. x 4'-0" LG. CONC. PILE CAP w/ 4 #5 T&B & #3 TIES @ 12"o.c.
PC-6:
2'-6" W. x VARIES x 1'-6" DP. x 4'-0" LG. CONC. PILE CAP w/ 5 #5 T&B & #3 TIES @ 12"o.c.



SECTION B-D2 ¾" = 1'-0"



SECTION C-D2 ¾" = 1'-0"



SECTION D-D2 A ¾" = 1'-0"

ROBERT E. SAMARA P.E., P.A.
Consulting Engineers
PE# 19649
7901 S.W. 67th Avenue, Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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ENVIRONMENTAL RESOURCES REGULATION DIVISION

DERM COAST:
PRELIMINARY APPR.
NAME
DATE 1/5/08

JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
A 11-27-07
REV PER BMS
& PILE CAPS
A 12-17-07
MOVED DOCK
& ADJ PILE CAPS

DATE: 11-07-07

DWN BY: A.B.

JOB NO: 07-70

SHEET: D-2
OF 2 SHEETS

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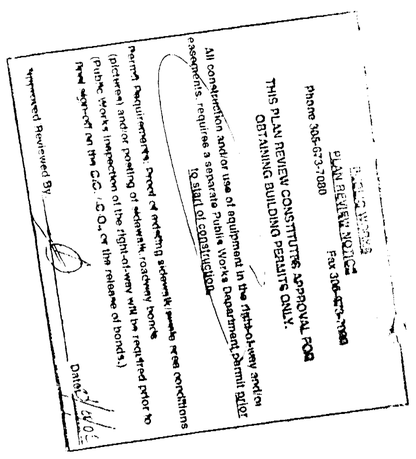
DERM
Environmental Resources Regulation Division

48 HOURS PRIOR TO EXCAVATING
CONTRACTOR SHALL CALL FOR LOCATION
OF UNDERGROUND UTILITIES
SUNSHINE ONE-CALL 1-800-432-4770
CITY OF MIAMI BEACH 305-673-7080



B0801356

5800 N. Bay Rd.

Office Copy

CITY OF MIAMI BEACH
Building Department
1700 Convention Ctr Drive, 2nd Floor
Miami Beach, Florida 33139

Inspections: (305) 673-7370 **Office: (305) 673-7610**

Bldg Small Work Permit

01-14-2008

Activity Number: B0801356

Status: APPROVED
Site Address: 5800 N BAY RD MBCH
Parcel #: 32150030270

Issued By: BUILCESN
Applied: 01/08/2008
Approved: 01/14/2008
Completed:
To Expire: 07/12/2008

Valuation: $25,000.00

Applicant: BUNNELL FOUNDATION INC
3033 NW NORTH RIVER DRIVE
MIAMI FL 33143
305-594-9900

Property Owner: MARK J GAINOR &W ELYSE S
MARK J GAINOR TRUSTEE
7463 FISHER ISLAND DR 331090717

CONDTION(s):

Description: **NEW DOCK & BOAT LIFT.**
Inspector Area: C Class Code: R3

==========

DETAIL LIST

Alteration/Repair Fees			
Alteration Bulding/Structures - Per Costs:	$25,000.00	B	$556.00
Awning, Canopy, Patio Cover - Per Costs:	$0.00		$0.00
Area Under Roof - RADON - Per Sq.Ft.:	0		$0.00
Walk-Thru - Per Valuation:	$0.00		$25.00
Repairs to Building/Structure - Per Costs:	$0.00		$0.00
Roofing or Re-roofing - Per Sq.Ft.:	0		$0.00
Window/Doors - Per # of:	0		$0.00
Signs 36-4 (Writer/Erect) - Per Sq.Ft.:	0		$0.00
Fence and/or Wall - Per Linear Feet:	0		$0.00
Partial Demo (Struct, Sign, Wall) - Per Costs:	$0.00		$0.00
Swimming Pool - Per Gallon:	0		$0.00
Painting - Per Costs:	$0.00		$0.00
Sandblasting - Per Costs:	$0.00		$0.00
Paving - Per Sq.Ft.:	0		$0.00
Concrete Slab - No Paving - Per Sq.Ft.:	0		$0.00
Trees - Per # of:	0		
Hedges - Per Linear Feet:	0		
Groundcover - Per Sq.Ft.:	0		
Landscaping Fee:			$0.00
Other Fees:			$0.00
Penalty Fee (If Applicable):			$0.00

[BSBLDPRM]

Activity Number: B0801356

Fire Safety Fees		
New Building or Addition - Per Sq.Ft.:	0	$0.00
Storage/Industrial Bldg - E & F Occup - Per Sq.Ft.:	0	$0.00
Greenhouse/Argiculture on Premises - Per Sq.Ft.:	0	$0.00
Screen Enclsoure/Trail on Premises - Per Sq.Ft.:	0	$0.00
SS Underground Tanks/App Shelter - Per #:	0	$0.00
Construction not shown Above - Per Costs:	$0.00	$0.00
Alt/Repair Building/Structure - Per Costs:	$0.00	$0.00
Marine Structure Fee		
Dock Area - Per Sq.Ft.:	491	$7.36
Seawall - Per Linear Feet:	0	$0.00
Boat Lifts, Davits, Hoist - Per # of:	1	$5.00
Batter, Mooring, Dock Piles - Per # of:	5	$5.00
Marine Structure Alt/Repair - Per Costs:	$0.00	$0.00
SFBC Compliance Surcharge		
New Const/Add - Res/Mult-Fam/Comm - Per Sq.Ft.:	0	$0.00
New Const/Add - Strg/Ind/Msc - Per Sq.Ft.:	0	$0.00
Cost for Other Construction:		$0.00
Training Fee		
Training Fee:		$25.00
Sanitation Fee:		$75.00
Additional Fees		
1st Reinspection:		$0.00
Continued Reinspections - Per # of:	0	$0.00
Building Joint Inspections - Per # of:	0	$0.00
Change of Contractor Per # of:	0	$0.00
Permit Extension - Per # of:	0	$0.00
Residential Card:		
Commercial Card:		
Permit Card Replacements:		$0.00
Lost Plan Fee - SF:		$0.00
Lost Plan Fee - Other:		$0.00
Overtime Inspection Fees:		$0.00
Total of All Fees:		$731.00
Total of Payments:		$731.00
Balance Due:		$0.00

==========

3 EXP. PERMITS

MIAMIBEACH

BUILDING DEPARTMENT
1700 Convention Center Drive
Miami Beach, FL 33139
Office: 305-673-7610 Fax: 305-673-7857

Upfront fee $125.00
$731.00

WORK PERMIT APPLICATION

FLORIDA BUILDING CODE IN EFFECT

DATE 10/07/2008 PERMIT # B0801350

If subsidiary or revision, provide the Master building permit number here B: _____

IS THIS PERMIT ASSOCIATED WITH A VIOLATION? If so; BV# _____

For DEMOLITION provide the year the structure was built _____ **Historic District** ☐Yes ☐No

Type of Property ☒Single Family ☐Commercial ☐Multi-Family/Condo

TYPE OF IMPROVEMENT: ☐Building ☐Electrical ☐Plumbing ☐Mechanical ☐**REVISION**

Describe New Dock & Boat Lift

Job Value $ 25,000.00 **Square Feet** 491.24 **Linear Feet** _____ **Pool Gallonage** _____ **No. of units** _____

Job Address 5300 North Bay Road, Miami Beach, FL 33139

Folio # 02-3216-005-0770 **Unit #** _____

City Miami Beach State _____ Zip 33139 Phone 305-336-4131

Owner/Owner Builder _____ Drivers License No. _____

Address _____

City _____ State _____ Zip _____ Phone _____

Fee Simple Titleholder's Name (if other than owner) _____

Address _____

City _____ State _____ Zip _____ Phone _____

Contractor Purnell Foundation, Inc License No. E-224

Address 3000 N.W. N. River Drive, Miami, Florida 33142

City _____ State _____ Zip 33142 Phone 305-633-3549

Cell# _____ Fax # _____

☐**Architect** _____ License No. _____

Address _____

City _____ State _____ Zip _____ Phone _____

☐**Engineer** Robert E. Sarraga License No. PE # 19649

Address 7901 SW 67th Avenue, South Miami, FL 33143

City South Miami State FL Zip _____ Phone 305-669-1916

PLEASE COMPLETE SHADED AREAS

BUILDING WORK PERMIT APPLICATIONS ARE AVAILABLE ON THE MIAMI BEACH WEBSITE AT :WWW.MIAMIBEACHFL.GOV

Bonding company Name_____

Address_____

City_____ State_____ Zip_____ Phone_____

Mortgage Lender's Name_____

Address_____

City_____ State_____ Zip_____ Phone_____

This application is hereby made to obtain a permit to do the work and installations as indicated. I certify that all work will be performed to meet the standards of all laws and construction regulations in this jurisdiction. I understand that **SEPARATE PERMITS** are required for ***Electrical, Mechanical, Plumbing, Signs, Swimming Pools, Spas, Windows, Sliding Glass Doors and Roofing.***

OWNER'S AFFIDAVIT: I certify that all the foregoing information is accurate and that all work will be done in compliance with all applicable laws regulating construction and Zoning.

NOTICE: In addition to the requirements of this permit, there may be additional restrictions applicable to this property that may be found in the public records of this county, and there may be additional permits required from other governmental entities such as water management districts, state agencies or federal agencies.

*** If the contractor is going to be hired by the tenant, check here. □**

Signature of Owner or Agent	Signature of Tenant	Signature of Qualifier
Printed Name of Owner or Agent	Printed Name of Tenant	Printed Name of Qualifier
Date 1-7-2008	Date_____	Date 1-7-2008
Signature of Notary Public	Signature of Notary Public	Signature of Notary Public
Identification_____	Identification_____	Identification_____
Sworn to and subscribed before me this 7th day of January 20 08. (Seal)	Sworn to and subscribed before me this ____ day of _____ 20____. (Seal)	Sworn to and subscribed before me this 7th day of January 20 08. (Seal)

BETTY ACOSTA
MY COMMISSION # DD 418728
EXPIRES: April 14, 2009
Bonded Thru Notary Public Underwriters

BETTY ACOSTA
MY COMMISSION # DD 418728
EXPIRES: April 14, 2009
Bonded Thru Notary Public Underwriters

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT. NOTICE OF COMMENCEMENT SHOULD BE FILED AT: 22 NW 1ST STREET, MIAMI, FL

STATE OF FLORIDA **COUNTY OF DADE**

Print Owner's Name_____ Owner's Signature_____

Sworn to and subscribed before me this _____ day of _____ 20_____, by: _____

( ) Personally Known ( ) Produced Identification - Type of Identification_____

Signature of Notary Public (Seal)

Application Approved By: _____

Permit Clerk

B0801356

ROBERT E. SAMARA, P.E. P.A.

Consulting Engineers

ENG LTR 001

April 25, 2008

Building Official
City of Miami Beach Building Department
1700 Convention Center Drive
2nd floor - City Hall
Miami Beach, Fl 33139

Re: Mark Gainor Residence
5800 N. Bay Road
Dock
Permit # B0801356

Dear Sir:

The enclosed pile driving log indicates piles have been driven to a minimum embedment depth of 12'. The pile log has been reviewed and certified that all the piles will sustain a minimum of 25 tons based on design , pile penetration and blow count at the bearing end.

If there are any questions, please do not hesitate to call.

Yours truly,

Robert E. Samara, P.E.

PE # 196649

7901 SW 67 Ave. Suite 207 Miami, FL 33143 Phone 305/662-1916 Fax 305/662-2491

Robert Samara P.E.P.A
Pile Driving Log

Project Name: Mark Gainor (Dock) **Hammer:** 4000 lbs **Drop** 10-12' **Date of Pile log:** 3/10/08

Address: 5800 N. Bay Road **Location:** **Tide:** Dock Support

Pile No.: 1	Pile No.: 2	Pile No.: 3	Pile No.: 4
Pile Size/Batter: 14"/ 0	Pile Size/Batter: 14"/ 0	Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0
Pile Lg: 24 '	Pile Lg: 24'	Pile Lg: 24'	Pile Lg: 24'
Water Depth			
5'	5'	5'	5'

Pile Penetration at Bay Botton or Grade

Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Water	1	Water	1	Water	1	Water
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6	Pre-Punched Hole	6	Pre-Punched Hole	6	Pre-Punched Hole	6	Pre-Punched Hole
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	17	10	17	10	17	10	18
11	18	11	18	11	17	11	18
12	18	12	20	12	18	12	19
13	20	13	20	13	19	13	19
14	20	14	21	14	20	14	20
15	21	15	21	15	21	15	20
16	22	16	23	16	22	16	21
17	24	17	24	17	23	17	21
18	26	18	24	18	24	18	23
19	28	19	27	19	27	19	24
20	29	20	29	20	28	20	27
21		21		21		21	
22		22		22		22	
23		23		23		23	

Field Technician: Bill O'Niell

Certified by: Robert Samara 4/25/08

Robert Samara P.E.P.A
Pile Driving Log

Project Name: Mark Gainor (Dock) **Hammer:** 4000 lbs **Drop** 10-12' **Date of Pile log:** 3/11/08

Address: 5800 N. Bay Road **Location:** Dock Support **Tide:**

Pile No.: 5	Pile No.: 6	Pile No.: 7	Pile No.: 8
Pile Size/Batter: 12"/ 0	Pile Size/Batter: 12"/ 0	Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0
Pile Lg: 24 '	Pile Lg: 24 '	Pile Lg: 24 '	Pile Lg: 24 '
Water Depth			
6 '	6 '	6 '	6 '

Pile Penetration at Bay Botton or Grade

Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	water	1	water	1	water	1	water
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7	Pre-Punched Hole	7	Pre-Punched Hole	7	Pre-Punched Hole	7	Pre-Punched Hole
8		8		8		8	
9		9		9		9	
10	18	10	18	10	19	10	18
11	18	11	19	11	20	11	18
12	19	12	20	12	22	12	20
13	18	13	21	13	21	13	21
14	21	14	22	14	23	14	22
15	21	15	22	15	23	15	23
16	22	16	24	16	24	16	23
17	23	17	25	17	24	17	24
18	24	18	27	18	25	18	24
19	24	19	27	19	26	19	26
20	25	20	28	20	28	20	27
21	27	21	29	21	29	21	28
22		22		22		22	
23		23		23		23	

Field Technician: Bill O'Neill

Certified by:

4/25/08

Robert Samara P.E.P.A
Pile Driving Log

Project Name: Mark Gainor (Dock) **Hammer:** 4000 lbs **Drop** 10-12' **Date of Pile log:** 3/12/08

Address: 5800 N. Bay Road **Location:** Tide: Dock Support

Pile No.: 9	Pile No.: 10	Pile No.: 11	Pile No.: 12
Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0
Pile Lg: 24 '	Pile Lg: 24 '	Pile Lg: 24 '	Pile Lg: 22 '
Water Depth			
6'	5'	5'	4'

Pile Penetration at Bay Botton or Grade

Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Water	1	Water	1	Water	1	Water
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	Pre-Punched Hole
6		6	Pre-Punched Hole	6	Pre-Punched Hole	6	
7	Pre-Punched Hole	7		7		7	
8		8		8		8	
9		9		9		9	
10	18	10	17	10	17	10	18
11	18	11	17	11	18	11	18
12	19	12	18	12	20	12	19
13	19	13	19	13	20	13	19
14	21	14	20	14	21	14	20
15	23	15	21	15	21	15	20
16	24	16	23	16	23	16	21
17	25	17	23	17	23	17	22
18	26	18	24	18	24	18	23
19	28	19	26	19	24	19	24
20	29	20	25	20	27	20	28
21	29	21	29	21	30	21	29
22		22		22		22	
23		23		23		23	

Field Technician: Bill O'Niell

Certified by: Robert Samara 4/25/08

Robert Samara P.E.P.A
Pile Driving Log

Project Name: Mark Gainor (Dock) **Hammer:** 4000 lbs **Drop** 10-12' **Date of Pile log:** 3/13/08

Address: 5800 N. Bay Road **Location:** Dock Support **Tide:**

Pile No.: 13	Pile No.: 14	Pile No.: 15	Pile No.:
Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0	Pile Size/Batter: 12"/0	Pile Size/Batter:
Pile Lg: 22'	Pile Lg: 22'	Pile Lg: 22'	
Water Depth			
4'	3'	3'	

Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Water	1	Water	1	Water	1	
2		2		2		2	
3		3		3		3	
4		4	Pre-Punched Hole	4	Pre-Punched Hole	4	
5	Pre-Punched Hole	5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	17	10	19	10	18	10	
11	17	11	19	11	19	11	
12	18	12	20	12	19	12	
13	19	13	22	13	20	13	
14	20	14	23	14	21	14	
15	23	15	24	15	22	15	
16	24	16	25	16	23	16	
17	24	17	25	17	24	17	
18	25	18	26	18	26	18	
19	26	19	27	19	27	19	
20	29	20	28	20	27	20	
21		21	29	21	30	21	
22		22		22		22	
23		23		23		23	

Field Technician: Bill O'Niell

Certified by: 4/25/08

B0801356
PILE 209 001

Bunnell Foundation, Inc.

Pile Driving Log

Project Name: Mark Gainor (Seawall) **Hammer:** 4100 lbs. **Date:** 03/10/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	1	Pile No.:	2	Pile No.:	3	Pile No.:	4
Pile Size:	Square 12"/3	Pile Size:	12"/3	Pile Size:	12"/3	Pile Size:	12"/3
Pile Lengt	22'	Pile Lengt	22'	Pile Lengt	22'	Pile Lengt	22'
Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	17	10	18	10	17	10	18
11	17	11	18	11	17	11	18
12	18	12	19	12	18	12	19
13	19	13	20	13	19	13	19
14	20	14	20	14	20	14	19
15	21	15	21	15	21	15	20
16	22	16	22	16	22	16	20
17	23	17	23	17	23	17	21
18	24	18	24	18	24	18	22
19	25	19	25	19	26	19	23
20	29	20	29	20	27	20	27
21		21		21		21	
22		22		22		22	
23		23		23		23	
24		24		24		24	
25		25		25		25	
26		26		26		26	
27		27		27		27	
28		28		28		28	
29		29		29		29	
30		30		30		30	
31		31		31		31	
32		32		32		32	
33		33		33		33	
34		34		34		34	
35		35		35		35	
36		36		36		36	

Approved [signature] 9/4/08

Bunnell Foundation, Inc.
Pile Driving Log

Project Name: Mark Gainor (Seawall) **Hammer:** 4100 lbs. **Date:** 03/11/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	5	Pile No.:	6	Pile No.:	7		8
Pile Size: Batter	12"/3	Pile Size:	12"/3	Pile Size:	12"/3		12"/3
Pile Lengt	22'	Pile Lengt	22'	Pile Lengt	22'		22'
Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	18	10	18	10	19	10	18
11	18	11	19	11	20	11	19
12	19	12	20	12	22	12	20
13	20	13	21	13	21	13	21
14	20	14	22	14	23	14	22
15	21	15	23	15	23	15	22
16	22	16	24	16	24	16	23
17	23	17	24	17	24	17	23
18	24	18	25	18	25	18	24
19	24	19	25	19	26	19	24
20	25	20	26	20	28	20	27
21	27	21	29	21	29	21	28
22	28	22		22		22	
23		23		23		23	
24		24		24		24	
25		25		25		25	
26		26		26		26	
27		27		27		27	
28		28		28		28	
29		29		29		29	
30		30		30		30	
31		31		31		31	
32		32		32		32	
33		33		33		33	
34		34		34		34	
35		35		35		35	
36		36		36		36	

Approved. 4/4/08

Bunnell Foundation, Inc.
Pile Driving Log

Project Name: Mark Gainer (Seawall) **Hammer:** 4100 lbs. **Date:** 03/12/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	9	Pile No.:	10	Pile No.:	11	Pile No.	12
Pile Size: (Batter)	12 "/3	Pile Size:	12 "/3	Pile Size:	12 "/3	Pile Size:	12 "/3
Pile Lengt:	22'	Pile Lengt:	22'	Pile Lengt:	22'	Pile Lengt	22'
Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	18	10	17	10	17	10	18
11	18	11	17	11	17	11	18
12	19	12	18	12	20	12	19
13	19	13	18	13	20	13	19
14	20	14	20	14	21	14	20
15	24	15	21	15	21	15	20
16	24	16	22	16	22	16	21
17	25	17	23	17	23	17	22
18	27	18	24	18	23	18	23
19	28	19	24	19	24	19	23
20	29	20	25	20	27	20	28
21	29	21	29	21	30	21	29
22		22		22		22	
23		23		23		23	
24		24		24		24	
25		25		25		25	
26		26		26		26	
27		27		27		27	
28		28		28		28	
29		29		29		29	
30		30		30		30	
31		31		31		31	
32		32		32		32	
33		33		33		33	
34		34		34		34	
35		35		35		35	
36		36		36		36	

Approved Robert Summers
4/4/08

Bunnell Foundation, Inc.
Pile Driving Log

Project Name: Mark Gainor (Seawall) **Hammer:** 4100 lbs. **Date:** 03/13/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	13	Pile No.:	14	Pile No.:	15	Pile No.:	16
Pile Size: Batter 12 "/3		Pile Size: 12 "/3		Pile Size: 12 "/3		Pile Size: 12"/3	
Pile Lengt 22'		Pile Lengt 22'		Pile Lengt 22'		Pile Lengt 22'	
Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	17	10	19	10	18	10	17
11	17	11	19	11	19	11	17
12	18	12	20	12	19	12	18
13	19	13	22	13	20	13	18
14	20	14	23	14	21	14	19
15	23	15	24	15	22	15	20
16	24	16	25	16	23	16	21
17	24	17	25	17	24	17	21
18	25	18	26	18	26	18	20
19	26	19	27	19	27	19	22
20	29	20	28	20	27	20	24
21		21	29	21	30	21	26
22		22		22		22	
23		23		23		23	
24		24		24		24	
25		25		25		25	
26		26		26		26	
27		27		27		27	
28		28		28		28	
29		29		29		29	
30		30		30		30	
31		31		31		31	
32		32		32		32	
33		33		33		33	
34		34		34		34	
35		35		35		35	
36		36		36		36	

Approved
4/4/08

Bunnell Foundation, Inc.
Pile Driving Log

Project Name: Mark Gainor (Seawall) **Hammer:** 4100 lbs. **Date:** 03/14/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	17	Pile No.:	18	Pile No.:	19	Pile No.:	20
Pile Size: (Batter)	12 "/3	Pile Size:	12 "/3	Pile Size:	12 "/3	Pile Size:	12 "/3
Pile Lengt:	22'	Pile Lengt:	22'	Pile Lengt:	22'	Pile Lengt:	22'
Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.	Depth	Blows / Ft.
1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole	1	Pre-Punched Hole
2		2		2		2	
3		3		3		3	
4		4		4		4	
5		5		5		5	
6		6		6		6	
7		7		7		7	
8		8		8		8	
9		9		9		9	
10	17	10	18	10	18	10	17
11	17	11	19	11	18	11	18
12	18	12	19	12	19	12	18
13	18	13	20	13	19	13	20
14	19	14	22	14	20	14	21
15	20	15	23	15	21	15	22
16	21	16	24	16	22	16	23
17	22	17	24	17	23	17	24
18	23	18	25	18	24	18	25
19	24	19	26	19	25	19	25
20	25	20	27	20	25	20	26
21	26	21	28	21	28	21	29
22	28	22		22		22	
23		23		23		23	
24		24		24		24	
25		25		25		25	
26		26		26		26	
27		27		27		27	
28		28		28		28	
29		29		29		29	
30		30		30		30	
31		31		31		31	
32		32		32		32	
33		33		33		33	
34		34		34		34	
35		35		35		35	
36		36		36		36	

Approved [signature] 4/4/08

Bunnell Foundation, Inc.
Pile Driving Log

Project Name: Mark Gainor (Seawall) **Hammer:** 4100 lbs. **Date:** 03/14/08

Address: 5800 N. Bay Road **Location:** Batter Seawall Support Piles

Pile No.:	21	Pile No.:	22	23	
Pile Size:	broken 12 "/3	Pile Size:	12"/3	12"/3	
Pile Lengt	22'	Pile Lengt	22'	22'	

Depth	Blows / Ft.	Depth					
1	Pre-Punched Hole	1	Pre-Punched Hole		Pre-Punched Hole		Pre-Punched Hole
2		2					
3		3					
4		4					
5		5					
6		6					
7		7					
8		8					
9		9					
10	17	10	17		17		
11	18	11	17		18		
12	18	12	18		18		
13	18	13	18		19		
14	19	14	19		20		
15	20	15	20		22		
16	20	16	21		22		
17	21	17	21		23		
18	22	18	23		24		
19	23	19	24		25		
20	26	20	27		27		
21	29	21	30		31		
22		22					
23		23					
24		24					
25		25					
26		26					
27		27					
28		28					
29		29					
30		30					
31		31					
32		32					
33		33					
34		34					
35		35					
36		36					

Approved
4/9/08

CITY OF MIAMI BEACH
BUILDING DEPARTMENT
1700 CONVENTION CENTER DRIVE
2ND FLOOR - CITY HALL
MIAMI BEACH, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I, (we) have been retained by: Bunnell Foundation, INC. to perform special inspector services under the Florida Building Code at the 5800 N. Bay Road project on the below listed structures as of 1/8/08 (date). I am a professional engineer licensed in the State of Florida.

Process Number: B0801356 **Master Permit (IF APPLICABLE):** _____

- [X] Special Inspector for Pilings, FBC 1822.1.20
- [ ] Special Inspector for Soil Compaction, FBC 1820.3.1
- [ ] Special Inspector for Precast Attachments, FBC 1927.12.2 (By P.E. or R.A..)
- [ ] Special Inspector for Reinforced Masonry, FBC 2122.4
- [ ] Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A..)
- [ ] Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R. A..)
- [X] Special Inspector for Concrete Bm & pile Cap Reinforcements

NOTE: Only the marked boxes apply.

The following individual's) employed by this firm or me are authorized representatives to perform inspections*

1. John Buschmer, P.E.
2. _____
3. _____
4. _____

***NOTE: FBC 2001 HVZ sections 1927.12.2, 2218.2, 2319.17.2.4.2 requires either a Registered professional Engineer or Registered Architect to perform the actual inspections.**

I, (we) will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, (we) understand that a Special Inspection Log for each building must be displayed in a convenient location on the site for reference by the City of Miami Beach Building Department Inspector. All mandatory inspections, as required of the Florida Building Code, Inspection performed by the Special inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. Further, upon completion of the work under each building permit, I will submit to the Building Inspector at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.

Signed and Sealed

19649
License Number

Date: 1/8/08

Architect/Engineer Signature: Robert Samara

Architect/Engineer Name Printed: ROBERT SAMARA.

Address: 7901 S.W. 67th Ave

Phone Number: 305-662-1916

Owner/Agent Signature: Tommy Max

Owner/Agent Name Printed: _____

Building Department Accepted By: J.D. Acevedo 01/11/08

The following calculations for **gravity loading** are in accordance with the **Florida Building Code 2004 Edition,** and applicable amendments.

A. Design Dock Gravity Loads

a) Dock Dead Loads:

1" wide x 1 1/4 deep IPE wood deck grating $w_{deck} := 6.0$ psf.

Dock Dead Load :

$w_{dock\_dl} := w_{deck}$ $w_{dock\_dl} = 6.00$ psf.

b) Dock Live Load : $w_{ll} := 60.0$ psf.

c) Combined Dock Dead and Live Load, :

$w_{dl\_ll} := w_{dock\_dl} + w_{ll}$ $w_{dl\_ll} = 66.00$ psf.

NOTICE: In addition to the requirement of this permit, there may be additional restrictions applicable to this property that may be found in the Public Records of this County, and there may be additional permits required from other governmental entities such as water management's districts, state agencies, or federal agencies.
The City of Miami Beach assumes no responsibility for accuracy of or results from these plans which are approved subject to compliance with all Federal, State, and Local Laws, Rules, and Regulations.

Robert E. Samara, P.E.P.A.
Consulting Engineers

Structural
PE # 19649

7901 SW 67th Avenue, Suite 207
Miami FL 33143
Tel (305)-662-1916

4/25/08

New Dock at
Gainor Residence
3800 North Bay Road
Miami Beach, FL
Date: November 28, 2007
Rev: December 14,2007
Rev3:April,25, 2008
Page: 1

Check Dock Decking,

A. Check 1 1/4 deep x 1" wide IPE Wood Decking: $s := 2$ in.o.c max for grating slats.

For simple spans carrying 60 psf live load,

$w_{dl\_ll} = 66.00$ psf. $L := 1.33$ ft., max. clear

$w := \left(w_{dl\_ll}\right) \cdot \left(\frac{s}{12}\right)$ $w = 11.00$ plf.

$R := w \cdot \frac{L}{2}$ $R = 7.32$ lbs.

$M := w \cdot \frac{L^2}{8}$ $M = 2.43$ ft-lbs.

Check IPE grating $C_r := 1.0$ $C_{m\_fv} := .97$ $C_{m\_fb} := .85$

Moment= $M = 2.43$ ft.-lbs $F_b := 2000$ psi. $F_v := 90$ psi.

$R = 7.32$ lbs. $S_{req} := \frac{M \cdot 12}{F_b \cdot C_r \cdot C_{m\_fb}}$ $A_{req} := \frac{1.5 \cdot R}{F_v \cdot C_{m\_fv}}$

$S_{req} = 0.02$ in^3 $A_{req} = 0.13$ in^2

Select 1" x 1 1/4 deep S4S $S_{prov} := \frac{1 \cdot 1.25^2}{6}$ in^3, OK $A_{prov} := 1 \cdot 1.25$ in^2, OK

$S_{prov} = 0.26$ in^3, OK $A_{prov} = 1.25$ in^2, OK

Check grating for Max. Deflection, Δ_{max}:

$I_{prov} := \frac{1 \cdot 1.25^3}{12}$ $I_{prov} = 0.16$ in^4 $E_w := 3000000$

$\Delta_{grate} := \frac{5 \cdot \frac{w}{12} \cdot (L \cdot 12)^4}{384 \cdot E_w \cdot I_{prov}}$ $\Delta_{grate} = 0.00$ in. $< \frac{L \cdot 12}{240} = 0.07$ in, O.K.

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Structural
PE # 19649

7901 SW 67th Avenue, Suite 207
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4/25/08

Access dock 4'- 0" wide

Design 12"x 18" Dock Beam B-3 spanning 15.5 ft max between pile caps

$trib := \frac{4.0}{2}$ trib = 2.00 ft. w_sDL := 0.06 ksf $w\_sDL_u := 1.4 \cdot w\_sDL$

b := 12 in. t := 18 in. min L_c := 15.5 ft. max.

w_conc := .150 kcf $w\_bm_u := 1.4 \left(\frac{b \cdot t}{144}\right) w\_conc$ $w\_bm_u = 0.32$ klf.

w_LL := 0.060 ksf $w\_LL_u := 1.7 \cdot w\_LL$ $w\_LL_u = 0.10$ klf.

$w_u := (w\_LL_u + w\_sDL_u) \cdot trib + w\_bm_u$ $w_u = 0.687$ kips/lf.

a. Determine V_u and M_u :

$M_u := \frac{w_u \cdot L_c^2}{8}$ $M_u = 20.63$ ft-kips.

$V_{uB2} := w_u \cdot \frac{L_c}{2}$ $V_{uB2} = 5.32$ kips.

b. Design Shear stirrups and bottom Longitudinal rebars

$f_c := 5000$ psi. min $f_y = 60000$ psi

$d_{bot} := t - 3.5$ $d_{bot} = 14.50$ in., to provide 3.0" cover (min) over shear ties

For Shear: $\phi := 0.85$ $\phi V_c := \frac{\phi \cdot 2}{1000} \cdot \sqrt{f_c} \cdot b \cdot d_{bot}$

$\phi V_c = 20.92$ kips. $\frac{\phi V_c}{2} = 10.46$ kips., > $V_{uB2} = 5.32$ kips, Therefore shear reinf. not reqd. **However provide #3 closed ties at 12" o/c**

However, provide #3 ties at 12" o.c. for rebar placement

For Bending Moment: $\phi := 0.9$ $M_u = 20.63$ ft-kips

$$Abot_{s\_req} := \frac{b \cdot d_{bot} \cdot f_c - \sqrt{(b \cdot d_{bot} \cdot f_c)^2 - 2 \cdot b \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi}}}{f_y}$$

$\rho_{bot} := \frac{Abot_{s\_req}}{b \cdot d_{bot}}$ $\rho_{bot} = 0.0018$

$Abot_{s\_req} = 0.32$ $A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.48$ in.2, or

$A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.48$ in.2,

$A_{bar} := 0.31$ in.^{2} N := 3

$A_{s\_prov} := A_{bar} \cdot N$ $A_{s\_prov} = 0.93$ in.^{2} **Use 3 # 5 's top and bottom and #3 closed ties at 12" o/c**

VOID VOID VOID

Robert E. Samara, P.E.P.A.
Consulting Engineers

Structural
PE # 19649

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ACCESS WALKWAY 4'- 0" wide

Check 3 x 10 PT S.Pine stringer joist $s := 16$ in.o.c max to carry IPE grating

$w_{dl\_ll} = 66.00$ psf. $L_{joist} := 13.0$ ft., max. clear

$w_{joist} := (w_{dl\_ll}) \cdot \left(\frac{s}{12}\right)$ $w_{joist} = 88.00$ plf.

$R_l := w_{joist} \cdot \frac{L_{joist}}{2}$ $R_l = 572.00$ lbs.

$M_{joist} := w_{joist} \cdot \frac{L_{joist}^2}{8}$ $M_{joist} = 1859.00$ ft-lbs.

Check Joist size for Southern Pine No 2 or better $C_r := 1.0$ $C_{m\_fv} := .97$ $C_{m\_fb} := .85$

Moment= $M_{joist} = 1859.00$ ft.-lbs $F_b := 1050$ psi. $F_v := 90$ psi.

$R_l = 572.00$ lbs. $S_{req} := \frac{M_{joist} \cdot 12}{F_b \cdot C_r \cdot C_{m\_fb}}$ $A_{req} := \frac{1.5 \cdot R_l}{F_v \cdot C_{m\_fv}}$

$S_{req} = 24.99$ in^3 $A_{req} = 9.83$ in^2

Select 3 x 10 S4S $S_{prov} := 35.65$ in^3, OK $A_{prov} := 23.13$ in^2, OK

Check joist for Max. Deflection, Δ_{max}:

$I_{prov} := 164.9$ in^4 $E_w := 1600000$

$\Delta_{hdr} := \frac{5 \cdot \frac{w_{joist}}{12} \cdot (L_{joist} \cdot 12)^4}{384 \cdot E_w \cdot I_{prov}}$ $\Delta_{hdr} = 0.21$ in. < $\frac{L_{joist} \cdot 12}{360} = 0.43$ in, O.K.

Check capacity of 3/4" diam. expansion bolts spaced at 8" o.c. and with 8" min. embedment into concrete to support 3 x 8" PT wood ledger which in turn carries 3x10 stringer

For "Hilti" 3/4" diam. Kwik-Bolt II expansion bolts, with 8" min. embedment into concrete.
consider bolt diameter at wood ledger = 3/4"

$R_{ldgr} := w_{joist} \cdot \left(\frac{L_{joist}}{2}\right)$ $R_{ldgr} = 572.00$ lbs.

$Z_{s\_perp} := 810$ lbs/bolt. $C_{m\_bolt} := .70$ $t_s := 2.5$ in.

$V_{allow} := 1.5 Z_{s\_perp} \cdot C_{m\_bolt}$ $V_{allow} = 850.50$ lbs. > $R_{ldgr} = 572.00$ lbs., **OK.**

$V_{allow\_conc} := 3500$ lbs allowable shear O.K

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4/25/08

Design Pile Cap PC-5 3'- 9" long x 2'-6" x 1'-6" at 4'-0" wide dock

Pile Cap spanning 2.5 ft. clear max. over 4 piles :

$trib := 15.5$ ft.

$b_{cap} := 30.0$ in. $t_{cap} := 18$ in.

$L_c := 2.25$ ft. max.

$w\_conc := .150$ kcf $w\_bm_u := 1.4 \cdot \left(\frac{t \cdot b}{144}\right) \cdot \frac{w\_conc}{4}$ $w\_bm_u = 0.08$ klf.

$w_u := \left(w\_LL_u + w\_sDL_u + w\_bm_u\right) \cdot trib$ $w_u = 4.104$ kips/lf.

a. Determine V_u and M_u :

$M_u := \frac{w_u \cdot L_c^2}{8}$ $M_u = 2.60$ ft-kips.

$V_u := w_u \cdot \frac{L_c}{2}$ $V_u = 4.62$ kips.

b. Design Shear stirrups and bottom Longditudinal rebar:

$f_c := 5000$ psi. $f_y := 60000$ psi. $b_{cap} = 30.00$ in.

$d_{bot} := t_{cap} - 4.5$ $d_{bot} = 13.50$ in., to provide 4" cover (min) over shear ties

For Shear: $\phi := 0.85$ $\phi V_c := \frac{\phi \cdot 2}{1000} \cdot \sqrt{f_c} \cdot b_{cap} \cdot d_{bot}$

$\phi V_c = 48.68$ kips. $\frac{\phi V_c}{2} = 24.34$ kips., > $V_u = 4.62$ kips, Therefore no shear reinf. reqd.

$w_u = 4.10$ klf. $R_{u\_cap} := V_u$

However, provide **#3 ties at 12" o.c. for rebar placement**

For Cap Bending Moment: $\phi := 0.9$ $M_u = 2.60$ ft-kips

$$Abot_{s\_req} := \frac{b_{cap} \cdot d_{bot} \cdot f_c - \sqrt{\left(b_{cap} \cdot d_{bot} \cdot f_c\right)^2 - 2 \cdot b_{cap} \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi}}}{f_y}$$

$Abot_{s\_req} = 0.043$ in.^{2}

$A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.06$ in.2, $A_{bar} := 0.31$ in.^{2} $N := 5$

$A_{s\_prov} := A_{bar} \cdot N$ $A_{s\_prov} = 1.55$ in.^{2}

Use 5 # 5 's hooked to span across 3'-9" long cap section top and bottom;
Use #3 closed ties at 12" o/c

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Structural
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9/25/08

New Dock at
Gainor Residence
3800 North Bay Road
Miami Beach, FL
Date: November 28, 2007
Rev: December 14,2007
Rev3:April,25, 2008

Main dock 7'- 0" wide

Check 3 x 8 PT S.Pine stringer joist $s := 16.0$ in.o.c max to carry IPE grating

$w_{dl\_ll} = 66.00$ psf. $L_{joist} := 9.5$ ft., max. clear

$w_{joist} := (w_{dl\_ll}) \cdot \left(\frac{s}{12}\right)$ $w_{joist} = 88.00$ plf.

$R_l := w_{joist} \cdot \frac{L_{joist}}{2}$ $R_l = 418.00$ lbs.

$M_{joist} := w_{joist} \cdot \frac{L_{joist}^2}{8}$ $M_{joist} = 992.75$ ft-lbs.

Check Joist size for Southern Pine No 2 or better $C_r := 1.0$ $C_{m\_fv} := .97$ $C_{m\_fb} := .85$

Moment= $M_{joist} = 992.75$ ft.-lbs $F_b := 1200$ psi. $F_v := 90$ psi.

$R_l = 418.00$ lbs. $S_{req} := \frac{M_{joist} \cdot 12}{F_b \cdot C_r \cdot C_{m\_fb}}$ $A_{req} := \frac{1.5 \cdot R_l}{F_v \cdot C_{m\_fv}}$

$S_{req} = 11.68$ in^3 $A_{req} = 7.18$ in^2

Select 3 x 8 S4S $S_{prov} := 21.90$ in^3, OK $A_{prov} := 23.13$ in^2, OK

Check joist for Max. Deflection, Δ_{max}

$I_{prov} := 79.39$ in^4 $E_w := 1600000$

$\Delta_{hdr} := \frac{5 \cdot \frac{w_{joist}}{12} \cdot (L_{joist} \cdot 12)^4}{384 \cdot E_w \cdot I_{prov}}$ $\Delta_{hdr} = 0.13$ in. < $\frac{L_{joist} \cdot 12}{360} = 0.32$ in, O.K.

Check capacity of 3/4" diam. expansion bolts spaced at 16 o.c. and with 8" min. embedment into concrete to support 3 x 8" PT wood ledger which in turn carries stringers

For "Hilti" 3/4" diam. Kwik-Bolt II expansion bolts, with 6" min. embedment into concrete.
consider bolt diameter at wood ledger = 3/4"

$R_{ldgr} := w_{joist} \cdot \left(\frac{L_{joist}}{2}\right)$ $R_{ldgr} = 418.00$ lbs.

$Z_{s\_perp} := 810$ lbs/bolt. $C_{m\_bolt} := .70$ $t_s := 2.5$ in.

$V_{allow} := .75 Z_{s\_perp} \cdot C_{m\_bolt}$ $V_{allow} = 425.25$ lbs. > $R_{ldgr} = 418.00$ lbs., **OK.**

$V_{allow\_conc} := 3500$ lbs allowable shear O.K

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Structural
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4/25/08

Design 12"x 18" Dock Beam B-1 spanning 10.0 ft max between pile caps

$trib := \frac{7.0}{2}$ trib = 3.50 ft. $w\_sDL := 0.06$ ksf $w\_sDL_u := 1.4 \cdot w\_sDL$

$b := 12$ in. $t := 18$ in. max $L_c := 10.0$ ft. max.

$w\_conc := .150$ ksf $w\_bm := 1.4 \cdot \left(\frac{b \cdot t}{144}\right) \cdot w\_conc$ $w\_bm_u = 0.32$ klf.

$w\_LL := 0.060$ ksf $w\_LL_u := 1.7 \cdot w\_LL$ $w\_LL_u = 0.10$ klf.

$w_u := (w\_LL_u + w\_sDL_u) \cdot trib + w\_bm_u$ $w_u = 0.966$ kips/lf.

a. Determine V_u and M_u :

$M_u := \frac{w_u \cdot L_c^2}{8}$ $M_u = 12.07$ ft-kips.

$V_{uB1} := w_u \cdot \frac{L_c}{2}$ $V_{uB2} = 5.32$ kips.

b. Design Shear stirrups and bottom Longditudinal rebar:

$f_c := 5000$ psi. min $E_c := 4000$ psi.

$d_{bot} := t - 3.5$ $d_{bot} = 14.50$ in. to provide 3.0" cover (min) over shear ties

For Shear: $\phi := 0.85$ $\phi V_c := \frac{\phi \cdot 2}{1000} \cdot \sqrt{f_c} \cdot b \cdot d_{bot}$

$\phi V_c = 20.92$ kips. $\frac{\phi V_c}{2} = 10.46$ kips., > $V_{uB2} = 5.32$ kips, Therefore shear reinf. not reqd.

However, provide #3 ties at 12" o.c. for rebar placement

For Bending Moment: $\phi := 0.9$ $M_u = 12.07$ ft-kips

$$Abot_{s\_req} := \frac{b \cdot d_{bot} \cdot f_c - \sqrt{(b \cdot d_{bot} \cdot f_c)^2 - 2 \cdot b \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi}}}{f_y}$$

$\rho_{bot} := \frac{Abot_{s\_req}}{b \cdot d_{bot}}$ $\rho_{bot} = 0.0107$

$Abot_{s\_req} = 0.186$ in.^{2} $A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.28$ in.2, or

$A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.28$ in.2,

$A_{bar} := 0.31$ in.^{2} $N := 3$

$A_{s\_prov} := A_{bar} \cdot N$ $A_{s\_prov} = 0.93$ in.^{2} **Use 3 # 5 's top and bottom and #3 closed ties at 12" o/c**

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Structural
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Design Pile Cap PC - 3 at 7'-0" wide dock

Pile Cap spanning 4.5 ft. clear max. over 2 piles : $trib := 12.4$ ft.

$b_{cap} := 30.0$ in. $t_{cap} := 18$ in. $L_c := 4.5$ ft. max.

$w\_conc := .150$ kcf $w\_bm_u := 1.4 \cdot \left(\frac{t \cdot b}{144}\right) \cdot \frac{w\_conc}{7}$ $w\_bm_u = 0.05$ klf.

$w_u := \left(w\_LL_u + w\_sDL_u + w\_bm_u\right) \cdot trib$ $w_u = 2.864$ kips/lf.

a. Determine V_u and M_u :

$M_u := \frac{w_u \cdot L_c^2}{8}$ $M_u = 7.25$ ft-kips.

$V_u := w_u \cdot \frac{L_c}{2}$ $V_u = 6.44$ kips.

b. Design Shear stirrups and bottom Longditudinal rebar:

$f_c := 5000$ psi. $f_y := 60000$ psi. $b_{cap} = 30.00$ in.

$d_{bot} := t_{cap} - 4.5$ $d_{bot} = 13.50$ in., to provide 4" cover (min) over shear ties

For Shear: $\phi := 0.85$ $\phi V_c := \frac{\phi \cdot 2}{1000} \cdot \sqrt{f_c} \cdot b \cdot d_{bot}$

$\phi V_c = 19.47$ kips. $\frac{\phi V_c}{2} = 9.74$ kips., > $V_u = 6.44$ kips, Therefore no shear reinf. reqd.

$w_u = 2.86$ klf. $R_{u\_cap} := V_u$

However, provide **#3 ties at 12" o.c. for rebar placement**

For Cap Bending Moment: $\phi := 0.9$ $M_u = 7.25$ ft-kips

$$Abot_{s\_req} := \frac{b_{cap} \cdot d_{bot} \cdot f_c - \sqrt{\left(b_{cap} \cdot d_{bot} \cdot f_c\right)^2 - 2 \cdot d_{bot} \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi}}}{f_y}$$

$Abot_{s\_req} = 0.054$ in.^{2}

$A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.08$ in.2, $A_{bar} := 0.31$ in.^{2} $N := 5$

$A_{s\_prov} := A_{bar} \cdot N$ $A_{s\_prov} = 1.55$ in.^{2} **Use 5 # 5 's hooked to span across 6'-9" long cap section top and bottom, #3 ties at 12" o/c**

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Structural
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4/25/08

Design Pile Cap PC - 2 at 7'-0" wide dock

Pile Cap spanning 4.5 ft. clear max. over 2 piles : $trib := 12.4$ ft.

$b_{cap} := 42$ in. $t_{cap} := 18$ in. $L_c := 4.5$ ft. max.

$w\_conc := .150$ kcf $w\_bm_u := 1.4 \cdot \left(\frac{t \cdot b}{144}\right) \cdot \frac{w\_conc}{7}$ $w\_bm_u = 0.05$ klf.

$w_u := (w\_LL_u + w\_sDL_u + w\_bm_u) \cdot trib$ $w_u = 2.864$ kips/lf.

a. Determine V_u and M_u :

$M_u := \frac{w_u \cdot L_c^2}{8}$ $M_u = 7.25$ ft-kips.

$V_u := w_u \cdot \frac{L_c}{2}$ $V_u = 6.44$ kips.

b. Design Shear stirrups and bottom Longditudinal rebar:

$f_c := 5000$ psi. $f_y := 60000$ psi. $b_{cap} = 42.00$ in.

$d_{bot} := t_{cap} - 4.5$ $d_{bot} = 13.50$ in., to provide 4" cover (min) over shear ties

For Shear: $\phi := 0.85$ $\phi V_c := \frac{\phi \cdot 2}{1000} \cdot \sqrt{f_c} \cdot b \cdot d_{bot}$

$\phi V_c = 19.47$ kips. $\frac{\phi V_c}{2} = 9.74$ kips., > $V_u = 6.44$ kips, Therefore no shear reinf. reqd.

$w_u = 2.86$ klf. $R_{u\_cap} := V_u$

However, provide **#3 ties at 12" o.c. for rebar placement**

For Cap Bending Moment: $\phi := 0.9$ $M_u = 7.25$ ft-kips

$$Abot_{s\_req} := \frac{b_{cap} \cdot d_{bot} \cdot f_c - \sqrt{(b_{cap} \cdot d_{bot} \cdot f_c)^2 - 2 \cdot d_{bot} \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi}}}{f_y}$$

$Abot_{s\_req} = 0.038$ in.^{2}

$A_{s\_min} := 1.5 \cdot Abot_{s\_req}$ $A_{s\_min} = 0.06$ in.2, $A_{bar} := 0.31$ in.^{2} $N := 8$

$A_{s\_prov} := A_{bar} \cdot N$ $A_{s\_prov} = 2.48$ in.^{2}

Use 8 # 5 's hooked to span across 6'-9" long cap section top and bottom, #3 ties at 12" o/c provide 2 #5 hook bars top at cantilever portion to support beam B-2

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9/25/08

New Dock at
Gainor Residence
3800 North Bay Road
Miami Beach, FL
Date: November 28, 2007
Rev: December 14,2007
Rev3:April,25, 2008

Check Maximum Pile Loading for 14" x 14" Precast Concrete Pile at 20000 # boat lift and 7' Dock at pile cap PC-3,

consider 1/2 of boatlift capacity supported by pile

Max boat lift load $P_{lift} := \frac{20000}{2 \cdot 1000}$ $P_{lift} = 10.00$ kips

$R_{beam} := \frac{V_{uB2}}{1.5}$ $R_{beam} = 3.55$ kips

$W_{cap} := \frac{48 \cdot 18}{144} \cdot \frac{7}{2} \cdot .150$ $W_{cap} = 3.15$ kips

Vertical overturning reaction on outboard pile:

Consider a lift lever arm from cl of outboard pile to centerline of boat hull $L_{liftarm} := 6.5$ ft.

Pile cl to cl spacing between inboard and outboard piles $S_{pile} := 3.67$ ft.

$R_{over} := P_{lift} \cdot \frac{(L_{liftarm} + S_{pile})}{S_{pile}}$ $R_{over} = 27.71$ kips

$P_{req} := R_{beam} + W_{cap} + R_{over}$

$P_{req} = 34.41$ Kips max per pile,

$\frac{P_{req}}{2} = 17.21$ Tons < $P_{allow} := 35$ Tons, per FBC 2004, therefore O.K.

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Check inboard and outboard 14" x 14" precast piles To Restrain New Boat lift for overturning

Consider a lift lever arm from cl of outboard pile to centerline of boat hull $L_{liftarm} := 6.5$ ft.

Pile cl to cl spacing between inboard and outboard piles at access pier $S_{pile} := 3.67$ ft.

$M_{over} := P_{lift} \cdot \left(L_{liftarm} + \frac{S_{pile}}{2} \right)$ $M_{over} = 83.35$ ft-kips divided between the 2 piles at PC-2

Check bending stress on 14" x 14" 6000 psi precast concrete pile with 8 - 1/2" diam 270 ksi strands (Refer to Design Sheet for pile, as provided by manufacturer)

$M_{pile} := \frac{M_{over}}{2}$ $M_{pile} = 41.68$ ft-kips < $M_{cr} := 42$ ft-kips at $f_{cr} := 7.5 \cdot \sqrt{f_c}$ OK

Check Pile Foundation for: Embedment: $D := 12.0$ ft.

Assume increasing soil resistance with depth, gravel, ASTM Class.GW, High Density well compacted fill:

$k := 455$ lbs/ft/ft of depth

Fdn. diameter: $d := 12.0$ in. min., Overturning Safety Factor : $SF := 1.5$

$$D_{reqd} := \left(\frac{12 \cdot \frac{M_{over} \cdot 1000}{4} \cdot SF}{k \cdot \frac{d}{12} \cdot 1.33} \right)^{\frac{1}{3}}$$

$D_{reqd} = 8.53$ ft.per pile < $D = 12.00$ ft.per pile, O.K.

Pile driving formula, Per FBC 1806 and FBC 1822.2

For Drop Hammer, 4000 lb minimum with 6' drop, per FBC 1822.1.18

$P := 50000$ lbs, allowable total load, in pounds

$W := 4000$ lbs, weight of striking hammer, in pounds

$h := 6.0$ ft., height of fall of striking part of hammer, in feet

$S := 1.96$ in., average penetration per blow of the last 5 blows

$5S = 9.80$ in, total pentration of five final blows

Therefore, minimum number of blows over the last 3 inches of penetration

$n_{3\_in} := \frac{3}{S}$ $n_{3\_in} = 1.53$ blows, use 2 blows minimum for last 3" for pile log acceptance

Drop Hammer Formula $P := \frac{2 \cdot W \cdot h}{S - 1}$ $P = 50000.00$ lbs, allowable $\frac{P}{2000} = 25.00$ Tons, allowable

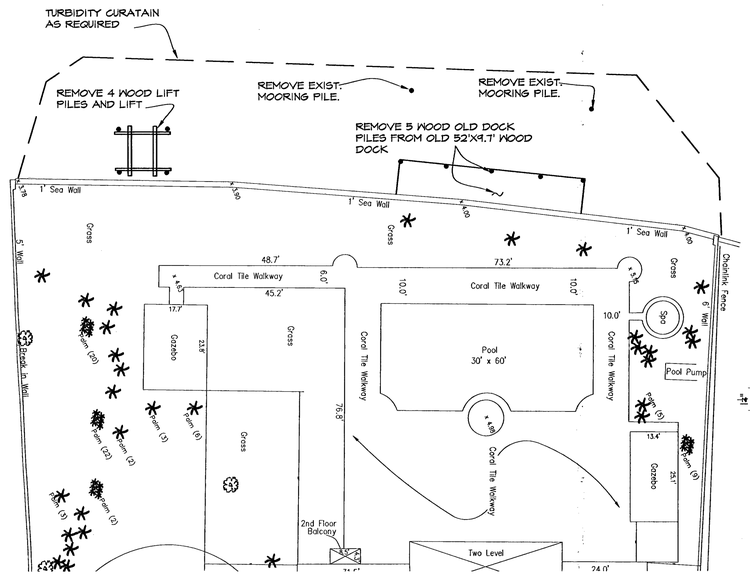
Pile Driving Log to be kept for each pile driven per FBC 1822.1.20

Robert E. Samara, P.E.P.A.
Consulting Engineers

Structural
PE # 19649

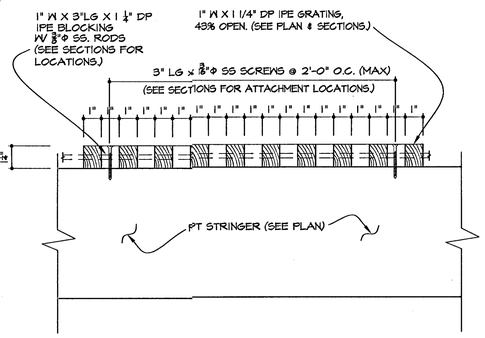
7901 SW 67th Avenue, Suite 207
Miami FL 33143
Tel (305)-662-1916

4/25/08

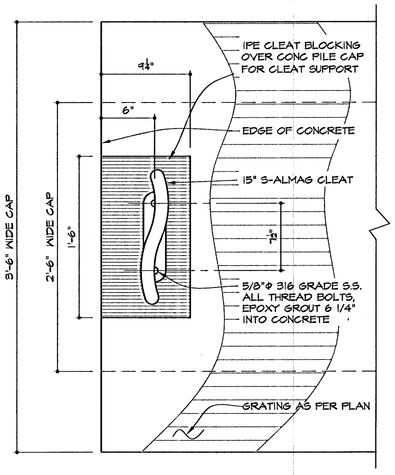


SITE PLAN 1" = 20'-0"

EXISTING CONDITIONS



4 IPE GRATING DETAIL 3" = 1'-0"



4 TYP. 15" CLEAT DETAIL

1½" = 1'-0"

GENERAL NOTES 4

PRECAST CONCRETE PILING

1. PRECAST CONCRETE PILES (W/ 5000 P.S.I. MIN. CONCRETE) FOR 12" x 12" PILES W/ (4) 7/16"Ø 270 K.S.I. ASTM A416 STRANDS, 14"X14" PILES FOR (8) 1/2"Ø 270 K.S.I. ASTM A416 STRANDS. EXTEND PILE REINF 12" MIN INTO PILE CAPS. LOW-LAX STRANDS W/ 2 1/2" MIN. CONCRETE COVER TO TIES. DRIVEN TO A MINIMUM BEARING CAPACITY OF (12"X12") 25 TONS, (14"X14") 35 TONS WITH 12' MIN. PENETRATION INTO FIRM MATERIAL BELOW SILT LAYER.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

CONCRETE AND REINFORCING STEEL

1. ALL CONCRETE (EXCEPT PRECAST PILES) SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI (.40 W/C RATIO) AT THE END OF 28 DAYS. FOUR (4) CONCRETE CYLINDERS SHALL BE TAKEN FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF AND SHALL BE TESTED AT 3, 7 AND 28 DAYS. SLUMP SHALL NOT EXCEED 5" (± 1").
2. ALL REINFORCEMENT SHALL BE 60000 PSI MINIMUM YIELD NEW BILLET STEEL IN ACCORDANCE WITH ASTM A615 GRADE 60. ALL BAR LAPS SHALL BE A MINIMUM OF 48 BAR DIAMETERS. PLACING OF REINFORCEMENT SHALL CONFORM TO THE LATEST ACI AND MANUAL OF STANDARD OF PRACTICE CODES.
3. ALL CONCRETE SHALL BE PLACED WITHIN 90 MINUTES FROM BATCH TIME, AND VIBRATED AS REQUIRED BY THE ACI MANUAL OF CONCRETE PRACTICE. TEMPERATURE OF CONCRETE AT THE TIME OF PLACEMENT SHALL BE BETWEEN 75° F. AND 100° F.
4. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" (EXCEPT STAIR NOSINGS), OR AS SHOWN ON THE PLANS.

WOOD

1. WOOD GRATING SHALL BE IPE 1" X 1 1/4", ALLOWING 43% LIGHT TRANSMISSION, AS PLAN ,DETAIL & SECTIONS. (F_b =2000 PSI MIN.)
2. PRIMARY WOOD FRAMING MEMBERS SHALL BE NO. 2 PRESSURE TREATED SOUTHERN PINE OR BETTER. (3X8, 3X10 F_b =1050 PSI MIN) 6X6 = #1 SOUTHERN PINE (F_b = 1350 PSI)

WOOD PILING

1. WOOD PILES SHALL BE 12" DIAMETER SOUTH AMERICAN GREENHEART PILES AS SHOWN ON THE PLANS, DRIVEN TO 12 FEET MINIMUM PENTRATION INTO FIRM MATERIAL

BOLTS

1. ALL BOLTS, WASHERS AND NUTS SHALL BE TYPE 304 STAINLESS STEEL.
2. PILES SHALL BE CUT OFF AT ELEVATIONS SHOWN ON THE PLANS & SECTIONS.

GENERAL

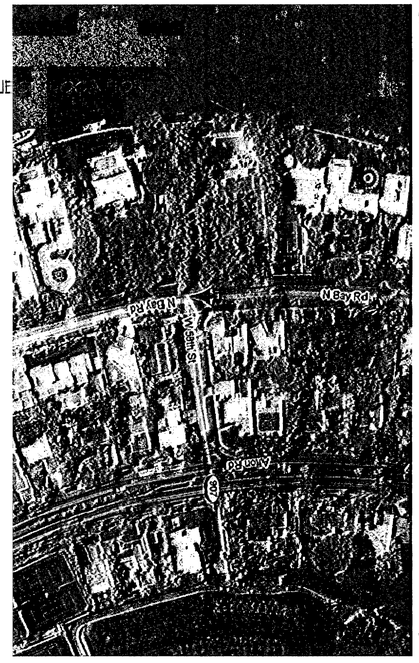
1. ELEVATIONS SHOWN REFER TO THE NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929.
2. ALL DIMENSIONS ON PLANS ARE SUBJECT TO VERIFICATION IN THE FIELD.
3. IT IS THE INTENT OF THESE PLANS TO BE IN ACCORDANCE WITH APPLICABLE CODES AND AUTHORITIES HAVING JURISDICTION. ANY DISCREPANCIES BETWEEN THESE PLANS AND APPLICABLE CODES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH WORK.
4. IT IS THE INTENT OF THESE PLANS AND THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH LOCAL, STATE, AND FEDERAL ENVIRONMENTAL PERMITS ISSUED FOR THIS PROJECT. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE AND SOVERN HIMSELF BY ALL PROVISIONS OF THESE PERMITS.
5. APPLICABLE BUILDING CODE: FLORIDA BUILDING CODE 2004 EDITION.

DESIGN LOADS

1. DOCK LL .....60 PSF

2 DOCK DESIGN FOR WIND LOADING WITHOUT BOAT MOORING TO DOCK IS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004 EDITION AND SECTION 6 OF ASCE 7-98. WIND SPEED 146 MPH, EXPOSURE CATEGORY "C" FOR COMPONENTS AND CLADDING, EXPOSURE CATEGORY "C" FOR MWFRS. IMPORTANCE FACTOR, I = 1.0. INTERNAL PRESSURE COEFFICIENT = 0.18

3. 50 MPH WIND SPEED APPLIED TO A 55' VESSEL WITH A 42" DRAFT. AT SLIP #1



LOCATION PLAN NTS

PUBLIC WORKS
PLAN REVIEW NOTICE
Phone 305-673-7000 Fax 305-673-7028

THIS PLAN REVIEW CONSTITUTES APPROVAL FOR OBTAINING BUILDING PERMITS ONLY.

All construction and/or use of equipment in the right-of-way and/or easements, requires a separate Public Works Department permit prior to start of construction.

Permit Requirements: Proof of existing sidewalk/swale area conditions (pictures) and/or posting of sidewalk/roadway bonds (Public Works Inspection of the right-of-way will be required prior to final sign-off on site G.C. / C.O., or the release of bonds.)

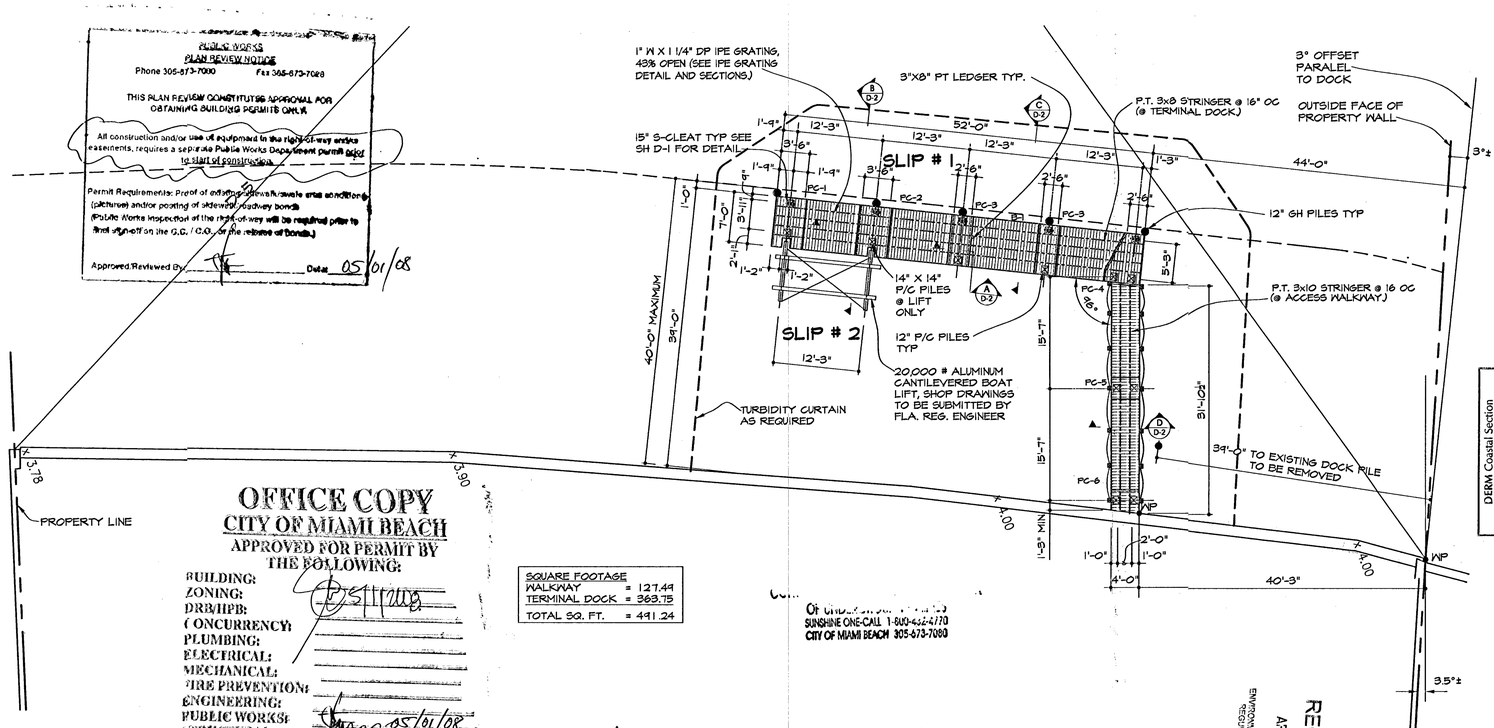
Approved/Reviewed By: _____ Date 05/01/08

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:

BUILDING:
ZONING:
DRB/HPB:
CONCURRENCY:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:
OTHER:

Florida Building Code Section 104.5.3
APPROVED FOR CODE COMPLIANCE

05/01/08



4 NEW PROPOSED DOCK PLAN 1" = 10'-0"

SQUARE FOOTAGE	
WALKWAY	= 127.44
TERMINAL DOCK	= 363.75
TOTAL SQ. FT.	= 491.24

DERM Coastal Section
PRELIMINARY APPROVAL
Name
Date 4/30/08
Boatlift shop drawings Needed

SUNSHINE ONE-CALL 1-800-432-4770
CITY OF MIAMI BEACH 305-673-7080

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DERM
ENVIRONMENTAL RESOURCES REGULATION DIVISION

ROBERT E. SAMARA P.E., P.A.
Consulting Engineers
PE # 19649
7901 S.W. 67th Avenue., Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
1 11-21-07 REV PER BMS & PILE CAPS
2 12-17-07 MOVED DOCK & REV DETAILS
3 01-28-08 REV. DECKING & BEAMS
4 04-25-08 REV. FRAIMING & REMOVED CONC BEAMS

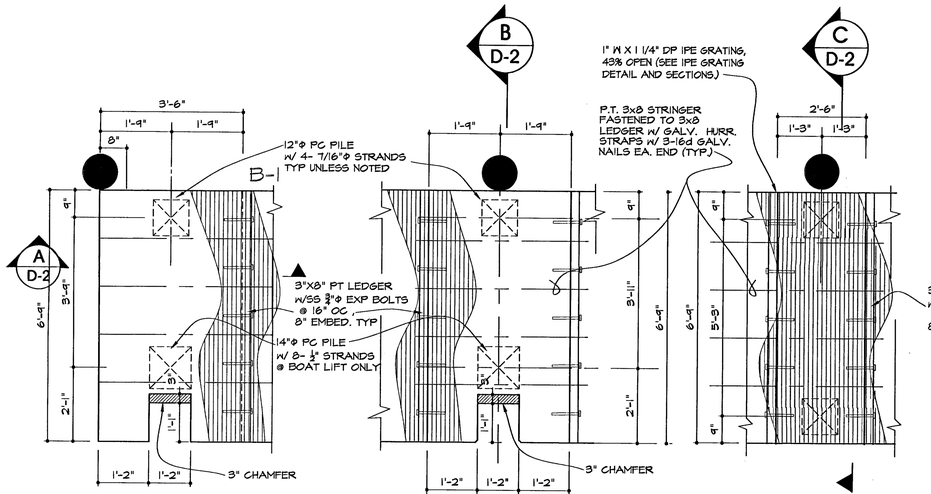
DATE: 11-07-07

DWN BY: A.B.

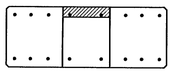
JOB NO: 07-70

SHEET: D-1

OF 2 SHEETS



PC-1 ⚠4

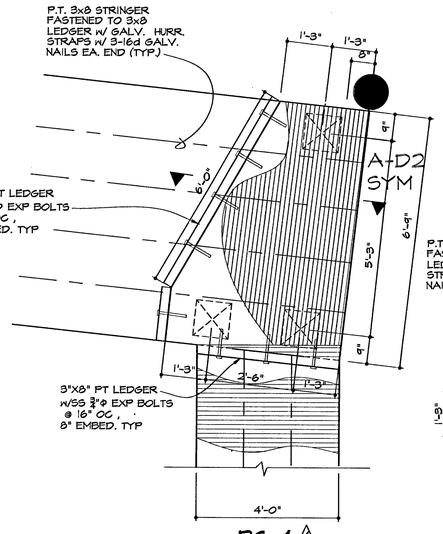


TOP AND BOT BAR PLACEMENT DETAIL
@ PC-1 & PC-2 (SEE PC SCHED.)

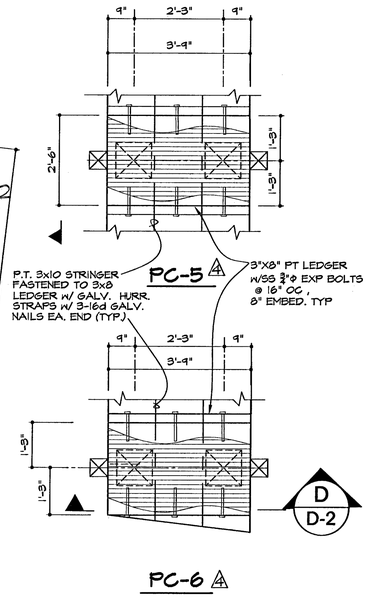
PC-2 ⚠4

PILE CAP DETAILS 1/2" = 1'-0" ⚠4

PC-3 ⚠4

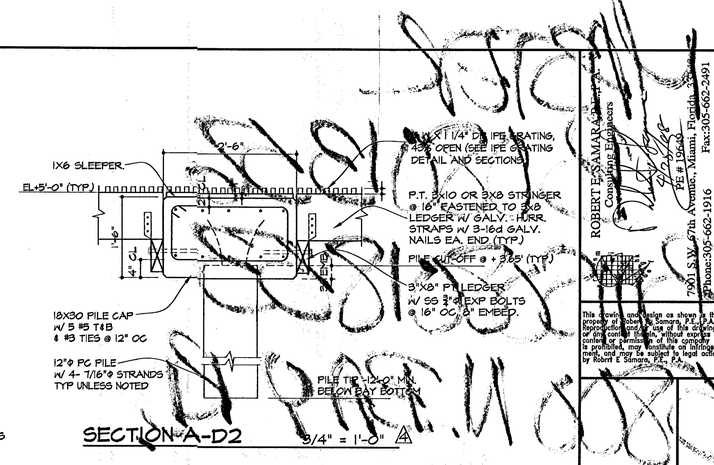


PC-4 ⚠4



PC-5 ⚠4

PC-6 ⚠4



SECTION A-D2 3/4" = 1'-0" ⚠4

CONCRETE PILE CAPS

PC-1:
3'-6" W. x 1'-6" DP. x 6'-9" LG. CONC. PILE CAP W/ 8 #5 T&B SEE BAR PLACEMENT DETAIL.

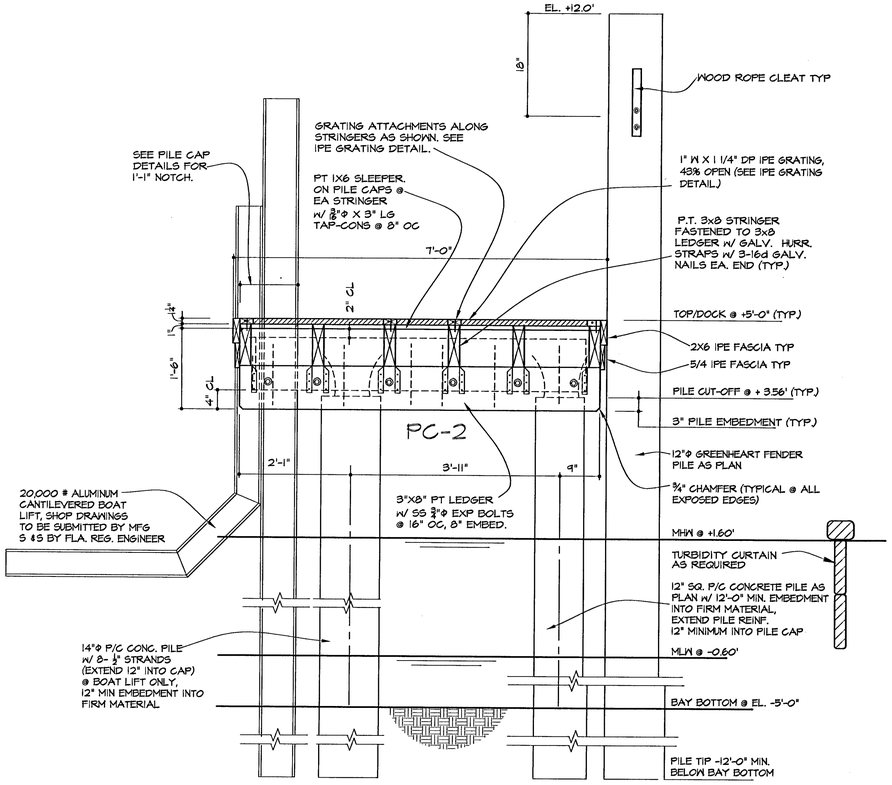
PC-2:
3'-6" W. x 1'-6" DP. x 6'-9" LG. CONC. PILE CAP W/ 8 #5 T&B & #3 TIES @ 12"o.c., SEE BAR PLACEMENT DETAIL.

PC-3:
2'-6" W. x 1'-6" DP. x 6'-9" LG. CONC. PILE CAP W/ 5 #5 T&B & #3 TIES @ 12"o.c.

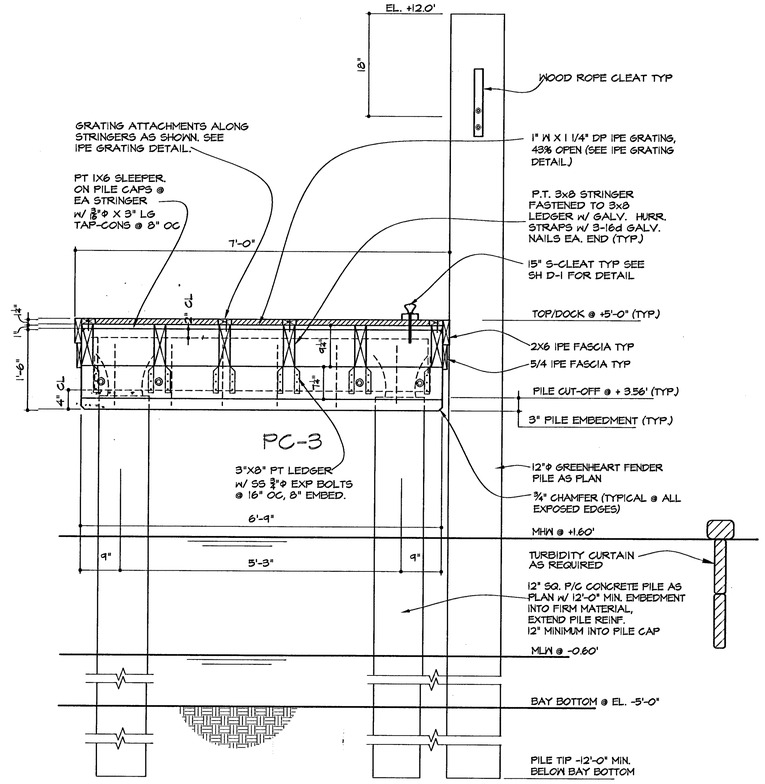
PC-4:
2'-6" W. x 6'-9" LG x 1'-6" DP. CONC. PILE CAP W/ 4 #5 T&B (3 WAYS) & #3 TIES @ 12"o.c.

PC-5:
2'-6" W. x 1'-6" DP. x 3'-9" LG. CONC. PILE CAP W/ 4 #5 T&B & #3 TIES @ 12"o.c.

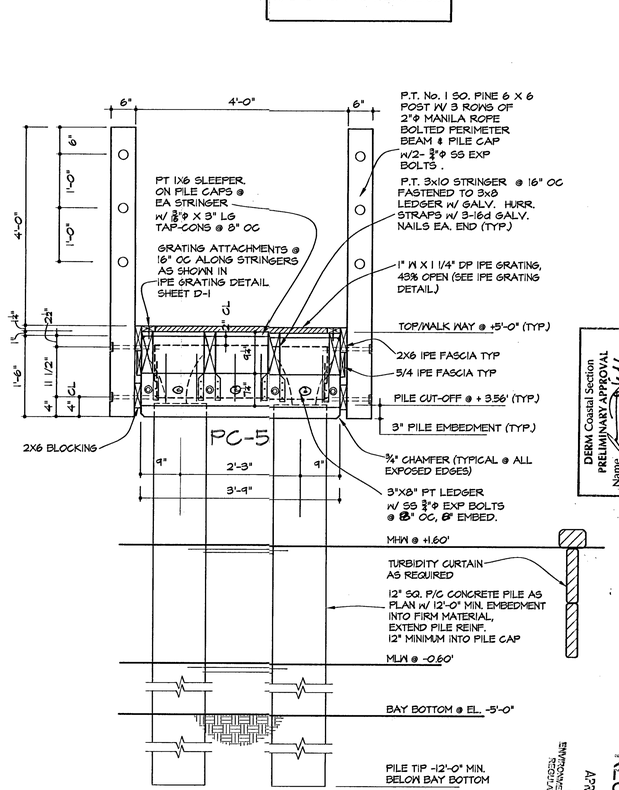
PC-6:
2'-6" W. x VARIES x 1'-6" DP. x 3'-9" LG. CONC. PILE CAP W/ 5 #5 T&B & #3 TIES @ 12"o.c.



SECTION B-D2 ⚠4 3/4" = 1'-0"



SECTION C-D2 ⚠4 3/4" = 1'-0"



SECTION D-D2 ⚠4 3/4" = 1'-0"

RECEIVED
APR 2 9 2008
DERM
ENVIRONMENTAL RESOURCES
REGULATION DIVISION

DERM Coastal Section
PRELIMINARY APPROVAL
Name: [illegible]
Date: 4/30/08

ROBERT E. SAMARA, P.E., P.A.
Consulting Engineers
PE # 19640
7900 S.W. 57th Avenue, Miami, Florida 33143
Phone:305-662-1916 Fax:305-662-2491

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JOB NAME: MARK GAINOR RESIDENCE
5800 NORTH BAY ROAD
MIAMI, FLORIDA

TITLE: NEW DOCK AND BOAT LIFT

REVISIONS:
⚠1 11-27-07
REV PER BMS & PILE CAPS
⚠2 12-17-07
MOVED DOCK & REV DETAILS
⚠3 01-28-08
REV. DECKING & BEAMS
⚠4 04-25-08
REV. FRAIMING & REMOVED CONC BEAMS

DATE: 11-07-07

DWN BY: A.B.

JOB NO: 07-70

SHEET: D-2

OF 2 SHEETS

MASTER
B0801352
BMS0801853
5800 N BAY RD.

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:

BUILDING:
ZONING: 5/1/08
DRB/HPB:
CONCURRENCY:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS: 05/9/08
STRUCTURAL: 05/14/08
ACCESSIBILITY:
LEVATOR:

As per Florida Building Code Section 104.3.3
FIRE COMPLIANCE

B08 1352
BMS 08 1853

CITY OF MIAMI BEACH
BUILDING DEPARTMENT
1700 CONVENTION CENTER DRIVE
2ND FLOOR - CITY HALL
MIAMI BEACH, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I, (we) have been retained by: Bunnell Foundation, Inc. to perform special inspector services under the Florida Building Code at the 5300 N. Bay Road project on the below listed structures as of 1/23/08 (date). I am a professional engineer licensed in the State of Florida.

Process Number: B0801516 **Master Permit (IF APPLICABLE):** _____

- [x] Special Inspector for Pilings, FBC 1822.1.20
- [ ] Special Inspector for Soil Compaction, FBC 1820.3.1
- [ ] Special Inspector for Precast Attachments, FBC 1927.12.2 (By P.E. or R.A..)
- [ ] Special Inspector for Reinforced Masonry, FBC 2122.4
- [ ] Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A..)
- [ ] Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R. A..)
- [ ] Special Inspector for Concrete Reinforcement

NOTE: Only the marked boxes apply.

The following individual's) employed by this firm or me are authorized representatives to perform inspections*

1. John Buscher P.E.
2. _____
3. _____
4. _____

***NOTE: FBC 2001 HVZ sections 1927.12.2, 2218.2, 2319.17.4.2 requires either a Registered professional Engineer or Registered Architect to perform the actual inspections.**

I, (we) will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, (we) understand that a Special Inspection Log for each building must be displayed in a convenient location on the site for reference by the City of Miami Beach Building Department Inspector. All mandatory inspections, as required of the Florida Building Code, Inspection performed by the Special inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. Further, upon completion of the work under each building permit, I will submit to the Building Inspector at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.

Architect/Engineer Signature: Robert E. Samara

Architect/Engineer Name Printed: Robert E Samara

Address: 7901 S.W. 67th Ave

Phone Number: 1305-6621916

Signed and Sealed

19649
License Number

Owner/Agent Signature: Angel Carreras

Owner/Agent Name Printed: Angel Carreras

Date: 1/23/08

Building Department Accepted By: _____

For Existing 24" thick (ave.) gravity wall, Provide New Batter pile at 9'-2" o/c for overturning support for existing wall.

A. Determine seawall retained soil characteristics and describe seawall geometry:

1) Soil Characteristics

For soil described as silty sand-gravel mix of medium density, ASTM classification GM

Soil weight, dry or moist $\gamma := 120$ psf. Water weight $\gamma_{H2O} := 62.4$ psf.

Soil weight, submerged $\gamma_{prime} := 58$ psf Friction angle, internal $\phi_{soil} := 30$ degrees

Coeff. of active earth pressure $k_a := 0.30$

Coeff. of passive earth pressure $k_p := 5.0$ S. F. for Passive pressure $G_s := 1.5$

Allowable lateral soil bearing value (coral rock) $k_{p\_allow} := 400$ psf/ft of depth

2) Seawall Geometry

Height of seawall (top of cap to top of berm) $H_{wall} := 9.0$ ft. max

Distance from top of cap to fill grade $d_{fill} := 0.5$ ft.

Height of retained fill (top of grade to top of berm) $H_f := H_{wall} - d_{fill}$ $H_f = 8.50$ ft.

Pile height (top of pile to top of berm) $H_a := H_{wall} - .5$ $H_a = 8.50$ ft.

Pile spacing (horiz. centerline spacing) $S_a := 9.17$ ft. max

Concrete Pile embedment depth (top of berm to bottom of pile) $D := 12.0$ ft.

Silt depth at top of berm $d_{silt} := 0.05 \cdot D$

Eff. pile embedment depth (for design) $D_{eff} := D - d_{silt}$ $D_{eff} = 11.40$ ft.

Wall embedment depth (top of berm to bottom of wall) $D_{wall} := 2.0$ ft.

Mean Low Water height (above bottom of berm) $H_{mlw} := 3.9$ ft.

Unbalanced water height above mean low water $H_u := 2.1$ ft.

Robert E. Samara, P.E.P.A.
Consulting Engineers

Structural
PE # 19649

12/19/07

7901 SW 67th Avenue, Suite 207
Miami FL 33143
Tel (305)-662-1916

Repair
of Existing Seawall at
Gainor Residence
5800 North Bay Road
Miami , FL

Date: December 19, 2007

B) **Determine bulkhead forces, and action line distances y_i above top of berm for design of new seawall cap and batter pile:**

a) Force due to surcharge on inside face of seawall $q_{sur} := 0.0$ psf. allowance

$$F_{sur} := \frac{q_{sur}}{1000} \cdot H_f \qquad F_{sur} = 0 \text{ kips/ft.} \qquad y_{sur} := \frac{H_f}{2}$$

b) Forces due to dry/moist backfill on inside face of seawall

$$F_{\gamma 1} := k_a \cdot \frac{\gamma}{1000} \cdot \frac{(H_f - H_{mlw})^2}{2} \qquad F_{\gamma 1} = 0.38 \text{ kips/ft.} \qquad y_{\gamma 1} := H_{mlw} + \frac{H_f - H_{mlw}}{3}$$

$$F_{\gamma 2} := k_a \cdot \frac{\gamma}{1000} \cdot (H_f - H_{mlw}) \cdot H_{mlw} \qquad F_{\gamma 2} = 0.65 \text{ kips/ft.} \qquad y_{\gamma 2} := \frac{H_{mlw}}{2}$$

c) Forces due to unbalanced water pressure on inside face of seawall

$$F_{\gamma H2O1} := \frac{\gamma_{H2O}}{1000} \cdot \frac{H_u^2}{2} \qquad F_{\gamma H2O1} = 0.14 \text{ kips/ft.} \qquad y_{\gamma H2O1} := H_{mlw} + \frac{H_u}{3}$$

$$F_{\gamma H2O2} := \frac{\gamma_{H2O}}{1000} \cdot (H_u) \cdot H_{mlw} \qquad F_{\gamma H2O2} = 0.51 \text{ kips/ft.} \qquad y_{\gamma H2O2} := \frac{H_{mlw}}{2}$$

d) Force due to submerged backfill on inside face of seawall

$$F_{\gamma prime} := k_a \cdot \frac{\gamma_{prime}}{1000} \cdot \frac{H_{mlw}^2}{2} \qquad F_{\gamma prime} = 0.13 \text{ kips/ft.} \qquad y_{\gamma prime} := \frac{H_{mlw}}{3}$$

e) Total Horizontal Force transmitted on back of wall

$$F_{total} := (F_{sur} + F_{\gamma 1} + F_{\gamma 2} + F_{\gamma H2O1} + F_{\gamma H2O2} + F_{\gamma prime}) \qquad F_{total} = 1.81 \text{ kips/ft}$$

Robert E. Samara, P.E.P.A.
Consulting Engineers

Structural
PE # 19649

12/19/07

7901 SW 67th Avenue, Suite 207
Miami FL 33143
Tel (305)-662-1916

C) **Determine Uniform Horizontal Loading on Concrete Cap Beam and Horiz. Force on Batter pile**

Take Moments about Toe of Existing Gravity Wall

a) Moment due to surcharge on inside face of seawall $y_{\gamma prime\_p} := -2.0$

$M_{sur} := F_{sur} \cdot (-y_{\gamma prime\_p} + y_{sur})$ $M_{sur} = 0.00$ ft-kips/ft.

b) Moments due to dry/moist backfill on inside face of seawall

$M_{\gamma 1} := F_{\gamma 1} \cdot (-y_{\gamma prime\_p} + y_{\gamma 1})$ $M_{\gamma 1} = 2.83$ ft-kips/ft.

$M_{\gamma 2} := F_{\gamma 2} \cdot (-y_{\gamma prime\_p} + y_{\gamma 2})$ $M_{\gamma 2} = 2.55$ ft-kips/ft.

c) Moments due to unbalanced water pressure on inside face of seawall .

$M_{\gamma H2O1} := F_{\gamma H2O1} \cdot (-y_{\gamma prime\_p} + y_{\gamma H2O1})$ $M_{\gamma H2O1} = 0.91$ ft-kips/ft.

$M_{\gamma H2O2} := F_{\gamma H2O2} \cdot (-y_{\gamma prime\_p} + y_{\gamma H2O2})$ $M_{\gamma H2O2} = 2.02$ ft-kips/ft.

d) Moment due to submerged backfill on inside face of seawall

$M_{\gamma prime} := F_{\gamma prime} \cdot (-y_{\gamma prime\_p} + y_{\gamma prime})$ $M_{\gamma prime} = 0.44$ ft-kips/ft.

e) Sum of Moments due to pressures on inside face of seawall , about point b:

$M_{sum} := M_{sur} + M_{\gamma 1} + M_{\gamma 2} + M_{\gamma H2O1} + M_{\gamma H2O2} + M_{\gamma prime}$

$M_{sum} = 8.75$ ft-kips/ft.

f) **Horizontal Loading Along Concrete Cap Beam , w_h:**

$w_h := \dfrac{M_{sum}}{-y_{\gamma prime\_p} + H_a}$ $w_h = 0.83$ klf.

g) **Horiz. Reaction on New Batter Pile, for Pile Spacing** $S_a = 9.17$ ft.
New Batter pile to take 100% of total lateral load:

$F_{reqd} := (w_h \cdot S_a) \cdot 1.0$ $F_{reqd} = 7.64$ kips. horizontal force on Batter pile

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D) Determine Required Concrete Cap Beam Size and Reinforcing:

a) Maximum Shear and Bending Moment, assume 2 span cont.:

$V_u := 1.15 \cdot 1.7 \cdot w_h \cdot \frac{S_a}{2}$ $V_u = 7.47$ kips. $M_u := 1.7 \cdot w_h \cdot \frac{S_a^2}{8}$ $M_u = 14.88$ ft-kips.

b) Check longditudinal steel and shear steel in cap beam for horizontal loading:

beam size is 42" wide x 21" deep (min) with 6#5's top and bottom eq. spaced and with 1 #5 intermediate at outside face

$f_c := 5000$ psi. $f_y := 60000$ psi. $b := 21$ in. $w := 42$ in. $\phi_{flex} := 0.9$ $\phi_{shear} := 0.85$

$d_{face} := w - 4.0$ in., to provide 3" cover (min) on # 3 ties. $d_{face} = 38.00$ in.

$\phi V_c := \frac{\phi_{shear}}{1000} \cdot 2 \cdot \sqrt{f_c} \cdot b \cdot d_{face}$ $\phi V_c = 95.93$ kips

$\frac{\phi V_c}{2} = 47.96$ kips. > $V_u = 7.47$ kips, therefore no shear reinforcement is required.

however, provide #3 closed ties at 24" o.c. for transverse crack control., and 6 #3 closed ties on each side of each new batter pile (see calculation below for required ties to resist eccentricity between wall and batter pile)

$$Aface_{s\_req} := \frac{b \cdot d_{face} \cdot f_c - \sqrt{(b \cdot d_{face} \cdot f_c)^2 - 2 \cdot b \cdot f_c \cdot \frac{M_u \cdot 12000}{\phi_{flex}}}}{f_y}$$

$Aface_{s\_req} = 0.09$ in² $A_{5\_bar} := 0.31$ in.²/bar $Aface_{s\_prov} := 3 \cdot A_{5\_bar}$

$Aface_{s\_prov} = 0.93$ in.² > $Aface_{s\_req} = 0.09$ in² **OK for 6 #5's top and bottom, and 1 #5 bar at mid-depth of seaward vertical face; consider out board bars to resist horiz. bending.**

E) Determine Required Axial Load Capacity in Batter Pile, for new pile to carry 100% of lateral load and existing seawall to carry 0% of lateral load,

$F_{reqd} = 7.64$ kips $Rise := 12$ in. $Batter := 3.0$ in. $Diag := (Rise^2 + Batter^2)^{0.5}$ $Diag = 12.37$

$P_{reqd} := \frac{Rise}{Batter} \cdot F_{reqd}$ $P_{reqd} = 30.55$ kips

$Tons_{reqd} := \frac{P_{reqd}}{2}$ $Tons_{reqd} = 15.28$ tons < $Tons_{allow} := 25$ tons, per FBC, for 12" x 12" pile O.K.

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Determine Required rebar dowels embedded in existing wall:

$Diag := \left(Rise^2 + Batter^2\right)^{0.5}$ $Diag = 12.37$ in.

$T_{u\_reqd} := 1.7 \frac{12}{Diag} \cdot P_{reqd}$ $T_{u\_reqd} = 50.39$ kips $\phi := 0.9$ $F_y := 60$ ksi

Determine Required Tension to be developed in new steel rebar dowels embedded in existing wall wall:

$DL_{wall} := \frac{24 \cdot (9.5 \cdot 12) \cdot S_a}{144} \cdot .15$ for 24" ave thickness x 9.5' high wall $DL_{wall} = 26.13$ kips over $S_a = 9.17$ ft.

$DL_{cap} := \frac{21 \cdot 42 \cdot S_a}{144} \cdot .15$ $DL_{cap} = 8.42$ kips over $S_a = 9.17$ ft.

$T_{u\_dowel\_reqd} := T_{u\_reqd} - \left(DL_{wall} + DL_{cap}\right)$ $T_{u\_dowel\_reqd} = 15.83$ kips

Per Hilti Product Technical Guide/3000 "Hilti" HIT HY 150" Injection Adhesive Anchor System, Table 4.2.2 on page 70, Ultimate Bond Strength and Steel Strength for Rebar in Concrete

$A_{bar} := 0.44$ for #6 rebar dowel $A_{bar} \cdot F_y = 26.40$ kips yield strength <—— controls

Minimum embedment in 3000 psi (min) concrete to develop ultimate bond strength for #6 Rebar = 10.0 in.

$A_{bar\_bond} := \frac{35.6}{4}$ $A_{bar\_bond} = 8.90$ kips in 3000 psi minimum concrete

$A_{bar} := 0.44$ for #6 rebar dowel $A_{bar} \cdot F_y = 26.40$ kips yield strength $\phi = 0.90$ 8

$T_{n\_prov} := \left(\phi \cdot A_{bar\_bond} \cdot 5.0\right)$ for #6 dowels spaced at 24" o/c along top of 9.33 ft tributary length of existing wall

$T_{n\_prov} = 40.05$ kips/ for #6 dowels at 24"o/c max with 12" min. epoxy embedment into existing wall along Sa = 9.33 ft > $T_{u\_dowel\_reqd} = 15.83$ kips, O.K

Therefore, provide #6 Hooked Dowels at 24" o/c, max with 12" min. epoxy embedment into wall

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Considering the eccentricity between front batter pile and rear Gravity Wall, determine required steel reinforcing at bottom face of cap beam to resist upward moment induced by eccentricity

$C_{u\_pile} := 1.5\frac{12}{Diag}\cdot P_{reqd}$ $C_{u\_pile} = 44.46$ kips

ecc := 1.25 ft, cl of batter pile to cl of wall

$M_{u\_ecc} := C_{u\_pile}\cdot ecc$ $M_{u\_ecc} = 55.57$ ft/kips

$b_{eff} := 24$ inches, consider 12" to left and right of pile location

$d := 20 - 4$ $d = 16.00$ in

$\phi_{flex} := 0.85$ $f_c := 5000$ psi. $f_y := 40000$ psi. for #3 ties

$$Abot_{s\_req} := \frac{b_{eff}\cdot d\cdot f_c - \sqrt{(b_{eff}\cdot d\cdot f_c)^2 - 2\cdot b_{eff}\cdot f_c\cdot\frac{M_{u\_ecc}\cdot 12000}{\phi_{flex}}}}{f_y}$$

$Abot_{s\_req} = 1.17$ in^2 $A_{3\_bar} := 0.11$ in.^{2}/bar $Abot_{s\_prov} := 12\cdot A_{3\_bar}$

$Abot_{s\_prov} = 1.32$ in.^{2} **OK for 6 #3 closed ties at each side of pile location. (install at 3" o/c each side of pile)**

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