

TABLE 1—HURRICANE TIES

MODEL NO.	FASTENERS (Quantity-Type)			ALLOWABLE LOADS ^{1,2} (lbs)			
	To Rafter	To Plates	To Stud	Connection Configurations ³	Uplift ^{4,5} $C_D=1.33$ or $C_D=1.6$	Lateral ^{6,7} $C_D=1.33$ or $C_D=1.6$	
						F_1	F_2
H1	6-8dx1½	4-8d	—	1	490	485	465
H10	8-8dx1½	8-8dx1½	—		905	585	525
H10R	8-8dx1½	8-8dx1½	—		905	585	525
H10-2	6-10d	6-10d	—		760	455	395
H2	5-8d	—	5-8d	2	335	—	—
H2.5	5-8d	5-8d	—	3	415	150	150
	—	5-8d	5-8d	8	370	—	—
H2.5A	5-8d	5-8d	—	3	600	110	110
H3	4-8d	4-8d	—		455	125	160
H5	4-8d	4-8d	—		455	115	200
H4	—	4-8d	4-8d	8	360	—	—
	4-8d	4-8d	—	9	360	165	160
H6	—	8-8d	8-8d	12	915	—	—
H7Z	4-8d	2-8d	8-8d	13	930	400	—
H15	4-10dx1½	4-10dx1½	12-10dx1½		1,300	480	—
H15-2	4-10dx1½	4-10dx1½	12-10dx1½		1,300	480	—

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

- Allowable loads are for one anchor installed to a minimum nominal 2x supported and minimum nominal 2x supporting wood member. A rafter minimum actual thickness of 2½ inches must be used when framing anchors are installed on each side of the rafter and on the same side of the plate.
- Allowable simultaneous loads in more than one direction on a single connector must be evaluated as follows:
Design Uplift / Allowable Uplift +
Design Lateral Parallel to Plate / Allowable Lateral Parallel to Plate +
Design Lateral Perpendicular to Plate / Allowable Lateral Perpendicular to Plate ≤ 1.0.
The three terms in the unity equation consider all possible forces that the hurricane tie may be designed and installed to resist. The number of terms that must be considered for simultaneous loading is determined by the registered design professional and is dependant on the method of calculating wind forces and the assumed load path that the connector is designed to resist.
- "Connection Configurations" shown in Figure 1c (next page) indicate the load directions F_1 and F_2 , and are details showing connector installations on the outside of the wall for clarity. Installation on the inside of the wall is acceptable to achieve the tabulated allowable loads.
- Connections in the same area (i.e. truss to plate connector and plate to stud connector) must be on installed on the same side of the wall to achieve the tabulated allowable uplift loads and ensure a continuous load path.
- Allowable uplift loads have been increased for wind or earthquake loading, and no further increase is allowed. Allowable loads must be reduced when other load durations govern.
- Allowable lateral loads in the F_1 direction must not be used to replace diaphragm boundary members or nailing or replace solid blocking required by code to laterally support the ends of joists/rafters.
- Additional shear transfer elements must be considered the connector installation induces cross grain bending or tension of the truss or rafter members.

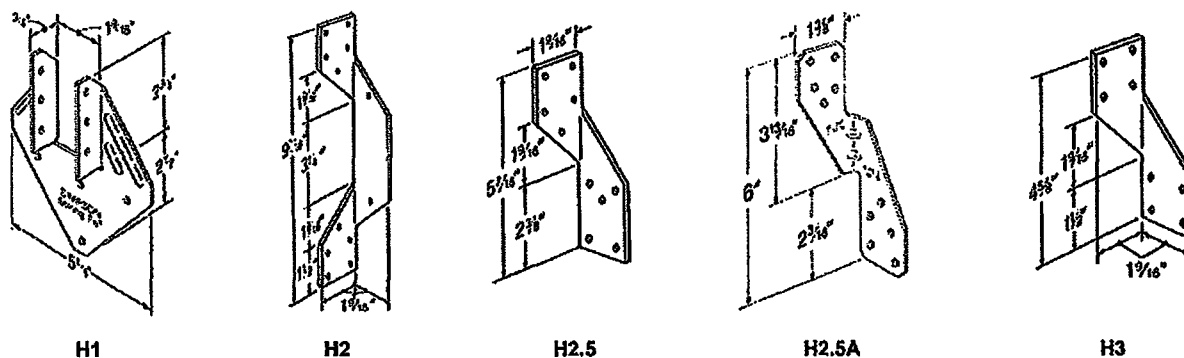


FIGURE 1a—H1, H2, H2.5, H2.5A, AND H3 HURRICANE TIES

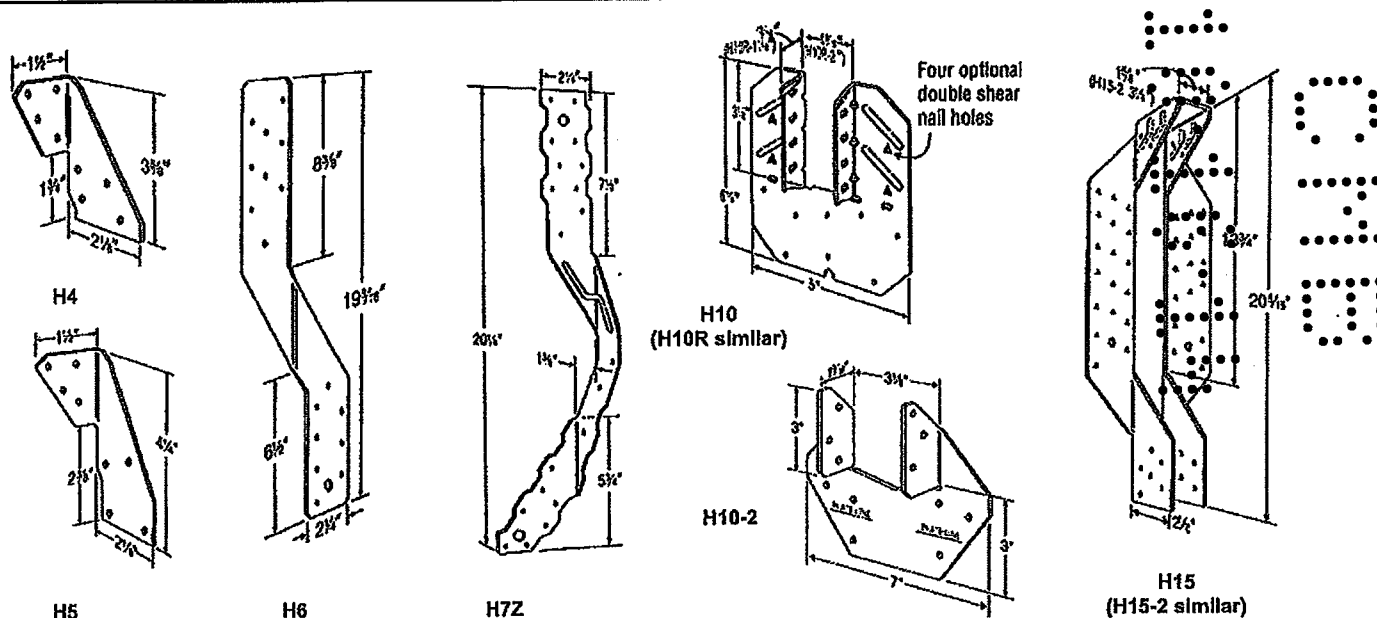


FIGURE 1b—H4, H5, H6, H7Z, H10, H10-2, AND H15 HURRICANE TIES

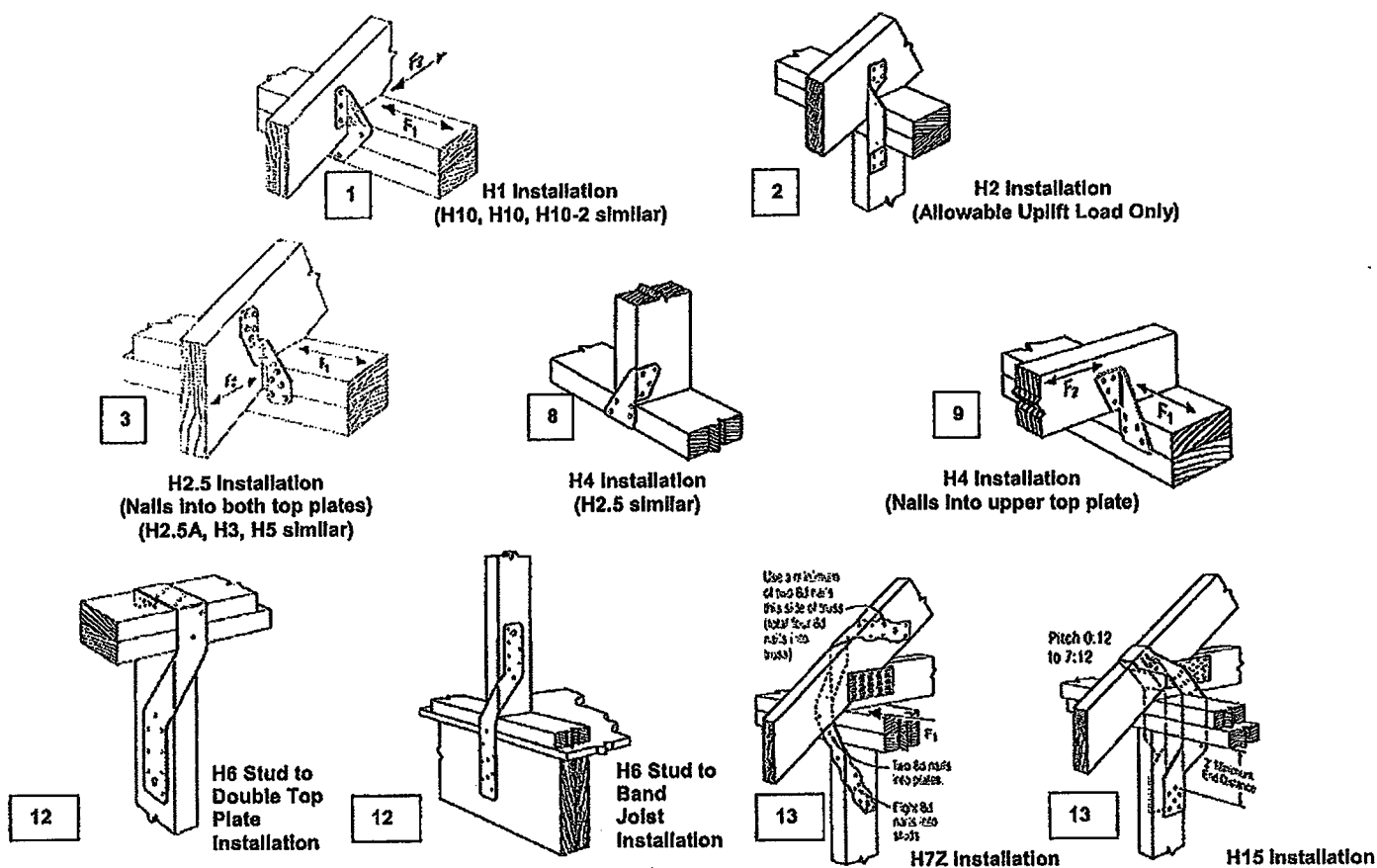


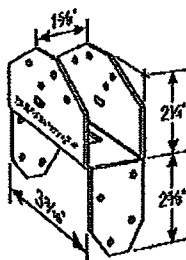
FIGURE 1c—CONNECTION CONFIGURATIONS OF HURRICANE TIE INSTALLATIONS SPECIFIED IN TABLE 1

TABLE 2—HS24 HURRICANE TIE^{1,2,3}

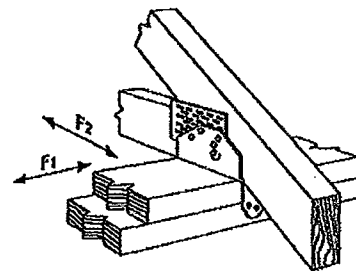
MODEL NO.	FASTENERS ¹ (Quantity-Type)		ALLOWABLE LOADS (lbs) where $C_D=1.33$ or $C_D=1.6$		
	To Rafter or Truss	To Double Top Plate	Uplift ²	Lateral ^{3,4}	
				F_1	F_2
HS24	8-8dx1 $\frac{1}{2}$ & 2-8d (slant)	8-8d	605	645	1,026
	8-8dx1 $\frac{1}{2}$	8-8d	605	590	640

For SI: 1 inch = 25.4 mm, 1 lbs = 4.5 N.

1. "Slant" nailing refers to 8d common nails installed as toenails on each side of the connector. The nails must be driven through the connector at an angle approximately 30° with the rafter/truss member with the nail penetrating through the rafter/truss member into the wood double top plate.
2. The uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.
3. Allowable lateral loads in the F_1 direction must not be used to replace diaphragm boundary members or nailing or replace solid blocking required by code to laterally support the ends of joists/rafters.
4. Additional shear transfer elements must be considered the connector installation induces cross grain bending or tension of the truss or rafter member.
5. F_1 load direction is parallel to plate, and F_2 load direction is perpendicular to plate.



HS24 Dimensions



HS24 Installation and Allowable Load Directions

FIGURE 2—HS24 HURRICANE TIE

TABLE 3—RST HURRICANE TIES

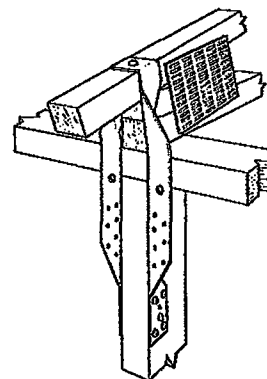
MODEL NO.	RST HURRICANE TIE DIMENSIONS (in)		FASTENERS (Quantity-Type)		ALLOWABLE UPLIFT LOADS ^{1,2} (lbs) $C_D = 1.33$ or $C_D = 1.6$
	(W)	(L)	To Rafter or Truss	To Stud	
RST-1	1 $\frac{1}{2}$	12 $\frac{1}{2}$	1-10dx1 $\frac{1}{2}$	10-10dx1 $\frac{1}{2}$	550
RST-2	3 $\frac{1}{8}$	11 $\frac{11}{16}$	2-10dx1 $\frac{1}{2}$	12-10dx1 $\frac{1}{2}$	550

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

1. Allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.
2. Allowable uplift loads are based on lumber having an assigned specific gravity of 0.55, such as Southern Pine (SP).



RST-1 (RST-2 similar)



RST-1 Installation

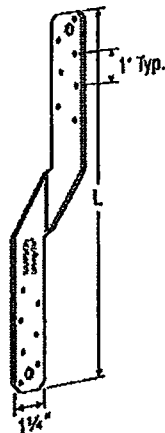
FIGURE 3—RST HURRICANE TIE

TABLE 4—LTS, MTS, AND HTS TWIST STRAPS

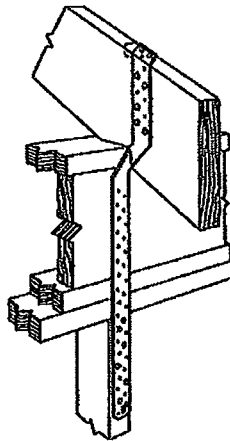
TWIST STRAP SERIES	MODEL NO.	STRAP LENGTH (in)	TOTAL QUANTITY OF FASTENERS ¹		ALLOWABLE UPLIFT LOADS ^{2,3} (lbs)		
			When Installed with 10d Common Nails	When Installed with 10dx1½ Common Nails	When Installed with 10d Common Nails	When Installed with 10dx1½ Common Nails	
					C _D = 1.33 or C _D = 1.6	C _D = 1.33	C _D = 1.6
LTS ⁴	LTS12	12	12	12	775	720	720
	LTS16	16					
	LTS18	18					
	LTS20	20					
MTS ⁴	MTS12	12	14	14	1,000	840	1,000
	MTS16	16					
	MTS18	18					
	MTS20	20					
	MTS30	30					
HTS ⁵	HTS16	16	16	16	1,260	1,005	1,150
	HTS20	20	20	24	1,450	1,450	1,450
	HTS24	24					
	HTS28	28					
	HTS30	30					
	HTS30C	30					

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N

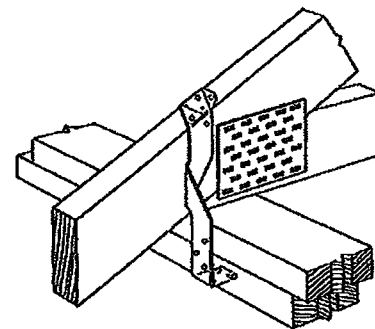
1. Half of the fasteners must be installed on each end of the strap to achieve the allowable uplift load.
2. Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.
3. Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.
4. Each model of the LTS and MTS twist strap series (except for the MTS30) has more nail holes than the minimum quantity of nails specified in the table.
5. HTS30C has the twist in the center of the strap length.



LTS12
(MTS and HTS Similar)



Typical MTS Installation –
Rafter to Stud



Typical MTS Installation –
Truss to Double Top Plate

FIGURE 4—TWIST STRAPS

TABLE 5—LFTA LIGHT FLOOR TIE ANCHOR¹

MODEL NO.	LFTA ANCHOR DIMENSIONS (in)			FASTENERS ² (Quantity-Type)	ALLOWABLE TENSION LOAD ^{4,5} (lbs) $C_D = 1.33$ or $C_D = 1.6$
	Strap Width (W)	Clear Span	Overall Length (L)		
LFTA	2½	17	38¾	16-10d Common	1,205

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N

1. The LFTA anchor is used to transfer tension forces between vertically aligned wood studs across floor framing with floor joists having a maximum nominal depth of 12 inches.
2. Half of the fasteners must be installed on each end of the strap to achieve the allowable uplift load.
3. Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.
4. Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.

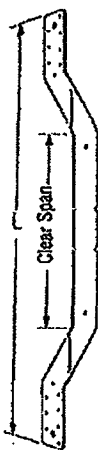


FIGURE 5—LFTA LIGHT FLOOR TIE ANCHOR (See Table 5)

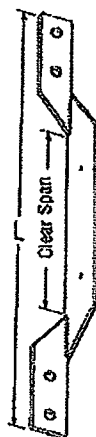


FIGURE 6a—FTA FLOOR TIE ANCHOR (See Table 6)



FIGURE 6b—FTA FLOOR TIE ANCHOR INSTALLATION (See Table 6)

TABLE 6—FTA FLOOR TIE ANCHORS¹

MODEL NO.	FTA ANCHOR DIMENSIONS (in)			FASTENERS ^{2,3} (Quantity-Type)	ALLOWABLE TENSION LOADS ^{4,5} (lbs)									
	Tie Strap Width	Clear Span	Overall Length		When Lumber Thickness, t _m = (Inches)									
					1½	2	2½		3	3½				
					Where C _D =									
					1.33	1.6	1.33	1.6	1.33	1.6	1.33	1.6	1.33	1.6
FTA2	3	17	37½	4-5/8" dia. M.B.	1,575	1,890	2,095	2,515	2,600	3,120	2,820	3,385	2,820	3,385
FTA5	3½	17	45½	4-3/4" dia. M.B.	1,865	2,240	2,500	3,000	3,125	3,750	3,725	4,400	4,050	4,400
FTA7	3½	17	56	6-7/8" dia. M.B.	3,095	3,715	4,185	5,020	5,175	6,210	6,360	7,600	7,395	7,600

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N, 1 psi = 6.89 kPa.

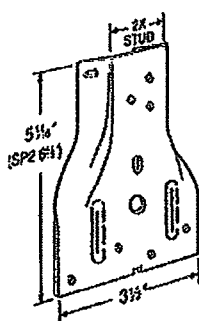
1. FTA anchors must be located on the vertical wood studs/posts so that the minimum end distance of the first bolt is equal to or greater than 4¾ inches, i.e., seven times the bolt diameter (7D).
2. Machine Bolts (M.B.) must comply with ANSI/ASME Standard B18.2.1.
3. A washer, not less than a standard cut washer, is required on the lumber side opposite the FTA between the wood and the nut.
4. Tabulated allowable tension loads must be selected based on duration of load as permitted by the applicable building code.
5. Tabulated allowable tension loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.

TABLE 7—SP AND SPH STUD PLATE TIES

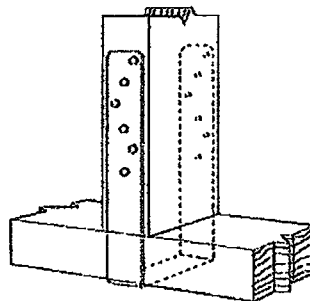
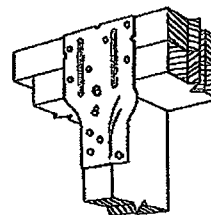
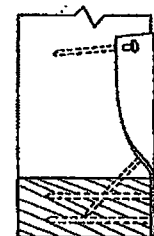
CONNECTOR SERIES	MODEL NO.	CONNECTOR DIMENSIONS (in)		FASTENERS ¹ (Quantity-Type)		ALLOWABLE UPLIFT LOADS ^{2,3} (lbs)			
		(W)	(L)	To Stud	To Plate	C _D = 1.33		C _D = 1.6	
						S.G.=0.50	S.G.=0.55	S.G.=0.50	S.G.=0.55
SP	SP1	—	—	6-10d	4-10d	585	585	585	585
	SP2	—	—	6-10d	6-10d	890	890	1,065	1,065
	SP4	3 ⁹ / ₁₆	7 ¹ / ₄	6-10dx1 ¹ / ₂	—	735	735	885	885
				6-16dx2 ¹ / ₂	—	800	800	930	930
	SP6	5 ⁹ / ₁₆	7 ³ / ₄	6-10dx1 ¹ / ₂	—	735	735	885	885
				6-16dx2 ¹ / ₂	—	800	800	930	930
SPH	SPH4	3 ⁹ / ₁₆	8 ³ / ₄	10-10dx1 ¹ / ₂	—	—	1,240	—	1,240
				12-10dx1 ¹ / ₂	—	1,360	1,490	1,360	1,490
	SPH6	5 ⁹ / ₁₆	9 ¹ / ₄	10-10dx1 ¹ / ₂	—	—	1,240	—	1,240
				12-10dx1 ¹ / ₂	—	1,360	1,490	1,360	1,490
	SPH8	7 ⁵ / ₁₆	8 ³ / ₈	10-10dx1 ¹ / ₂	—	—	1,240	—	1,240
				12-10dx1 ¹ / ₂	—	1,360	1,490	1,360	1,490

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

1. For Models SP1 and SP2, one 10d common stud nail must be installed as a toenail. It must be driven through the connector at an angle approximately 30° with the stud with the nail penetrating through the stud into the wood sill plate. (See detail on this page entitled "SP1 Nailing Profile.")
2. Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.
3. Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.
4. Allowable uplift loads are given for wood assemblies consisting of lumber having an assigned specific gravity (S.G.) of 0.50, such as Douglas fir-larch, and 0.55, such as southern pine.



SP1/SP2

SP1 Installation:
Stud to Sill PlateSP2 Installation:
Stud to Double Top Plate

SP1 Nailing Profile

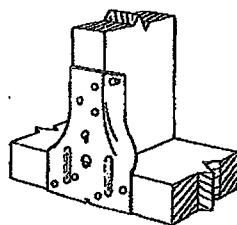
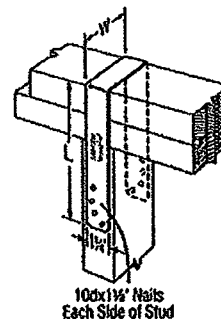
Typical SPH4 Installation:
Stud to Wood Sill PlateTypical SP4 Installation:
Double Top Plate to Stud (SPH Similar)

FIGURE 7—SP AND SPH STUD PLATE TIES

TABLE 8—RSP4 REVERSABLE STUD PLATE CONNECTOR¹

MODEL NO.	FASTENERS (Quantity-Type)		ALLOWABLE LOADS ^{2,3} (lbs) where $C_D = 1.33$ or $C_D = 1.6$			
	To Stud	To Plate	Connection Configuration ⁴	Uplift	Lateral ⁵	
					F ₁	F ₂
RSP4	4-8dx1 ¹ / ₂	4-8dx1 ¹ / ₂	Stud to Double Top Plate	450	210	250
			Stud to Sill Plate	315	210	250

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

1. Refer to Figure 8a for overall dimensions of the RSP4 plate connector.
2. Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.
3. Tabulated allowable loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.
4. Refer to Figure 8b and 8c for connection configurations.
5. F₁ load direction is parallel to plate, and F₂ load direction is perpendicular to plate.



Figure 8a—RSP4 Stud Plate Connector Dimensions

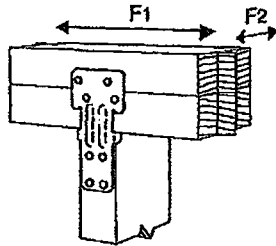


Figure 8b—RSP4 Installation: Stud to Double Top Plate

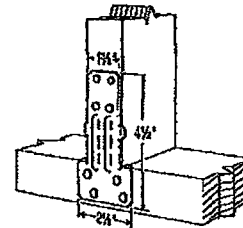


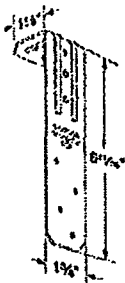
Figure 8c—RSP4 Installation: Stud to Sill Plate

TABLE 9—SSP AND DSP STUD PLATE TIE CONNECTORS

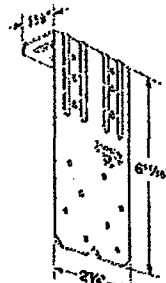
MODEL NO.	FASTENERS (Quantity-Type)			ALLOWABLE UPLIFT LOADS ^{1,2,3,4} (lbs) Where $C_D = 1.33$ or $C_D = 1.6$		
	Studs	Double Top Plate	Sill Plate	Double Top Plate	Sill Plate	
				S.G. = 0.50	S.G. = 0.50	S.G. = 0.43
SSP	4-10dx1 ¹ / ₂	3-10dx1 ¹ / ₂	—	350	—	—
		—	1-10dx1 ¹ / ₂	—	420	325
	4-10d	3-10d	—	435	—	—
		—	1-10d	—	455	420
DSP	8-10dx1 ¹ / ₂	6-10dx1 ¹ / ₂	—	775	—	—
		—	2-10dx1 ¹ / ₂	—	660	545
	8-10d	6-10d	—	825	—	—
		—	2-10d	—	825	600

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

1. Tabulated allowable uplift loads have been increased for wind or earthquake loading no further increase allowed. Reduce loads when other load durations govern.
2. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
3. For Sill Plate allowable uplift loads, all round nail holes in the connector must be filled with the specified quantity and type of nails.
4. For Double Top Plate allowable uplift loads, all round and triangular nail holes the tie connectors must be filled with the specified quantity and type of nails.



SSP



DSP

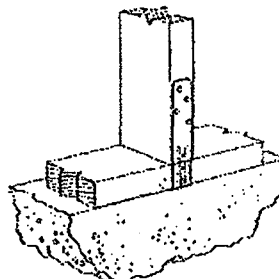
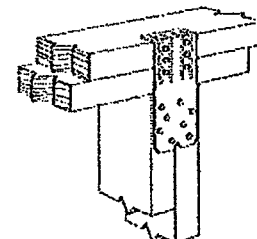
SSP Installation:
Single Stud to Sill PlateDSP Installation:
Double Stud to Double Top Plate

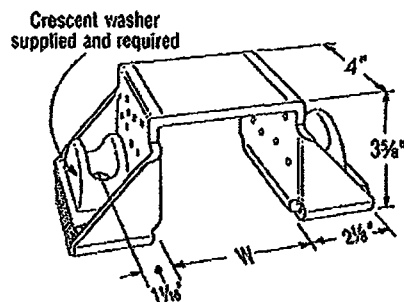
FIGURE 9—SSP/DSP STUD PLATE TIES

TABLE 9—HGT HEAVY GIRDER TIEDOWN CONNECTORS^{1,2}

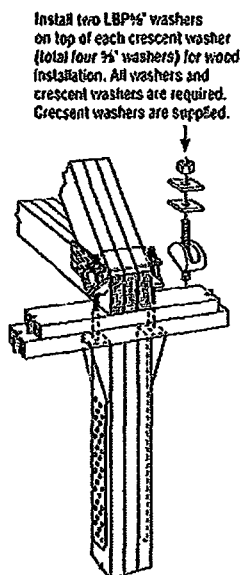
MODEL ^{3,4} NO.	HGT CONNECTOR WIDTH (W) (In.)	DISTANCE BETWEEN ANCHOR RODS (on center) (Inches)	FASTENERS (Quantity-Type)		ALLOWABLE UPLIFT LOADS ^{5,6,7} (lbs) Where $C_D=1.33$ or $C_D=1.6$
			Anchor Rod ⁸	To Multi-ply Truss	
HGT-2	3 ⁵ / ₁₆	5 ³ / ₄	2- ⁵ / ₁₆ " Dia.	16-10d	10,980
HGT-3	4 ¹⁵ / ₁₆	7 ³ / ₈	2- ⁵ / ₁₆ " Dia.	16-10d	10,530
HGT-4	6 ⁹ / ₁₆	9	2- ⁵ / ₁₆ " Dia.	16-10d	9,520

For SI: 1 inch = 25.4mm, 1 lbs = 4.45 N.

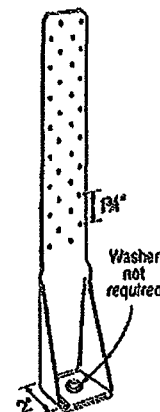
1. The HGT connector can accommodate top chord slopes from minimum 3:12 (14°) to maximum 8:12 (34°) and are provided with crescent washers for sloped top chord installations.
2. All elements of the tie-down assembly (multi-ply trusses, vertically aligned wood studs/posts, and the hold-down connectors attached to the wood studs/posts) must be designed to resist applied loads.
3. The HGT-2, HGT-3, and HGT-4 connector attaches to the heel joint of a two-ply, three-ply, and four-ply wood truss, respectively, where each ply thickness is nominal 2 inches.
4. When the HGT-3 is used with a two-ply truss, shimming is required, and the shimming material must be similar (thickness and grade of lumber) as the truss member material. Additionally, the entire assembly must be designed by a registered design professional to act as one unit.
5. Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.
6. The uplift loads have been increased for wind or earthquake loading with no further increase is allowed. Reduce loads when other load durations govern.
7. Anchor rods must be used to connect the HGT connector to the hold-down or tie-down devices attached to the vertically aligned wood studs/post. The anchor rod material specifications must be identified by the registered design professional.
8. Two LBP ⁵/₁₆-inch washers must be installed on top of each crescent washer. LBP washers and crescent washers are required. Crescent washers are supplied with the connector. LBP ⁵/₁₆ washers are available from Simpson Strong-Tie Company, and are 2-inch square by ⁵/₁₆-inch thick galvanized steel washers with a center bolt hole to accommodate a ⁵/₁₆-inch diameter threaded bolt/rod.



HGT-2
(HGT-3 and HGT-4 similar)



Typical HGT-3 installation, with HTT22 Tension Tie connectors used to attach the HGT Tiedown to the vertically aligned wood post/column. The design of the HTT22 tension tie connectors attached to the wood post/column is outside the scope of this report.



HTT22 Tension Tie
(Not covered in this
Evaluation Report.)

FIGURE 9—HGT HEAVY GIRDER TIEDOWN CONNECTOR

Miami-Dade County Building**Product Control Search Results****QUERY PARAMETERS**

File Classification: High velocity hurricane zone

Applicant: None

Category: None

Subcategory: A/C Stands

Material: None

Impact Rate: None

Maximum Design Pressure(+): 0.00

Maximum Design Pressure(-): 0.00

NOA	APPLICANT	CATEGORY	SUBCATEGORY	MATERIAL	DESCRIPTION	IMPACT	MDP+	MDP-	CLASS_DESC	EXPIRES
11-0824.01 [Library/productcontrol/noa/11082401.pdf]	F & L Aluminum Parts, Inc. (pc-result_detail_app.asp?app_alias=102416)	Roof	A/C Stands	Aluminum	Aluminum Roof Mounted Stand Frame Support for Air Conditioning Units	[None]	0	0	High velocity hurricane zone	December 28 2016
13-0321.06 [Library/productcontrol/noa/13032106.pdf]	F & L Aluminum Parts, Inc. (pc-result_detail_app.asp?app_alias=102416)	Roof	A/C Stands	Aluminum	Aluminum Roof Top Stands	[None]	0	0	High velocity hurricane zone	May 15 2019
13-1017.09 [Library/productcontrol/noa/13101709.pdf]	Miami Tech, Inc. (pc-result_detail_app.asp?app_alias=100897)	Roof	A/C Stands	Aluminum	Aluminum A/C Stand	[None]	0	0	High velocity hurricane zone	January 15 2019
14-0123.13 [Library/productcontrol/noa/14012313.pdf]	Metakum Enterprises, Inc. (pc-result_detail_app.asp?app_alias=102642)	Structural Steel and Misc. Metals	A/C Stands	Aluminum	Aluminum Roof Stand Frame Support for A/C Condensing Units	[None]	0	0	High velocity hurricane zone	June 28 2017
									Recs	4

Page Last Edited: Tue May 28, 2013 11:10:18 AM





MIAMI-DADE COUNTY
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

www.miamidade.gov

NOTICE OF ACCEPTANCE (NOA)

F & L Aluminum Parts, Inc.
1720 N.W. 22nd Court, Unit #3
Pompano Beach, Florida 33069

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed and accepted by Miami-Dade County PERA-Product Control Section to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Section (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. PERA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Section that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Aluminum Roof Mounted Stand Frame Support for Air Conditioning Units

APPROVAL DOCUMENT: Drawing No. FNL11003, titled "Aluminum Stands for Rooftop Equipment, Square Posts", sheets 1 through 3 of 3, prepared by Nu-Wind Engineering, dated July 15, 2011, signed and sealed by Christian Langley, P.E., on March 07, 2012, bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and the expiration date by the Miami-Dade County Product Control Section.

MISSILE IMPACT RATING: None

LABELING: Each stand frame shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA revises & renews NOA # 09-0709.04 and consists of this page 1, evidence submitted pages E-1 & E-2 as well as approval document mentioned above.

The submitted documentation was reviewed by Helmy A. Makar, P.E., M.S.



Helmy A. Makar
04/12/2012

NOA No. 11-0824.01
Expiration Date: 12/28/2016
Approval Date: 04/12/2012
Page 1

F & L Aluminum Parts, Inc.**NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED**

1. **EVIDENCE SUBMITTED UNDER PREVIOUS APPROVAL #06-0922.03**
 - A. **DRAWINGS**
 1. *Drawing No. 06-501, titled " Air Conditioning Stands ", sheets 1 through 3 of 3, prepared by Thornton Tomasetti, dated September 13, 2006, signed and sealed by John W. Knezevich, P.E.*
 - B. **TESTS**
 1. *None.*
 - C. **CALCULATIONS**
 1. *Calculation titled " Air Conditioning Stands Calculations ", dated September 15, 2006, sheets 1 through 160 of 160, signed and sealed by J. W. Knezevich, P.E.*
 - D. **QUALITY ASSURANCE**
 1. *By Miami-Dade County Building Code Compliance Office.*
 - E. **MATERIAL CERTIFICATIONS**
 1. *None.*
2. **EVIDENCE SUBMITTED UNDER PREVIOUS APPROVAL # 09-0709.04**
 - A. **DRAWINGS**
 1. *Drawing No. S-1, titled " Air Conditioning Stands Florida ", sheets 1 through 3 of 3, prepared by Milton Cubas, P.E., Inc., dated May 12, 2009, signed and sealed by Milton Cubas, P.E., on December 02, 2009.*
 - B. **TESTS**
 1. *None.*
 - C. **CALCULATIONS**
 1. *Calculation titled " Air Conditioning Stands ", dated May 13, 2009, sheets 1 through 206 of 206, signed and sealed by Milton Cubas, P.E.*
 - D. **QUALITY ASSURANCE**
 1. *By Miami-Dade County Building Code Compliance Office.*
 - E. **MATERIAL CERTIFICATIONS**
 1. *None.*


 Jeremy A. Makar, P. E., M.S.
 PERA, Product Control Unit Supervisor
 NOA No. 11-0824.01
 Expiration Date: 12/28/2016
 Approval Date: 04/12/2012

F & L Aluminum Parts, Inc.NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED**3. NEW EVIDENCE SUBMITTED****A. DRAWINGS**

1. *Drawing No. FNL.11003, titled "Aluminum Stands for Rooftop Equipment, Square Posts", sheets 1 through 3 of 3, prepared by Nu-Wind Engineering, dated July 15, 2011, signed and sealed by Christian Langley, P.E., on March 07, 2012.*

B. TESTS

1. *None.*

C. CALCULATIONS

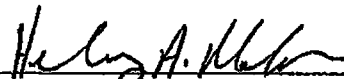
1. *Calculation titled "Air Conditioning Stands Calculations", dated August 10, 2011, sheets 1 through 50 of 50, prepared by Nu-Wind Engineering, signed and sealed by Christian Langley, P.E.*
2. *Calculation titled "Air Conditioning Stands Calculations", dated March 07, 2012, sheets 1 through 30 of 30, prepared by Nu-Wind Engineering, signed and sealed by Christian Langley, P.E.*

D. QUALITY ASSURANCE

1. *By Miami-Dade County Department of Permitting, Environment, and regulatory Affairs (PERA).*

E. MATERIAL CERTIFICATIONS

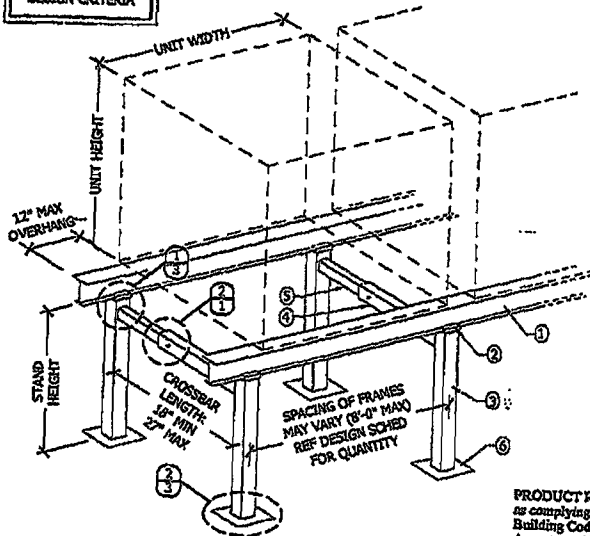
1. *None.*



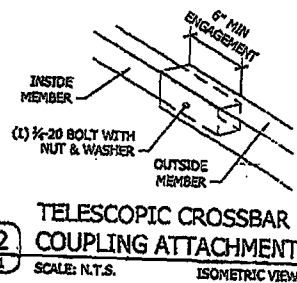
Helmy A. Makar, P. E., M.S.
 PERA, Product Control Unit Supervisor
 NOA No. 11-0824.01
 Expiration Date: 12/28/2016
 Approval Date: 04/12/2012

REFER TO DESIGN
SCHEDULE FOR
ALLOWABLE WIND
LOADS AND OTHER
DESIGN CRITERIA

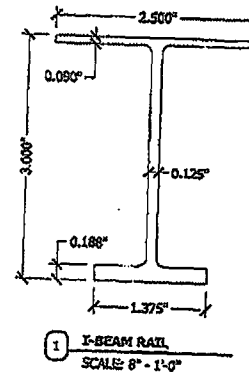
ALUMINUM ROOFTOP EQUIPMENT STAND WITH SQUARE POSTS & TELESCOPIC CROSSBAR



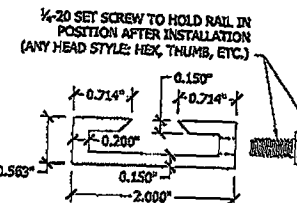
1 STAND ASSEMBLY
SCALE: N.T.S.
ISOMETRIC VIEW



2 TELESCOPIC CROSSBAR COUPLING ATTACHMENT
SCALE: N.T.S.
ISOMETRIC VIEW



1 I-BEAM RAIL
SCALE: 8" = 1'-0"



2 RAIL CONNECTOR
SCALE: 8" = 1'-0"

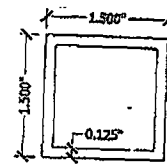
GENERAL NOTES

- THIS SYSTEM HAS BEEN EVALUATED IN ACCORDANCE WITH THE 2007 FLORIDA BUILDING CODE WITH 2009 SUPPLEMENTS, FOR USE WITHIN & OUTSIDE THE HIGH VELOCITY HURRICANE ZONE (HVHZ).
- THIS SYSTEM HAS BEEN EVALUATED IN ACCORDANCE WITH THE 2010 FLORIDA BUILDING CODE, FOR USE WITHIN & OUTSIDE THE HIGH VELOCITY HURRICANE ZONE (HVHZ).
- THE SYSTEM DEPICTED HEREIN HAS BEEN EVALUATED IN ACCORDANCE WITH THE 2010 FLORIDA BUILDING CODE, FOR USE WITHIN AND OUTSIDE THE HIGH VELOCITY HURRICANE ZONE (HVHZ).
- THIS SYSTEM HAS BEEN EVALUATED WITHOUT A ONE-THIRD INCREASE IN ALLOWABLE STRESS. WIND LOAD DURATION FACTOR $C_d=1.6$ HAS BEEN USED FOR WOOD ANCHOR DESIGN.
- SITE WIND PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A SITE-SPECIFIC BASIS IN ACCORDANCE WITH THE GOVERNING CODE.
- THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT SHALL PREPARE SITE-SPECIFIC DOCUMENTS AND APPLY FOR ONE-TIME MIAMI-DADE NOA FOR USE IN CONJUNCTION WITH THIS APPROVAL.
- PERMIT HOLDER SHALL VERIFY THE ADEQUACY

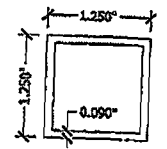
- OF THE EXISTING STRUCTURE TO WITHSTAND SUPERIMPOSED LOADS.
- ALL ALUMINUM EXTRUSIONS SHALL BE 6061-T6 ALLOY & TEMPER, UNLESS NOTED OTHERWISE. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS REQUIREMENTS, USING FILLER ALLOYS 4043, 4047, 5183, 5356, OR 5556.
- STANDS SHALL BE INSTALLED WITH MINIMUM CLEAR HEIGHT AS SPECIFIED IN THE ABOVE-NOTED BUILDING CODE. "STAND HEIGHT" AS USED HEREIN IS NOT NECESSARILY EQUIVALENT TO "STAND CLEAR HEIGHT" AS SPECIFIED IN THE BLDG CODE.
- VIBRATION ISOLATOR PADS SHALL BE PROVIDED BY CONTRACTOR BETWEEN UNITS & STAND.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THAT FASTENING AS SHOWN HEREIN WILL NOT VOID THE EQUIPMENT MANUFACTURER'S WARRANTY, ESPECIALLY WHERE UNITS ARE INSTALLED WITH OVERHANG PAST RAIL. (SEE TIE-DOWN DETAILS).
- ALL BOLTS & WASHERS SHALL BE ZINC COATED STEEL, GALVANIZED STEEL, OR STAINLESS STEEL WITH A MINIMUM TENSILE YIELD STRENGTH OF 60 KSI.
- PLASTIC COMPONENTS USED WITHIN THE HVHZ MUST MEET ALL APPLICABLE FIRE/SMOKE/UV PERFORMANCE REQUIREMENTS AS SET FORTH IN THE ABOVE-NOTED BUILDING CODE.
- ANY STEEL IN CONTACT WITH ALUMINUM SHALL BE PAINTED OR PLATED AS PRESCRIBED IN THE ABOVE-NOTED BUILDING CODE.

PRODUCT REVISED
as complying with the Florida
Building Code
Acceptance No. 11-0824-01
Expiration Date 12/28/2016
By *William A. Hester*
Miami Date Product Control

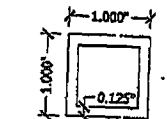
"SYSTEM 1" COMPONENTS



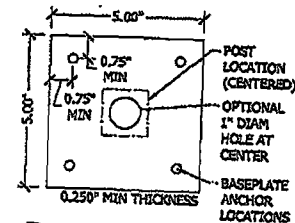
3a "SYSTEM 1" POST
SCALE: 8" = 1'-0"



4a "SYSTEM 1" CROSSBAR (OUTSIDE MEMBER)
SCALE: 8" = 1'-0"

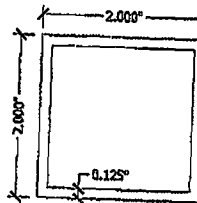


5a "SYSTEM 1" CROSSBAR (INSIDE MEMBER)
SCALE: 8" = 1'-0"

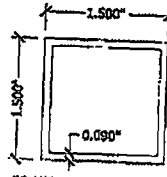


6a "SYSTEM 1" BASEPLATE
SCALE: 3" = 1'-0"

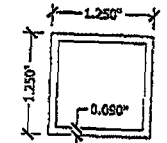
"SYSTEM 2" COMPONENTS



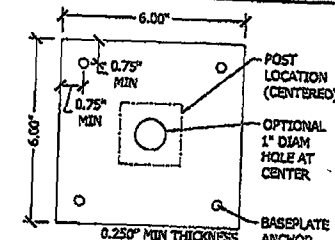
3b "SYSTEM 2" POST
SCALE: 8" = 1'-0"



4b "SYSTEM 2" CROSSBAR (OUTSIDE MEMBER)
SCALE: 8" = 1'-0"



5b "SYSTEM 2" CROSSBAR (INSIDE MEMBER)
SCALE: 8" = 1'-0"



6b "SYSTEM 2" BASEPLATE
SCALE: 3" = 1'-0"

FLU-MIND ENGINEERING
1200 N. FEDERAL HWY., #200
BOCA RATON, FL 33432
Tel: (561) 993-8865
Fax: (561) 993-8866
www.flu-mind.com

DATE	DESCRIPTION	BY
08/28/2014	INITIAL SUBMITTAL	FLU
09/02/2014	REVISION	FLU

ALUMINUM STANDS FOR
ROOFTOP EQUIPMENT
(SQUARE POSTS)
MIAMI-DADE NOA

F & L ALUMINUM PARTS, Inc.
1710 NW 22nd St., Unit 3
FORT LAUDERDALE, FL 33304

DRAWING NUMBER
ENL 11003

SHEET
1 of 3

DESIGN SCHEDULE

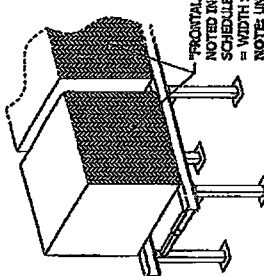
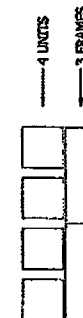
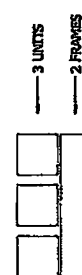
UNIT SIZE (FRONTAL AREA)	STAND HEIGHT	SYSTEM 1*				SYSTEM 2*			
		1 UNIT 2 FRAMES	2 UNITS 3 FRAMES	3 UNITS 4 FRAMES	4 UNITS 5 FRAMES	1 UNIT 2 FRAMES	2 UNITS 3 FRAMES	3 UNITS 4 FRAMES	4 UNITS 5 FRAMES
4.0 sqft	18"	17.0 PSF	27.0 PSF	36.0 PSF	45.0 PSF	20.0 PSF	30.0 PSF	40.0 PSF	50.0 PSF
	21"	15.7 PSF	25.7 PSF	34.7 PSF	43.7 PSF	18.7 PSF	28.7 PSF	37.7 PSF	46.7 PSF
	24"	14.3 PSF	24.3 PSF	33.3 PSF	42.3 PSF	17.3 PSF	27.3 PSF	36.3 PSF	45.3 PSF
6.25 sqft	18"	18.9 PSF	28.9 PSF	37.9 PSF	46.9 PSF	21.9 PSF	31.9 PSF	40.9 PSF	50.9 PSF
	21"	17.6 PSF	27.6 PSF	36.6 PSF	45.6 PSF	20.6 PSF	30.6 PSF	39.6 PSF	49.6 PSF
	24"	16.2 PSF	26.2 PSF	35.2 PSF	44.2 PSF	19.2 PSF	29.2 PSF	38.2 PSF	47.2 PSF
7.5 sqft	18"	20.7 PSF	30.7 PSF	39.7 PSF	48.7 PSF	23.7 PSF	33.7 PSF	42.7 PSF	51.7 PSF
	21"	19.4 PSF	29.4 PSF	38.4 PSF	47.4 PSF	22.4 PSF	32.4 PSF	41.4 PSF	50.4 PSF
	24"	18.0 PSF	28.0 PSF	37.0 PSF	46.0 PSF	21.0 PSF	31.0 PSF	40.0 PSF	49.0 PSF
9.0 sqft	18"	22.2 PSF	32.2 PSF	41.2 PSF	50.2 PSF	25.2 PSF	35.2 PSF	44.2 PSF	53.2 PSF
	21"	20.9 PSF	30.9 PSF	39.9 PSF	48.9 PSF	23.9 PSF	33.9 PSF	42.9 PSF	51.9 PSF
	24"	19.5 PSF	29.5 PSF	38.5 PSF	47.5 PSF	22.5 PSF	32.5 PSF	41.5 PSF	50.5 PSF
12.25 sqft	18"	24.4 PSF	34.4 PSF	43.4 PSF	52.4 PSF	27.4 PSF	37.4 PSF	46.4 PSF	55.4 PSF
	21"	23.1 PSF	33.1 PSF	42.1 PSF	51.1 PSF	26.1 PSF	36.1 PSF	45.1 PSF	54.1 PSF
	24"	21.7 PSF	31.7 PSF	40.7 PSF	49.7 PSF	24.7 PSF	34.7 PSF	43.7 PSF	52.7 PSF
16.0 sqft	18"	26.7 PSF	36.7 PSF	45.7 PSF	54.7 PSF	29.7 PSF	39.7 PSF	48.7 PSF	57.7 PSF
	21"	25.4 PSF	35.4 PSF	44.4 PSF	53.4 PSF	28.4 PSF	38.4 PSF	47.4 PSF	56.4 PSF
	24"	24.0 PSF	34.0 PSF	43.0 PSF	52.0 PSF	27.0 PSF	37.0 PSF	46.0 PSF	55.0 PSF

DESIGN SCHEDULE NOTES

- DESIGN SCHEDULE GIVES MAXIMUM ALLOWABLE WIND LOAD FOR EACH COMBINATION OF UNIT SIZE, STAND HEIGHT, AND UNIT/POST CONFIGURATION.
- UNIT SIZE (FRONTAL AREA) IS AREA OF UNIT FACE PARALLEL TO BEAM RAIL (= UNIT HEIGHT X UNIT WIDTH), AS DEPICTED HEREIN. UNIT HEIGHT SHALL NOT EXCEED UNIT WIDTH.
- FOR STANDS WITH VARYING UNIT SIZES, ENTER DESIGN SCHEDULE USING MAXIMUM SIZE OF ALL UNITS TO BE INSTALLED ON EACH STAND.
- "STAND HEIGHT" IS AS DEPICTED HEREIN.
- "UNIT & POST CONFIGURATIONS" INDICATE NUMBER OF UNITS & NUMBER OF FRAMES PER STAND, AS DEPICTED IN DIAGRAMS. "FRAME" HERE DENOTES ASSEMBLAGE OF 2 POSTS WITH A CROSSBAR.
- "N" UNITS & "N" FRAMES INDICATES ANY NUMBER OF UNITS WITH AN EQUAL NUMBER OF FRAMES PER STAND. "N" UNITS & "N" FRAMES INDICATES ANY NUMBER OF UNITS WITH A NUMBER OF FRAMES PER STAND EQUAL TO THE NUMBER OF UNITS MINUS ONE.
- EACH UNIT SHALL HAVE A MAXIMUM WEIGHT OF 300 LBS.
- REFERENCE ANCHOR SCHEDULE FOR ALLOWABLE ANCHORS AND INSTALLATION CRITERIA.

REACTION SCHEDULE

STAND HEIGHT	SYSTEM 1*			SYSTEM 2*		
	REACTION AT BASE PSF	REACTION AT BASE PSF	REACTION AT BASE PSF	REACTION AT BASE PSF	REACTION AT BASE PSF	REACTION AT BASE PSF
18"	27.0	36.0	45.0	27.0	36.0	45.0
21"	25.7	34.7	43.7	25.7	34.7	43.7
24"	24.3	33.3	42.3	24.3	33.3	42.3

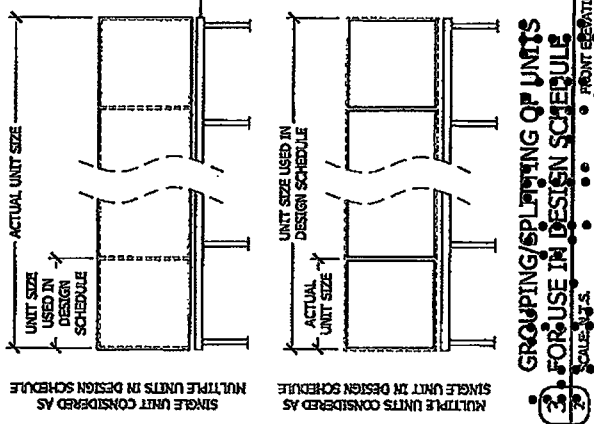


PRODUCT REVISIONS
as compliant with the Florida
Building Code
Acceptance No. 11-0824-01
Expiration Date 12/31/2016
By: [Signature]
Miami Design/Construct

1 UNIT & POST CONFIGURATIONS

SCALE: N.T.S.

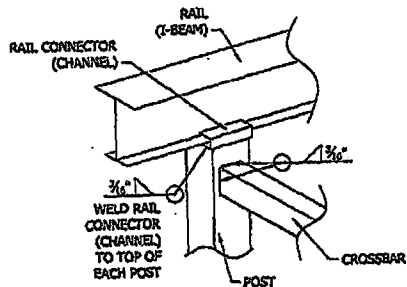
DIAGRAMS



GROUPING/SPLITTING OF UNITS FOR USE IN DESIGN SCHEDULE

SCALE: N.T.S.

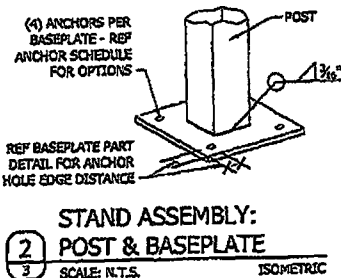
340



STAND ASSEMBLY:

POST, CROSSBAR, & RAIL CONN

1
3 SCALE: N.T.S. ISOMETRIC

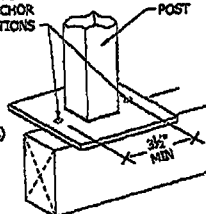


STAND ASSEMBLY:

POST & BASEPLATE

2
3 SCALE: N.T.S. ISOMETRIC

ALTERNATE: (2) ANCHORS PER BASEPLATE FOR INSTALLATION TO WOOD RAFTERS - REF ANCHOR SCHED FOR OPTIONS



ALT BASEPLATE ANCHORAGE SHALL BE LIMITED TO THE FOLLOWING:

- (2) UNITS MAX
- (3) FRAMES MIN
- 6.25 SQ FT MAX UNIT SIZE (FRONTAL AREA)
- 24" MAX STAND HT
- MAX ALLOWABLE DESIGN PRESSURES:
 - SYSTEM 1: 55.3 PSF
 - SYSTEM 2: 71.4 PSF

ALTERNATE

BASEPLATE ANCHORAGE

3
3 SCALE: N.T.S. ISOMETRIC

ANCHOR SCHEDULE:

TO CONCRETE (MIN 2,000 PSI)

- 3/4" POWERS WEDGE BOLT
3" MIN EMBED
4/2" MIN EDGE DISTANCE
- 3/4" HILTI KWIK BOLT III
3/4" MIN EMBED
5" MIN EDGE DISTANCE

TO WOOD HOST STRUCTURE

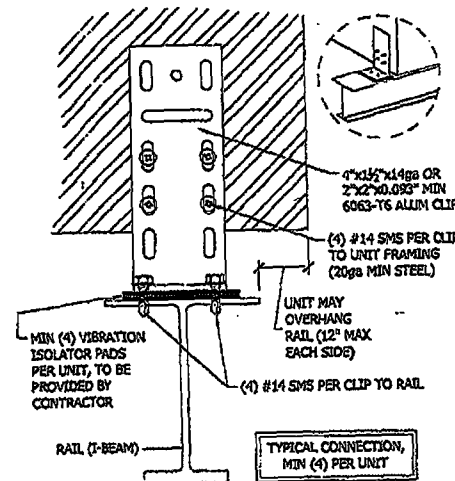
- 3/4" LAG SCREW
3/4" MIN THREAD PENETRATION

TO STEEL (MIN 3/16" THICK)

- 1/2" TENS SCREWS OR 3/4" 20 SELF-THREADING METAL SCREWS (SAE GRADE 5)

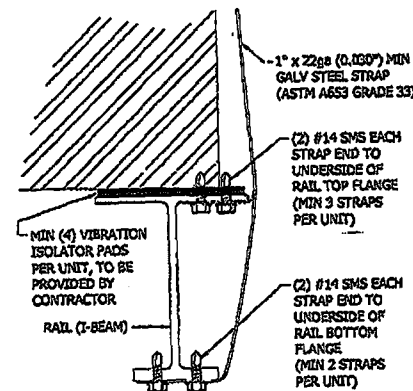
ANCHOR NOTES:

- ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS, & LOCATED PER BASEPLATE COMPONENT DETAIL(S).
- ENSURE MINIMUM EMBEDMENT, EDGE DISTANCE, & SPACING FOR ALL ANCHORS ARE IN ACCORDANCE WITH ANCHOR SCHEDULE.
- MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDES SHEATHING, UNDERLAYMENT, INSULATION, AND OTHER ROOFING MATERIALS.
- MINIMUM 3/4" EDGE DISTANCE IS CONSIDERED IN DESIGN FOR ALL ANCHORS TO WOOD HOST STRUCTURE (i.e. ANCHOR SHALL BE LOCATED IN CENTER OF TRUSS/RAFTER WHERE FASTENED TO NARROW FACE OF NOMINAL 2x LUMBER).
- WOOD HOST STRUCTURE SHALL BE "SOUTHERN PINE" WITH G=0.55 OR GREATER SPECIFIC GRAVITY (4 DENSITY).
- WHERE HOST STRUCTURE IS WOOD FRAMING, EXISTING CONDITIONS MAY VARY. FIELD VERIFY THAT FASTENERS ARE INTO ADEQUATE WOOD FRAMING MEMBERS, NOT INTO PLYWOOD (U.N.O.).
- SELF-TAPPING OR SELF-THREADING METAL SCREWS SHALL BE INSTALLED WITH FULL THREAD ENGAGEMENT INTO METAL HOST STRUCTURE AND MAY HAVE A FLAT HEAD, PAN HEAD, TRUSS HEAD, OR OTHER HEAD STYLES.
- ANCHORS THAT INCORPORATE MACHINE SCREWS SHALL HAVE MINIMUM OF 1/2" ENGAGEMENT OF THREADS IN BASE ANCHOR AND MAY HAVE ANY HEAD STYLE, UNLESS INDICATED OTHERWISE BY MFR.



CLIPPED UNIT CONNECTION

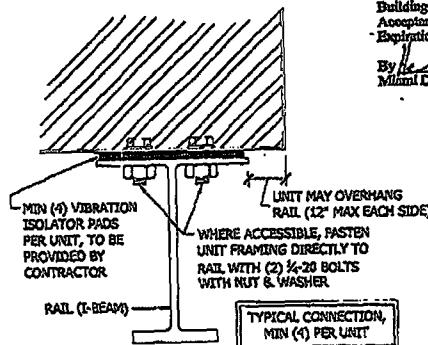
4
3 SCALE: 1:2 VERT SECTION



STRAPPED UNIT CONNECTION

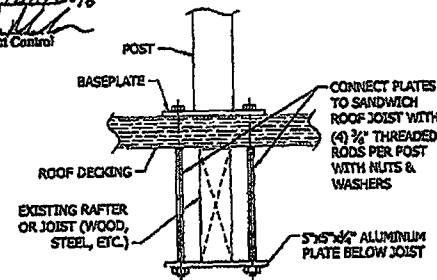
5
3 SCALE: 1:2 VERT SECTION

PRODUCT REVISED
to comply with the Florida
Building Code
Acceptance No. 11-0924.01
Expiration Date 12/31/2016
By: [Signature]
Miami Design Control



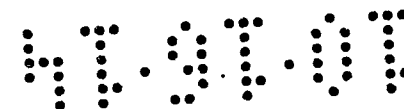
DIRECT UNIT CONNECTION

6
3 SCALE: 6" = 1'-0" VERT SECTION



RAFTER/JOIST BASEPLATE ANCHORAGE

7
3 SCALE: 2" = 1'-0" VERT SECTION



ALU-WIND
ENGINEERING
1001 N. FEDERAL HWY. #200
BOCA RATON, FL 33433
TEL: (561) 719-3779
FAX: (561) 719-3779

DESIGN APPROVALS
DATE: 11/10/16
BY: [Signature]

DATE	DESCRIPTION	INITIALS	SUBMITTAL
11/10/16	11-0924.01	[Signature]	[Signature]

ALUMINUM STANDS FOR
ROOFER EQUIPMENT
(SQUARE POSTS)
HUNG-ON-NOA

F & L ALUMINUM
PARTS, INC.
1710 NW 22nd CT, UNIT 3
POMPAHO BEACH, FL 33064

DRAWING NUMBER:
FNL11003
SHEET
3

ABBREVIATIONS

&

AND

JAN

JANITOR

∠

ANGLE

JT

JOINT

⊙

AT

K

THOUSAND (1,000)

⊙

CENTER LINE

KIT

KITCHEN

⊙

DIAMETER OR ROUND

LAM

LAMINATED

⊙

PERPENDICULAR

LAV

LAVATORY

⊙

POUND OR NUMBER

LP

LOW POINT

⊙

EXISTING

LVR

LOUVER

ABV

ABOVE

MACH

MACHINE

ACOUS

ACOUSTICAL

MATL

MATERIAL

AD

AIR DRAIN

MAX

MAXIMUM

ADJ

ADJACENT/ADJUSTABLE

MDF

MEDIUM DENSITY FIBERBOARD

ADU

ABOVE FINISHED FLOOR

MECH

MECHANICAL

A.H.U.

AIR HANDLER UNIT

MEMB

MEMBRANE

ALT

ALTERNATE

MEZZ

MEZZANINE

ALUM

ALUMINUM

MFR

MANUFACTURE

ANOD

ANODIZED

MH

MAN HOLE

APPROX

APPROXIMATE (LY)

MIN

MINIMUM

BD

BOARD

MIR

MIRROR

BITUM

BITUMINOUS

MISC

MISCELLANEOUS

BLDG

BUILDING

MOS

MASONRY OPENING

BLK

BLOCK

MR

MOISTURE RESISTANT

BLKG

BLOCKING

MNT

MOUNTED

BM

BEAM

MTL

METAL

BOT

BOTTOM

MUL

MULLION

BRZ

BRONZE

N

NORTH

C

CABINET

NIC

NOT IN CONTRACT

CAB

CATCH BASIN

NO OR #

NUMBER

CER

CERAMIC

NOM

NOMINAL

CJ

CONTROL JOINT

NTS

NOT TO SCALE

CL

CLOSET

OA

OVERALL

CLR

CLEAR

ON CENTER

ON CENTER

CLR OPG

CLEAR OPENING

OD

OUTSIDE DIAMETER

CNTR

COUNTER

OFF

OFFICE

CO

COLUMNA

OPNG

OPENING

COL

CONCRETE

OFRO

OVER FLOW ROOF DRAIN

CONC

CONCRETE

PARTN

PARTITION

CONSTR.

CONSTRUCTION

PE

PEDESTAL

CONT

CONTINUOUS

PH

PENTHOUSE

CONTR

CONTRACTOR

P.L

PROPERTY LINE

COORD

COORDINATE

PLAM

PLASTIC LAMINATE

CPT

CARPET

PLAS

PLASTER

CT

CERAMIC TILE

PLBG

PLYWOOD

CTR

CENTER

PLND

PANEL

D

DRYER

PNT

PAINT

DBL

DOUBLE

PR

PREFABRICATED

DEPT

DEPARTMENT

PREFAB

PREFABRICATED

DET

DETAIL

PS

PULL STATION

DF

DRINKING FOUNTAIN

PSF

POUNDS PER SQUARE FOOT

DD

DECK DRAIN

PSI

POUNDS PER SQUARE INCH

DIAM

DIAMETER

PT

PRESSURE TREATED

DIM

DIMENSION

PRCST

PREFABRICATED

DISP

DISPENSER

PREFAB

PREFABRICATED

DN

DOWN

PVMT

POLYVINYL CHLORIDE

DR

DOWN

PVC

POLYVINYL CHLORIDE

DS

DOWNPOUT

QT

QUARRY TILE

DTL

DETAIL

QTY

QUALITY

DW

DISHWASHER

R

RISER OR RADIUS

DWG

DRAWING

R&S

ROOF AND SHELF

E

EAST

RA

RADIUS

EA

EACH

REF

REFRIGERATOR

EJ

EXPANSION JOINT

REFN

REINFORCED

ELEV

ELEVATION

REQ'D

REQUIRED

ELECT

ELECTRICAL

REV

REVISION

EMER

EMERGENCY

RM

ROOM

ENCLOS

ENCLOSURE

RO

ROUGH OPENING

EQ

EQUAL

RTU

ROOF TOP UNIT

EQUIP

EQUIPMENT

RWL

RAIN WATER LEADER

EW

ELECTRIC WATER COOLER

S

SOLID CORE

EXH

EXHAUST

SC

SCHEDULE

EXIST

EXISTING

SEC

SECTION

EXP

EXPANSION

SH

SHOWER

EXT

EXTERIOR

SHR

SHOWER

FA

FIRE ALARM

SH

SHOWER

FD

FLOOR DRAIN

SH

SHOWER

FN

FOUNDATION

SH

SHOWER

FE

FIRE EXTINGUISHER

SH

SHOWER

FEC

FIRE EXTINGUISHER CABINET

SH

SHOWER

FF

FINISH FLOOR

SH

SHOWER

FF/FIN.FLR

FINISH FLOOR

SH

SHOWER

FHC

FIRE HOSE CABINET

SH

SHOWER

FHR

FIRE HOSE RACK

SH

SHOWER

FKT

FIXTURE

SH

SHOWER

FLR

FLOOR

SH

SHOWER

FLUOR

FLUORESCENT

SH

SHOWER

FND

FOUNDATION

SH

SHOWER

FP

FIRE PROOF

SH

SHOWER

FRT

FIRE RETARDANT TREATED

SH

SHOWER

FS

FULL SIZE

SH

SHOWER

FT

FOOT

SH

SHOWER

FTG

FOOTING

SH

SHOWER

FURN

FURNITURE

SH

SHOWER

FURR

FURRING

SH

SHOWER

GA

GAUGE

SH

SHOWER

GALV

GALVANIZED

SH

SHOWER

GAR

GARAGE

SH

SHOWER

GC

GENERAL CONTRACTOR

SH

SHOWER

GL

GLASS

SH

SHOWER

GL BLK

GLASS BLOCK

SH

SHOWER

GND

GROUND

SH

SHOWER

GRND

GROUND

SH

SHOWER

GYP

GYP SUM BOARD

SH

SHOWER

HB

HOSE BIBB

SH

SHOWER

HC

HANDICAPPED

SH

SHOWER

HDWR

HARDWARE

SH

SHOWER

HM

HOLLOW METAL

SH

SHOWER

HP

HIGH POINT

SH

SHOWER

HR

HOUR (S)

SH

SHOWER

HT

HEIGHT

SH

SHOWER

HVAC

HEATING/VENTILATING

SH

SHOWER

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INSIDE DIAMETER

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INFORMATION

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INSULATION

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INTERIOR

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SHOWER

INV

INVERT

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SHOWER

SYMBOLS

—

BREAK LINE

⊙

COLUMN TAG

⊙

COLUMN CENTERLINE

⊙

CEILING HEIGHT TAG

⊙

DATUM POINT TAG

⊙

ELEVATION HEIGHT FROM / TO

⊙

DETAIL TAG

⊙

DETAIL NUMBER

⊙

DETAIL BLOW-UP TAG

⊙

DETAIL NUMBER

⊙

AREA DETAIL BOUNDARY

⊙

DRAWING TITLE TAG

⊙

SCALE SIZE

⊙

DETAIL NUMBER

⊙

DRAWING TITLE LABEL

⊙

DOOR TYPE TAG

⊙

DOOR NUMBER

⊙

EXTERIOR ELEVATION TAG

⊙

ELEVATION NUMBER

⊙

SHEET NUMBER

⊙

INTERIOR ELEVATION TAG

⊙

ELEVATION NUMBER

⊙

SHEET NUMBER

⊙

EXTERIOR ELEVATION

⊙

FINISH TAG

⊙

FINISH MATERIAL

⊙

WALL RECESSED

⊙

WALL MOUNTED

⊙

FIRE EXTINGUISHER

⊙

FIXTURE NOTE

⊙

MATCH LINE

⊙

MATCH LINE

⊙

SHEET MATCH LINE

⊙

CORRESPONDING SHT NUMBER

⊙

NORTH ARROW

⊙

OFFICE AREA

⊙

ROOM TAG

⊙

ROOM NAME

⊙

ROOM NUMBER

⊙

SECTION DETAIL TAG

⊙

DETAIL NUMBER

⊙

SHEET NUMBER

⊙

SLOPE

⊙

SLOPE RATIO SYMBOL

⊙

SLOPE LABEL

⊙

SLOPE RATIO

⊙

STEP NOTICE SYMBOL

⊙

HEIGHT OF STEP

⊙

WALL TYPE SYMBOL

⊙

WINDOW/SLIDING DOOR

⊙

TYPE SYMBOL

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COVER

COVER SHEET, INDEX OF DWG'S, LEGAL DESCRIPTION, GENERAL NOTES, AND LOCATION MAP

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FOR REFERENCE ONLY

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SITE PLAN

A1.02

LIFE SAFETY PLAN

A1.03

PHASING PLAN

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A2.02

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ENLARGED FIRE ALARM PLANS

E 2.05

ELECTRICAL PANEL SCHEDULE

E 2.06

PARTIAL ELECTRICAL RISER

GENERAL NOTES / SCOPE OF WORK.

1.

ALL WORK SHALL BE IN ACCORDANCE TO THE FOLLOWING CODES:
(F.B.C.E) FLORIDA BUILDING CODE EXISTING BUILDING-2010 EDITION
(F.B.C) FLORIDA BUILDING CODE-2010 EDITION
(F.B.C.A) FLORIDA ACCESSIBILITY BUILDING CODE - 2010 EDITION
(F.F.P.C) FLORIDA FIRE PREVENTION CODE - 2010 LATEST EDITION
(NFPA 101) LIFE SAFETY CODE HANDBOOK -2009 EDITION

2.

BUILDING HAS AN EXISTING FIRE ALARM SYSTEM. NO SPRINKLER SYSTEM.

3.

DESIGN SHALL COMPLY WITH THE FLORIDA FIRE PREVENTION CODE 2010 ED.

4.

CLASSIFICATION OF REHABILITATION WORK PER F.F.P.C SEC 43.1.1 SHALL BE CLASSIFIED AS THE FOLLOWING CATEGORY:
-MODIFICATION

5.

BUILDING COMPLIES WITH "WORK AREA COMPLIANCE METHOD" PER F.B.C.E. 101.5.2

6.

WORK PERFORMED IS AN ALTERATION LEVEL 2 PER F.B.C.E.-2010 SEC.404.1

7.

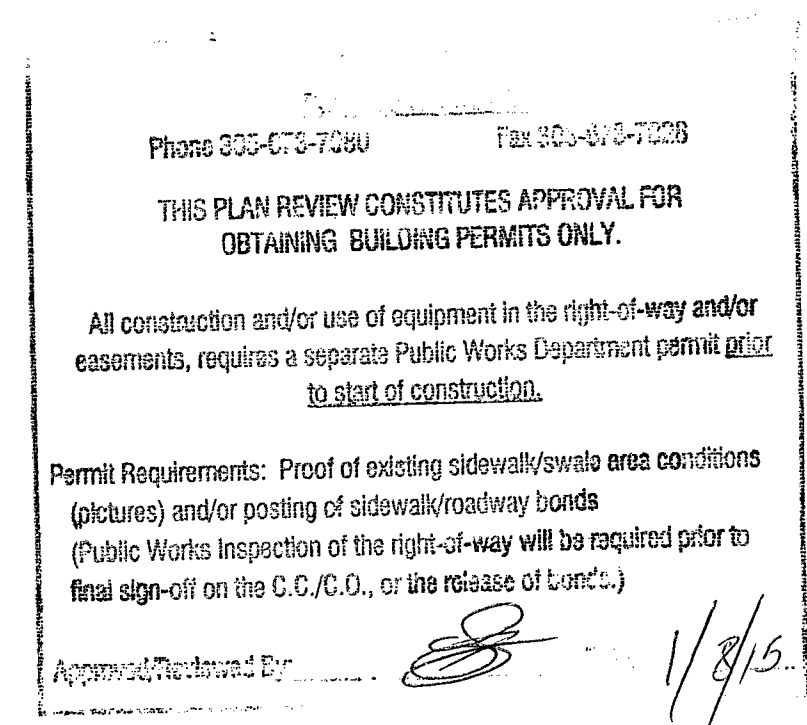
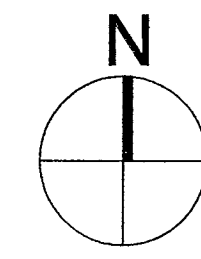
THIS WORK IS INTERIOR IMPROVEMENTS & GENERAL SCOPE IS DESCRIBED AS FOLLOW:
- PROVIDE MOBILITY FEATURES, ADA BATHROOMS & HEARING IMPAIRED/COMMUNICATION FEATURES TO UNITS 302 & 402 AS REQUIRED PER 806.2 & 806.3 FAC.
- CONVERT UNITS 301 & 401 TO COMMUNAL GANG BATHROOMS, REFER TO SHEET A2.01
- CONVERT EXISTING UNIT 300 INTO COMMUNAL SPACE WITH NEW KITCHEN
- PROVIDE HEARING IMPAIRED/COMMUNICATION FEATURES COMPLYING WITH 806.3 FAC TO UNITS 303 & 403



LEGAL DESCRIPTION

LOT 1, BLOCK 57, OF FISHER'S FIRST SUBDIVISION OF ALTON BEACH, ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 2, AT PAGE 77, OF THE PUBLIC RECORDS OF MIAMI-DADE COUNTY, FLORIDA.

LOCATION MAP



OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:
BUILDING: [Signature]
ZONING: [Signature]
PLUMBING: [Signature]
ELECTRICAL: [Signature]
MECHANICAL: [Signature]
FIRE PREVENTION: [Signature]
FLOOD: [Signature]
PUBLIC WORKS: [Signature]
STRUCTURAL: [Signature]
ELEVATOR: [Signature]

NOTICE: In addition to the requirements of this permit, there may be additional restrictions upon the use of the property, that may be found in the Public Records of the City of Miami Beach, and there may be additional permits required from other governmental agencies, such as water management, health, and other agencies, or other agencies.

HOTEL EVA INTERIOR IMPROVEMENTS

1506 COLLINS AVENUE
MIAMI BEACH, FLORIDA 33139

ISSUED FOR PERMIT

MAY 23, 2014



ARCHITECT:
Beame Architectural Partnership
3059 Grand Avenue, Suite 440
Coconut Grove, Florida 33133
PH: 305.444.7100 FX: 305.444.9803
Lawrence Beame, R.A.

MEP ENGINEER:
JMM Consulting Engineers, LLC
10251 SW Sunset DR, Suite 103
Miami, Florida 33173
PH: 305.255.1621 FX: 305.255.1732
Jose M Martinez, P.E.

STRUCTURE ENGINEER:
UNITED Engineering, Inc.
12595 SW 137 Ave, Suite 112
Miami, Florida 33186
PH: 786.347.5250
Juan J. Fuentes, PE, SE, LEED AP

OWNER:
J3 Ventures LLC
1506 Collins Avenue
Miami Beach, Florida 33139
PH-Office: 206.601.2532
John Burnett / Jim Kennett

INDEX OF DRAWINGS

BREAK LINE

A

A

COLUMN TAG
COLUMN CENTERLINE

00'-00"
A.F.F.

CEILING HEIGHT TAG

ELEV. = +0'-0"
TOP OF ...

DATUM POINT TAG,
ELEVATION HEIGHT
FROM / TO

1
A1.01

DETAIL TAG,
DETAIL NUMBER
SHEET NUMBER

1
A1.01

DETAIL BLOW-UP TAG,
DETAIL NUMBER
SHEET NUMBER

AREA DETAIL BOUNDARY

DRAWING

DRAWING TITLE TAG,
SCALE SIZE
DETAIL NUMBER

SCALE: 1/8"=1'-0"

DRAWING TITLE LABEL

00

DOOR TYPE TAG,
DOOR NUMBER

1
A1.01

EXTERIOR ELEVATION TAG
ELEVATION NUMBER
SHEET NUMBER

1
A1.01

INTERIOR ELEVATION TAG,
ELEVATION NUMBER
SHEET NUMBER

1
A1.01

EXTERIOR ELEVATION

00-00

FINISH TAG,
FINISH NUMBER
FINISH MATERIAL

FF

FE

WALL RECESSED
FIRE
EXTINGUISHER

WALL MOUNTED
FIRE
EXTINGUISHER

ADD

FIXTURE NOTE,

A0.00

MATCH LINE

MATCH LINE
SHEET MATCH LINE

A0.00

CORRESPONDING SHET NUMBER

N

NORTH ARROW,

OFFICE AREA
000-A

ROOM TAG
ROOM NAME
ROOM NUMBER

1
A1.01

SECTION DETAIL TAG,
DETAIL NUMBER
SHEET NUMBER

SLOPE
1/8" PER FT.

SLOPE RATIO SYMBOL,
SLOPE LABEL
SLOPE RATIO

6"

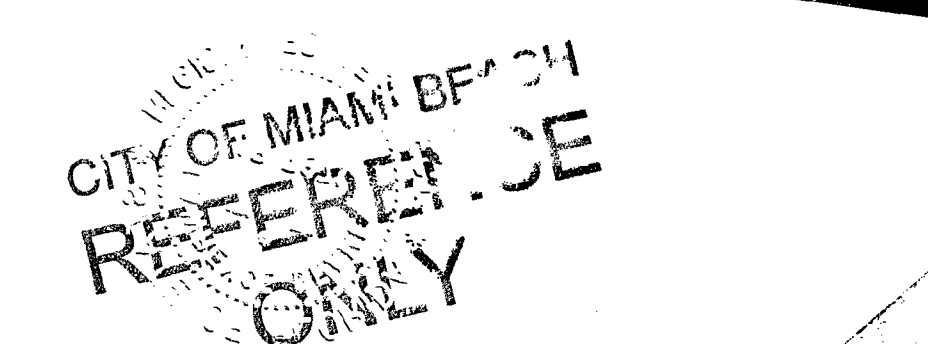
STEP NOTICE SYMBOL,
HEIGHT OF STEP

WALL TYPE SYMBOL

W/SL
A0.00

WINDOW/SLIDING DOOR
TYPE SYMBOL

John Burne

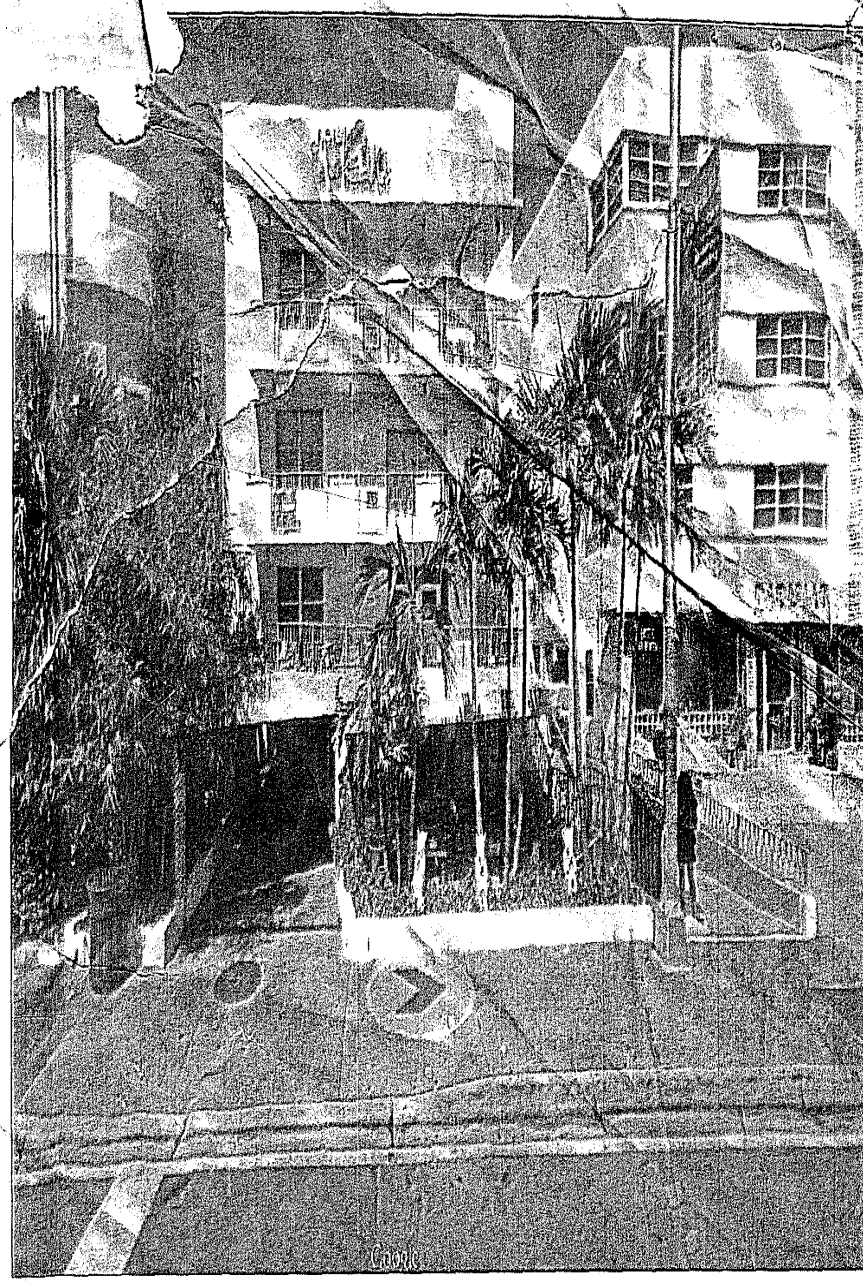


STRUCTURE ENGINEER:
UNITED Engineering, Inc.
 12595 SW 137 Ave, Suite 112
 Miami, Florida 33186
 PH: 786.347.5250
 an J. Fuentes, PE, SE, LEED AP

OWI
J3 Vent
1506 Coll
Miami Beach
PH-Office:
John Burne

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BEAME
ARCHITECTURAL
PARTNERSHIP



1506 COLLINS AVENUE
MIAMI BEACH, FLORIDA 33139

MAY 23, 2014

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Miami, Florida 33173
PH: 305.255.1621 FX: 305.255.173
Jose M Martinez, P.E.

J3 Ventures LLC
1506 Collins Avenue
Miami Beach, Florida 33139
PH-Office: 206.601.2532
John Burnett / Jim Kennett

WALDO NEW CUSTOMER DIVISION
Reviewed by: _____
Initial: _____
VERIFICATION REQUIRED: ☒ Not Required
ORAB LETTER 69-95 Required ☒ Not Required
OTHER: _____

97707

CITY OF MIAMI BEACH
REFERENCE
ONLY

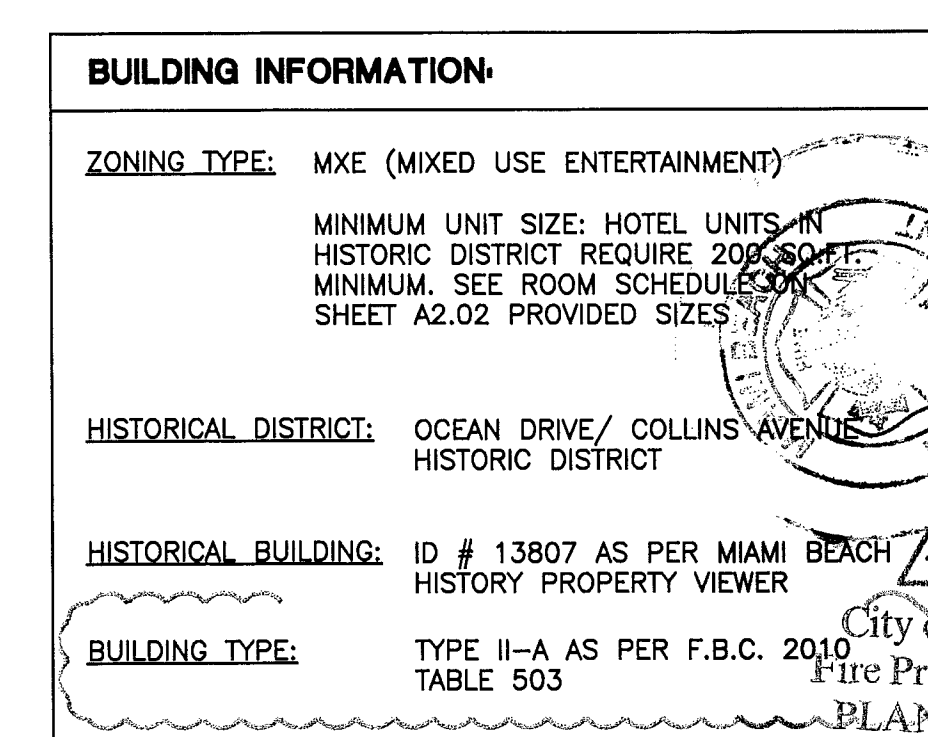
JOB COPY

CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:

BUILDING: _____
ZONING: _____
PLUMBING: PPF 4/29/14
ELECTRICAL: Ru 4/14/14
MECHANICAL: Long 4/14/14
FIRE PREVENTION: _____
FLOOD: _____
PUBLIC WORKS: _____
STRUCTURAL: _____
ELEVATOR: _____

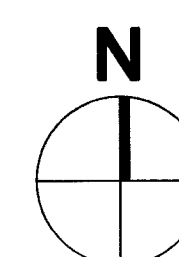
PRJ. MNGR. C.D.	DRAWING BY VS	SCALE AS SHOWN
SHEET TITLE		

JOB NUMBER 14010.00	SHEET NUMBER A1.01
DATE 05-23-14	

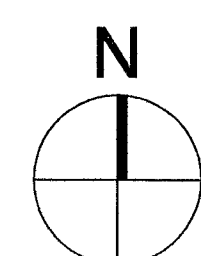
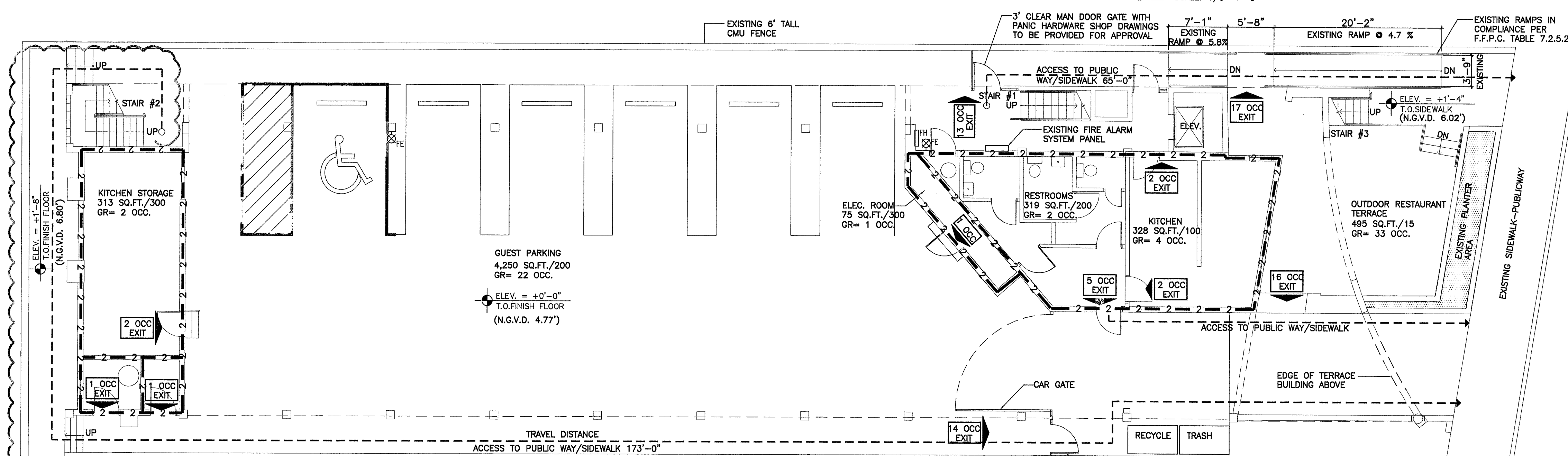
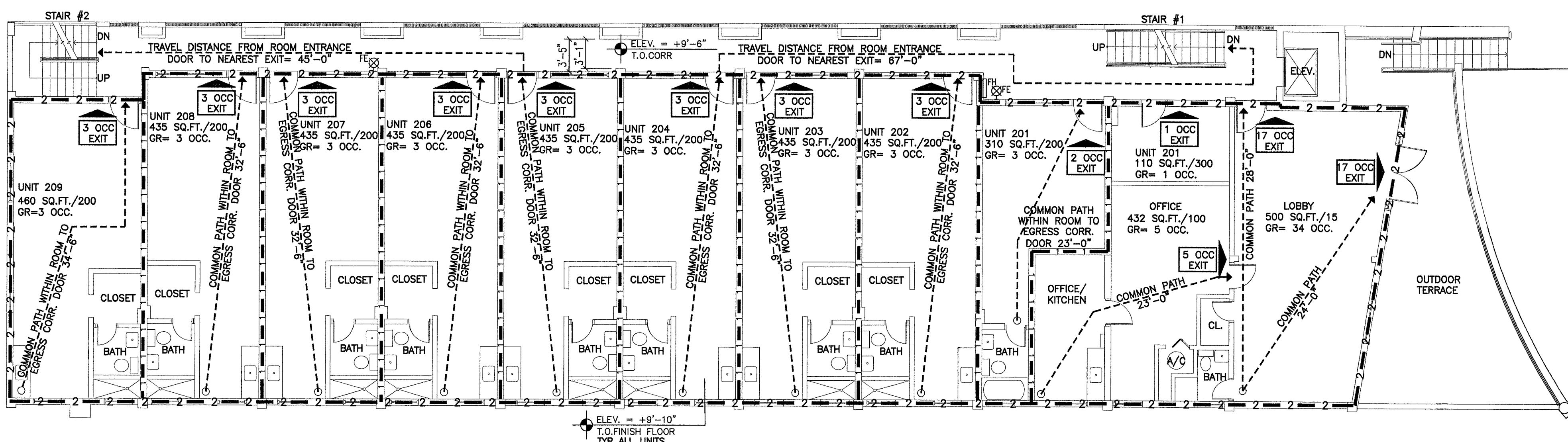
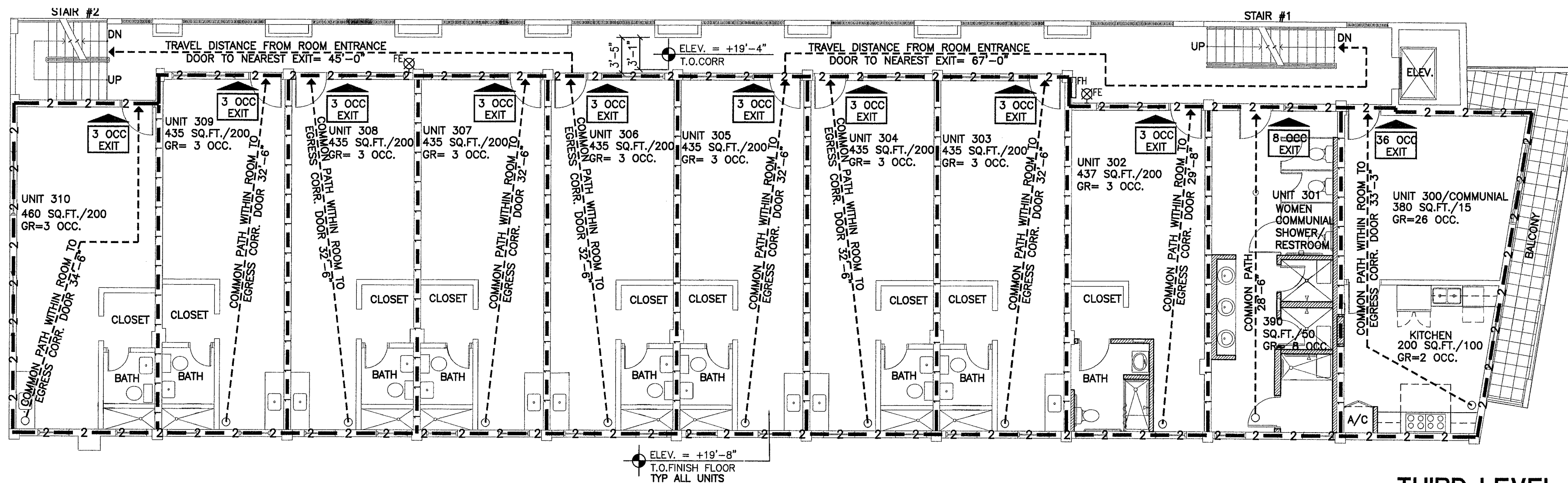
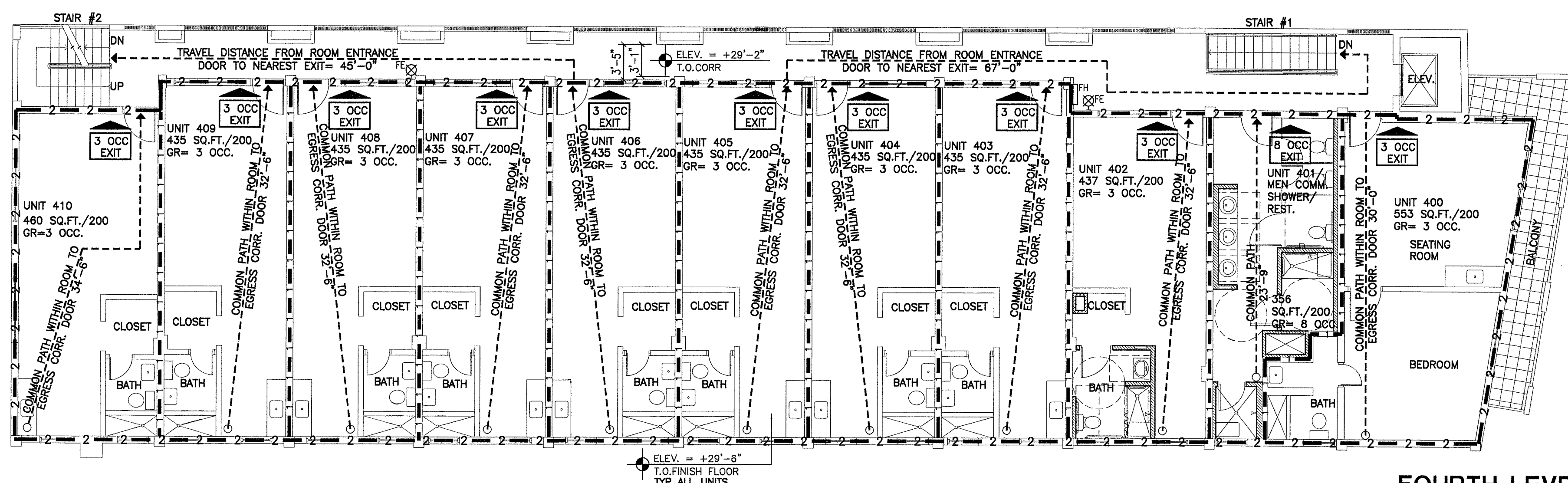


REVIEWED
CITY OF MIAMI BEACH
FIRE DEPARTMENT

01 SITE PLAN
SCALE: 1/16"=1'-0"



JOB NUMBER 14010.00	SHEET NUMBER A1.02
DATE 05-23-14	



BEAME
ARCHITECTURAL
PARTNERSHIP

3059 GRAND AVENUE, SUITE 440
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E-mail : bap@bapdesign.com
Florida Corp AA0002364
PH 305.444.7100 FX 305.444.9803
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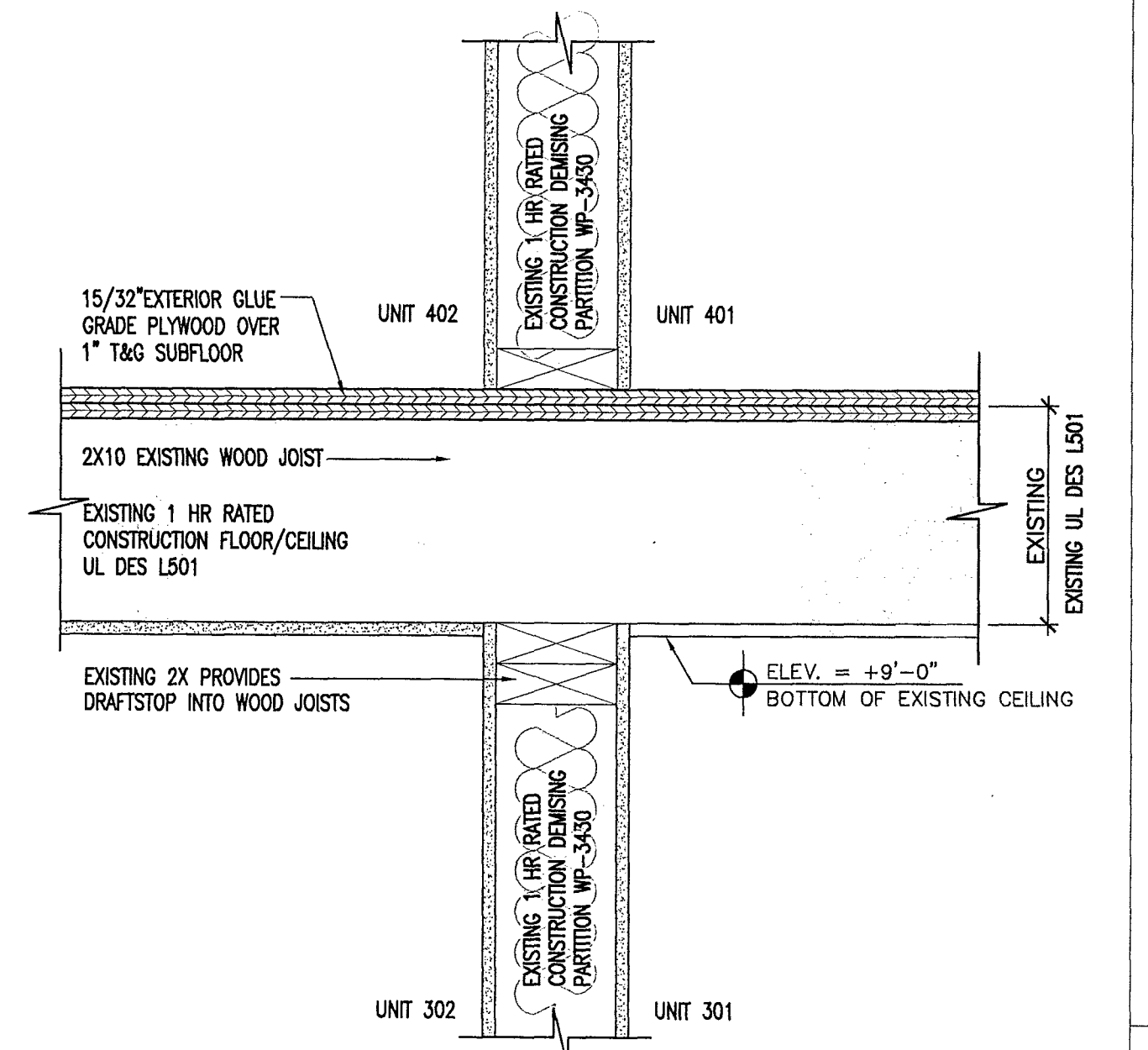
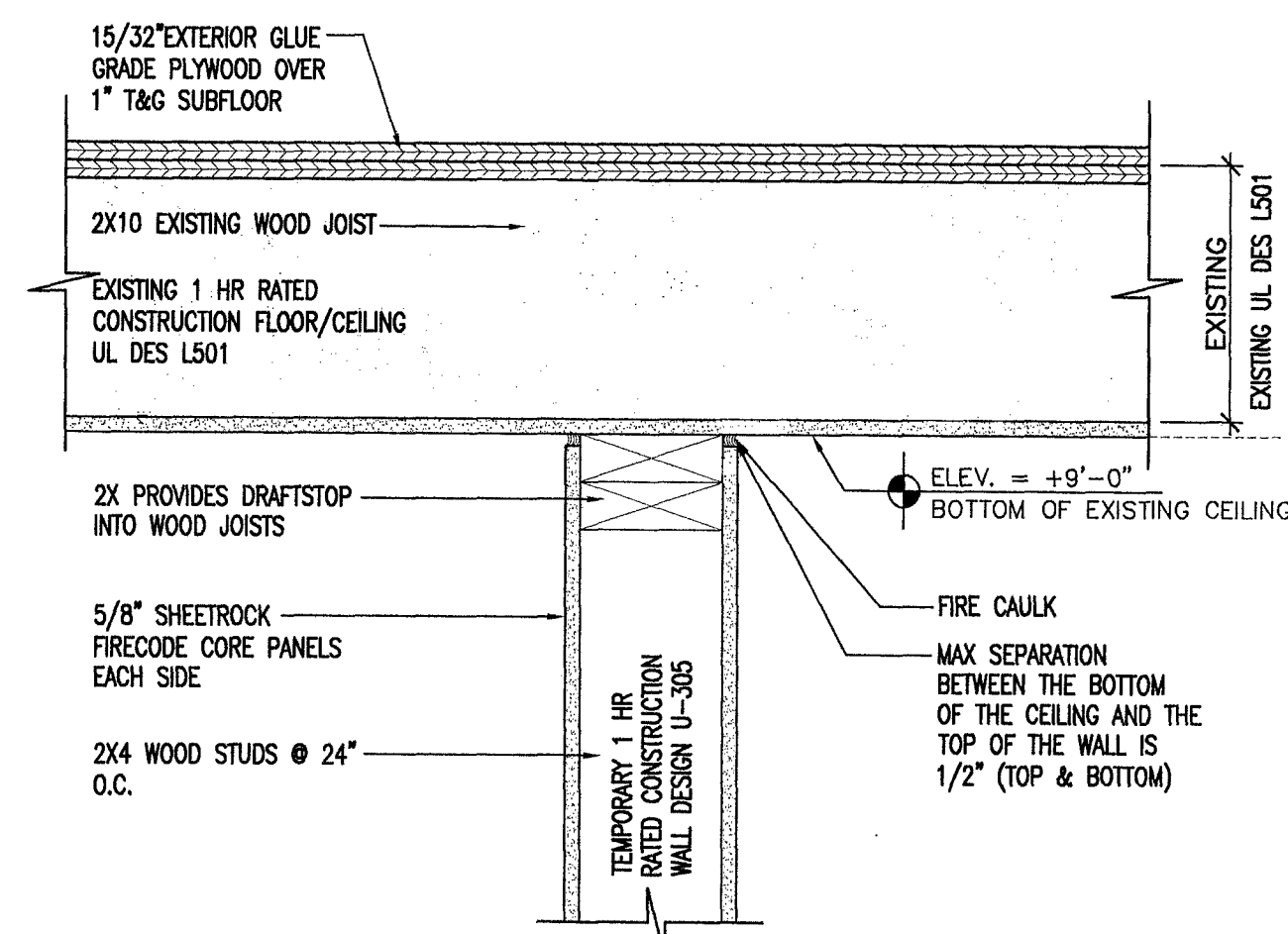
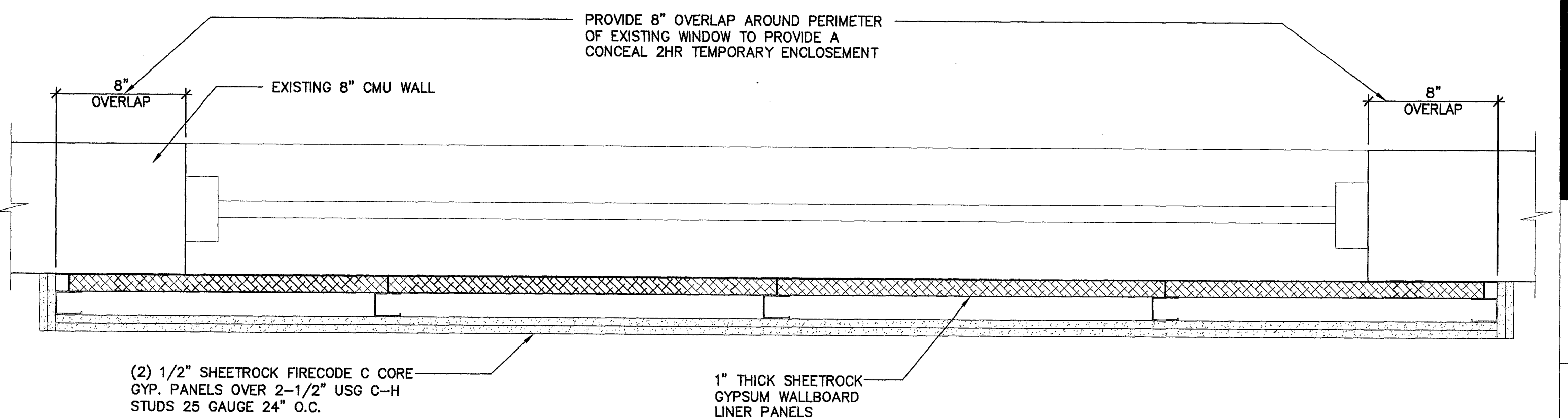
OWNER:
J3 VENTURES LLC
1506 COLLINS AVENUE
MIAMI BEACH, FLORIDA 33139

LAWRENCE BEAME, R.A.
REGISTRATION # 7871

HOTEL EVA
INTERIOR
IMPROVEMENTS
1506 COLLINS AVENUE
MIAMI BEACH,
FLORIDA 33139

PHASING
PLAN

JOB NUMBER
14010.00
DATE
05-23-14
SHEET NUMBER
14010.00
A1.03



- PHASE PLAN SUBMISSION:
- LEVEL OF ALTERATION PER F.F.P.C. 43.1.1. CLASSIFICATION OF REHABILITATION WORK CATEGORY FOR HOTEL EVA SHOULD BE THE FOLLOWING:
-(3) MODIFICATION
 - EXISTING OCCUPANCY LICENSE USE PER MIAMI BEACH RECORDS:
OCCUPANCY CLASSIFICATION : R1-HOTEL USE
 - PER F.F.P.C. SECTION 7.5.3.3 THE EXTERIOR EXIT ACCESS IS SERVED BY AT LEAST TWO REMOTE STAIRS THAT CAN BE ACCESSED WITHOUT ANY OCCUPANT TRAVELING PAST AN UNPROTECTED OPENING.
-WINDOWS IN EXISTING LOCATION WILL EITHER BE REMOVED OR HAVE A TEMPORARY 2-HR RATING CONCEALMENT, SEE PLAN.
-EXISTING DOORS WILL BE REPLACED WITH 90 MIN RATED DOORS, SEE PLAN.
 - NEW FIRE ALARM SYSTEM TO BE PROVIDED ALONG WITH 24-HR MONITORING STAFF ON SITE IN DURATION OF DEMOLITION & NEW WORK TO ENSURE NO DEBRIS IS LEFT IN PATH OF EGRESS FOR GUEST VISITING THE HOTEL. GC SUPERINTENDENT TO REMAIN ON SITE 24-HRS
 - GC TO PROVIDE BARRIERS & RATINGS OF DOORS AS REQUIRED PRIOR TO COMMENCING ANY DEMOLITION OR NEW WORK.

- GENERAL NOTES/LEGEND:
- UNITS TO REMAIN UNOCCUPIED THROUGH DURATION OF DEMOLITION & NEW CONSTRUCTION
 - EXISTING DOORS TO BE REPLACE WITH 90 MIN. OR 20 MIN RATED DOORS, SEE PLAN.
 - EXISTING WINDOW IN UNIT 301 TO BE REMOVED & FILLED WITH 8" CMU
 - EXISTING 1-HR RATED DEMISING WALLS TYPE WP 3430
 - WALLS DENOTED WITH 2 HR RATING ARE EXISTING TYPE #U905
 - PATH OF EGRESS
 - EXISTING FIRE EXTINGUISHERS
 - EXISTING FIRE HOSE CABINETS

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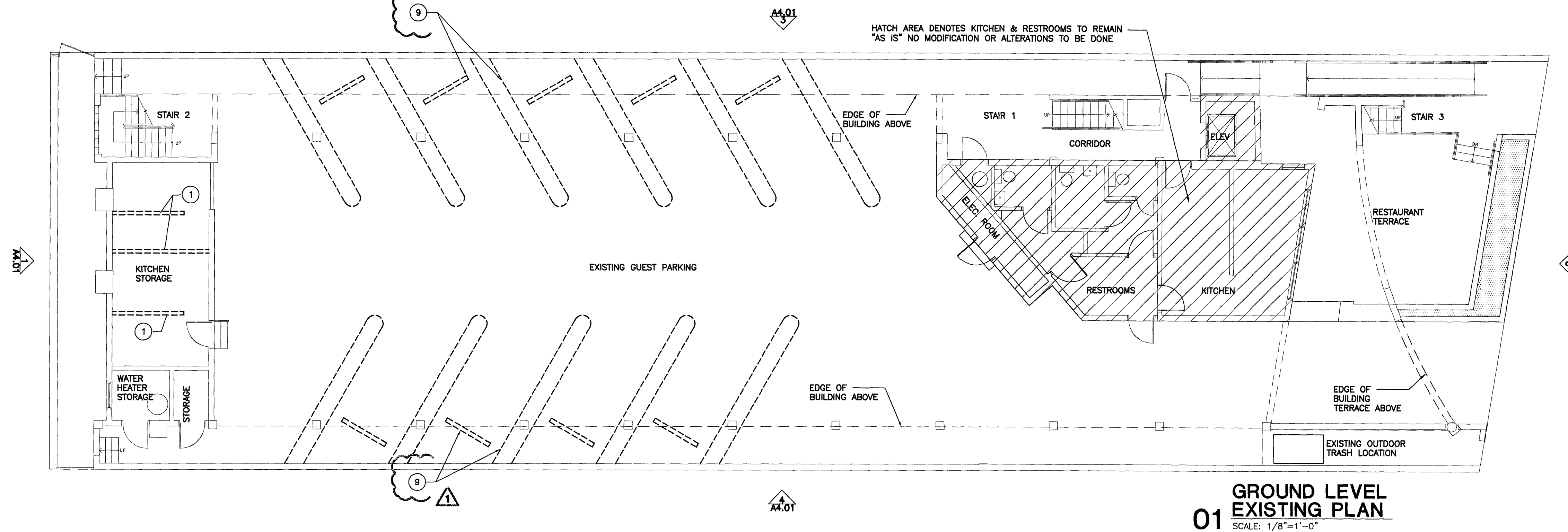
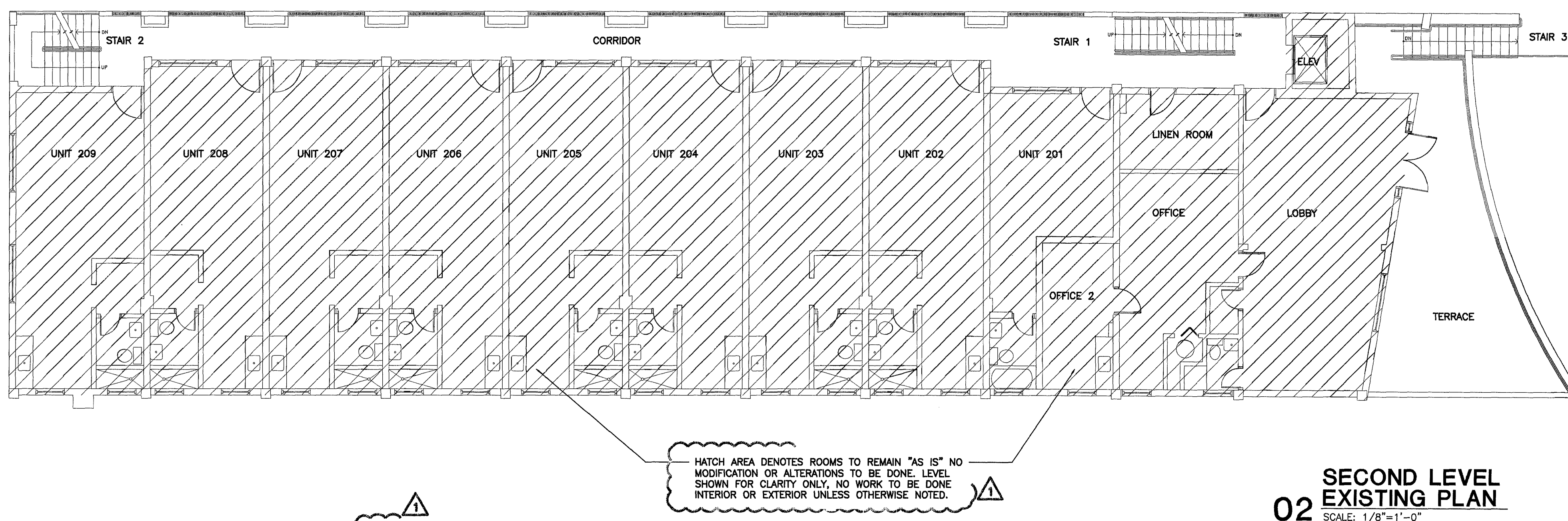
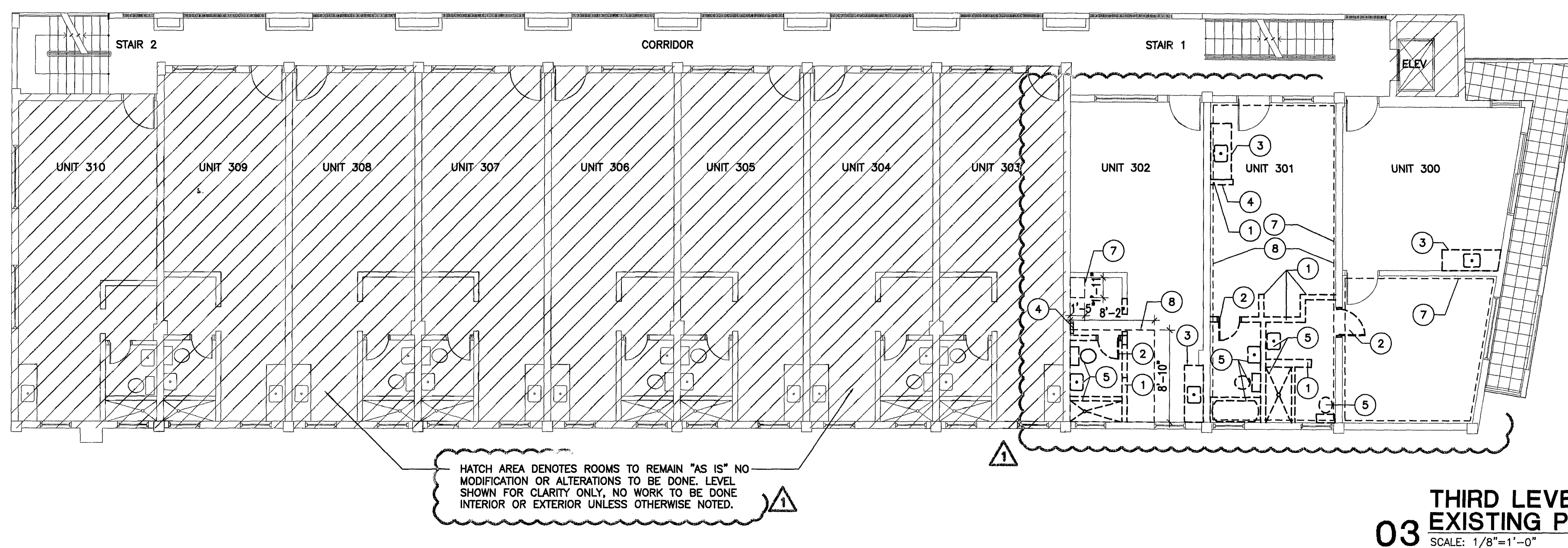
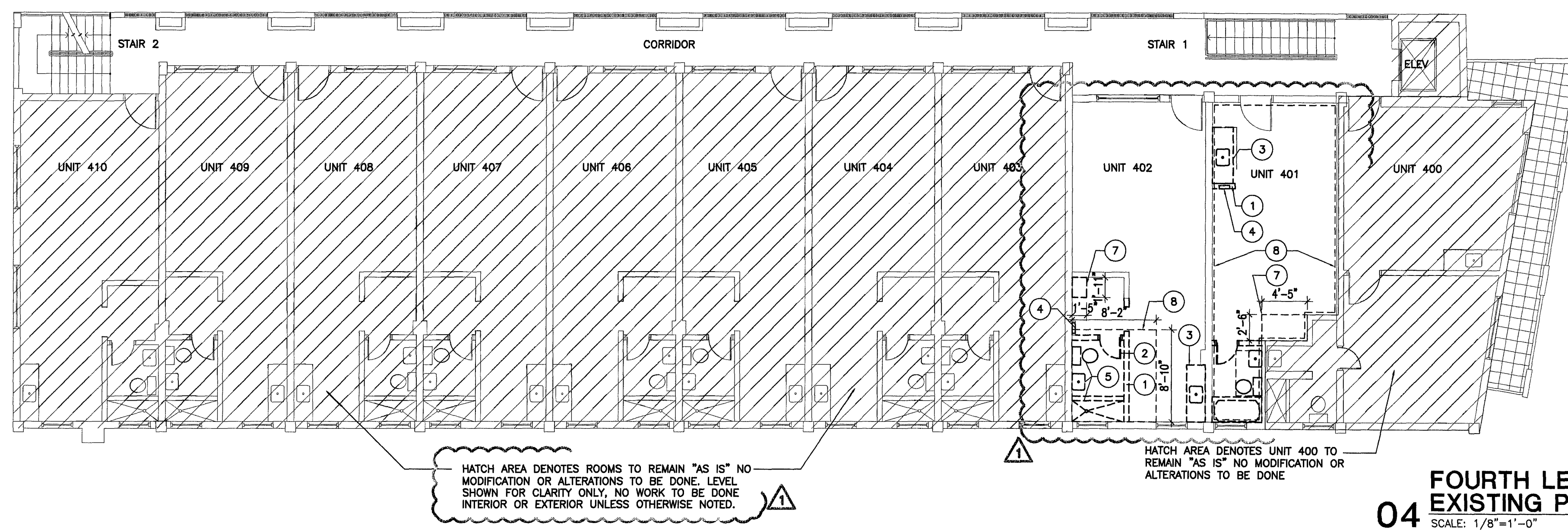
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MIAMI BEACH, FLORIDA 33139

LAWRENCE BEAME, R.A.
REGISTRATION # 7871



DEMOLITION KEY NOTES:

- EXISTING PORTION OR ENTIRETY OF WALLS TO BE REMOVED TO ACCOMMODATE NEW LAYOUT CONFIGURATION, REFER TO SHEET A2.01.
- EXISTING DOORS & FRAME TO BE REMOVED. REFER TO SHEET A2.01 FOR NEW WALL TYPE
- EXISTING KITCHEN SINK, CABINETS, COUNTER TOP TO BE REMOVED. REFER TO MEP FOR ADDITIONAL DEMOLITION NOTES.
- EXISTING ELECTRICAL PANEL TO BE REMOVED, RE-ROUTE NEW CONDUIT TO NEW ELECTRICAL PANEL LOCATION. REFER TO ELECTRICAL PLANS FOR ADDITIONAL INFORMATION.
- EXISTING BATHROOM FIXTURES, TO BE REMOVED TO ACCOMMODATE NEW ADA BATHROOM & FIXTURES.
- PORTION OF EXISTING CEILING TO BE REMOVED TO ALLOW FOR CHASE FOR NEW BATHROOM VENT, REFER TO DETAIL SHEET FOR ADDITIONAL INFORMATION.
- EXISTING OR ENTIRETY PORTION OF CEILING TO BE REMOVED TO ALLOW FOR NEW CEILING EXHAUST FAN CONTRACTOR TO FIELD VERIFY ALL CONDITIONS TO DETERMINE AMOUNT OF DEMOLITION REQUIRED.

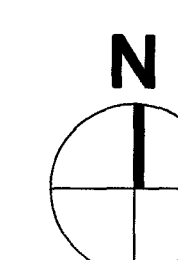
- ENTIRETY OR PORTION OF EXISTING FLOOR TO BE REMOVED.

- EXISTING PARKING WHEEL STOPS TO BE REMOVED. ALL PARKING STRIPPING PAINT TO BE REMOVED. PREPARE TO RECEIVE NEW STRIPPING.

DEMOLITION GENERAL NOTES:

- ALL BUILDING ELEMENTS INTERIOR & EXTERIOR TO REMAIN UNLESS OTHERWISE NOTED.
- AS-BUILT CONDITIONS MAY NOT COINCIDE WITH THOSE INDICATED ON THE CONSTRUCTION DOCUMENTS. CONTRACTOR SHALL BE RESPONSIBLE FOR DEMOLITION, BUT NOT LIMITED TO, ALL THE ITEMS LISTED FOR DEMOLITION IN THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL INSPECT THE EXISTING CONDITIONS AND SHALL BE RESPONSIBLE FOR DEMOLITION OF EXISTING WORK AS REQUIRED TO COMPLETE THE NEW WORK.
- REFER TO MEP DRAWINGS FOR ADDITIONAL DEMOLITION NOTES.
- CONTRACTOR TO CONTACT ARCHITECT OF ANY DISCREPANCIES FOUND ON SITE NOT NOTED ON THE CONSTRUCTION DOCUMENTS

REVIEWED
CITY OF MIAMI BEACH
FIRE DEPARTMENT



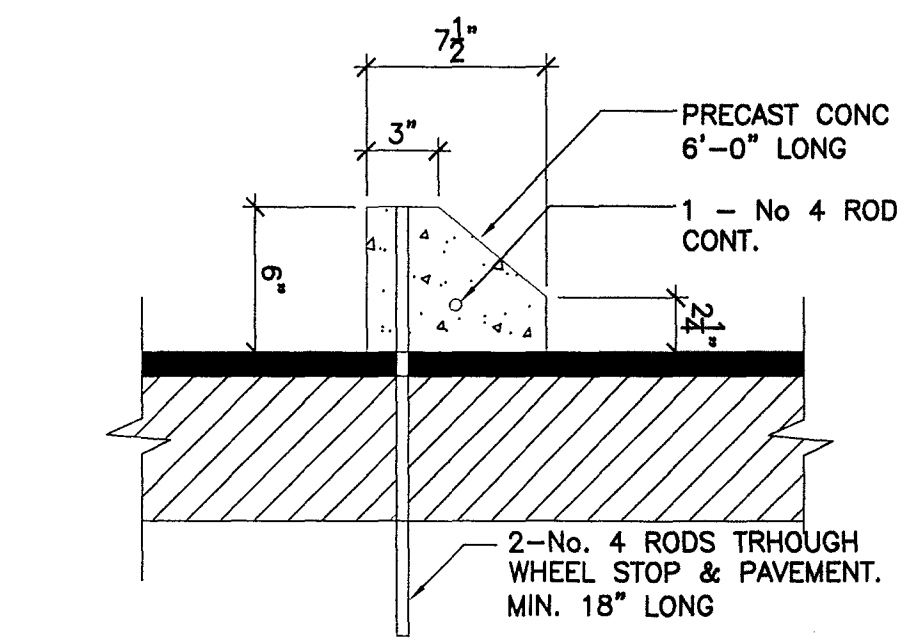
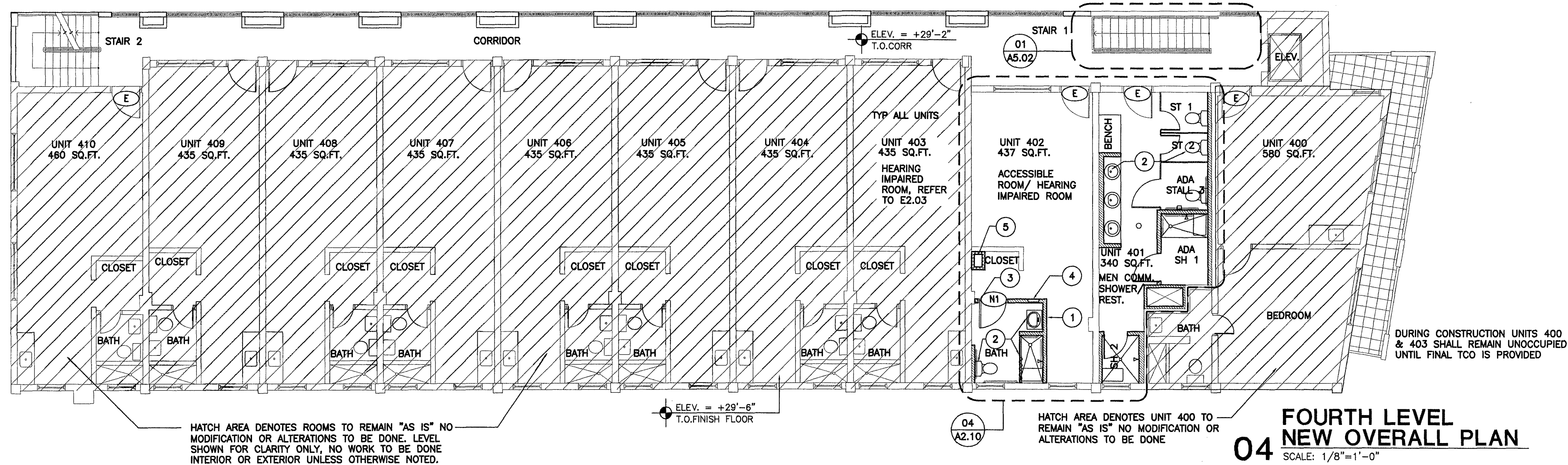
**HOTEL EVA
INTERIOR
IMPROVEMENTS**
1506 COLLINS AVENUE
MIAMI BEACH,
FLORIDA 33139



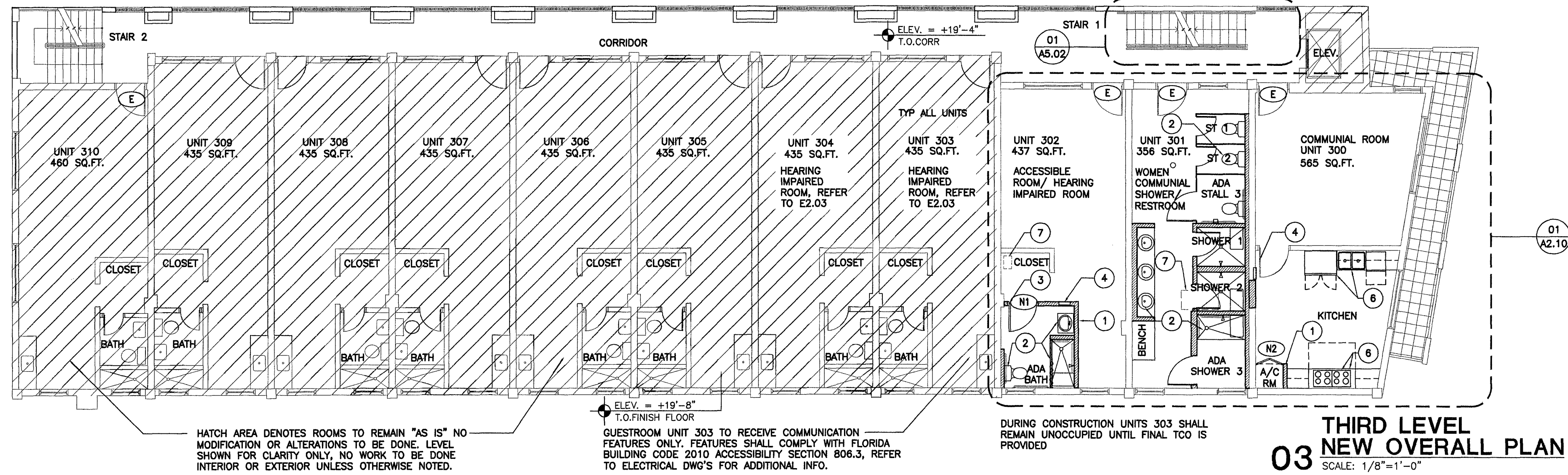
DEMOLITION FLOOR PLANS

JOB NUMBER
14010.00
DATE
05-23-14
SHEET NUMBER
A2.01

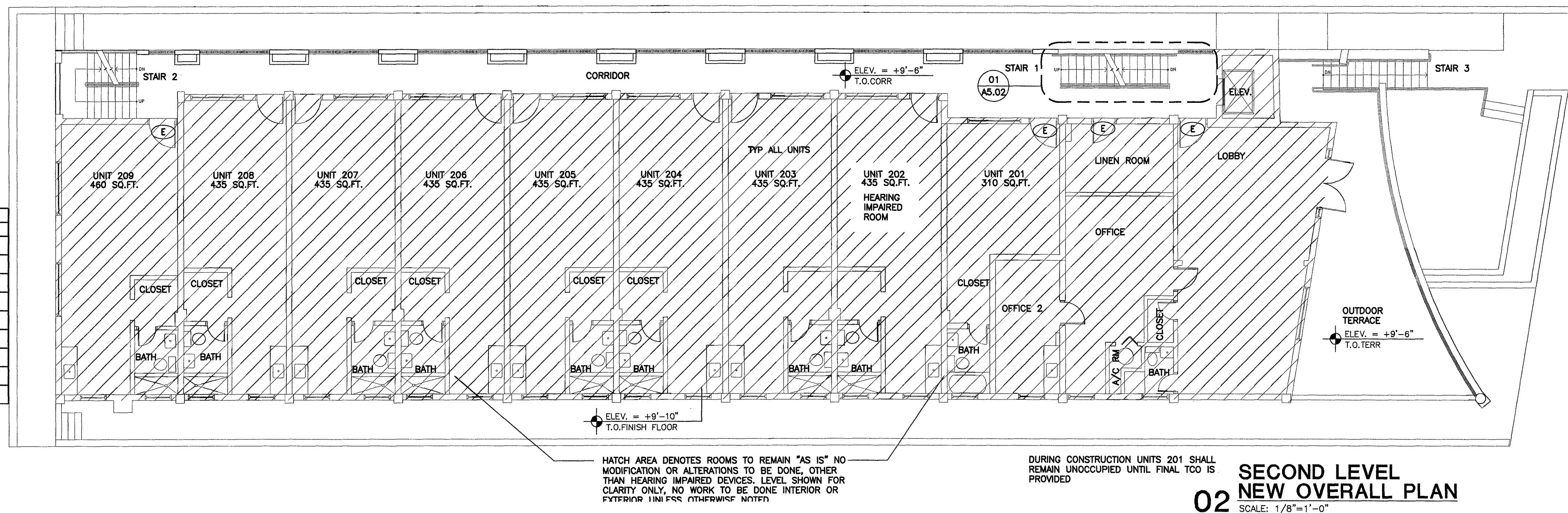
UNIT	SQ. FT.
400	580 SQ.FT.
401	340 SQ.FT.
402	437 SQ.FT.
403	435 SQ.FT.
404	435 SQ.FT.
405	435 SQ.FT.
406	435 SQ.FT.
407	435 SQ.FT.
408	435 SQ.FT.
409	435 SQ.FT.
410	480 SQ.FT.



UNIT	SQ. FT.
300	565 SQ.FT.
301	356 SQ.FT.
302	437 SQ.FT.
303	435 SQ.FT.
304	435 SQ.FT.
305	435 SQ.FT.
306	435 SQ.FT.
307	435 SQ.FT.
308	435 SQ.FT.
309	435 SQ.FT.
310	480 SQ.FT.



UNIT	SQ. FT.
201	310 SQ.FT.
202	435 SQ.FT.
203	435 SQ.FT.
204	435 SQ.FT.
205	435 SQ.FT.
206	435 SQ.FT.
207	435 SQ.FT.
208	435 SQ.FT.
209	480 SQ.FT.



GENERAL NOTES		
TOTAL GUEST ROOMS AND AMOUNT OF ROOMS THAT HAVE MOBILITY AND COMMUNICATION FEATURES WHICH COMPLY WITH THE FAC224 ON TABLE 224.2 AND 224.4		
TOTAL NUMBER OF UNITS	28	
TOTAL NUMBER OF UNITS WITH COMMUNICATION FEATURES (HEARING IMPAIRED)	6	
TOTAL NUMBER OF ACCESSIBLE UNITS WITH MOBILITY FEATURES (ADA ROOMS)	2	
UNIT 202, 302, 303, 304, 402 & 403		
UNIT 302 & 402		

- KEY NOTES**
- NEW WALL PARTITIONS REFER TO ENLARGE PLANS FOR TYPICAL WALL TYPE & NOTES
 - NEW BATHROOM FIXTURES, REFER TO ENLARGE PLANS FOR ADDITIONAL INFORMATION
 - NEW 3'-0" SOLID CORE WOOD DOORS
 - NEW ELECTRICAL PANELS, REFER TO ELECTRICAL DWG'S FOR ADDITIONAL INFORMATION
 - NEW CHASE WALL FOR BATHROOM VENTS, REFER TO ENLARGE PLANS FOR ADDITIONAL INFORMATION
 - NEW KITCHEN CABINETS, FIXTURES & APPLIANCES, REFER TO ENLARGE PLAN & MEP DRAWINGS FOR ADDITIONAL INFORMATION
 - LOCATION OF CHASE ABOVE

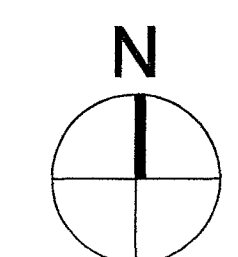
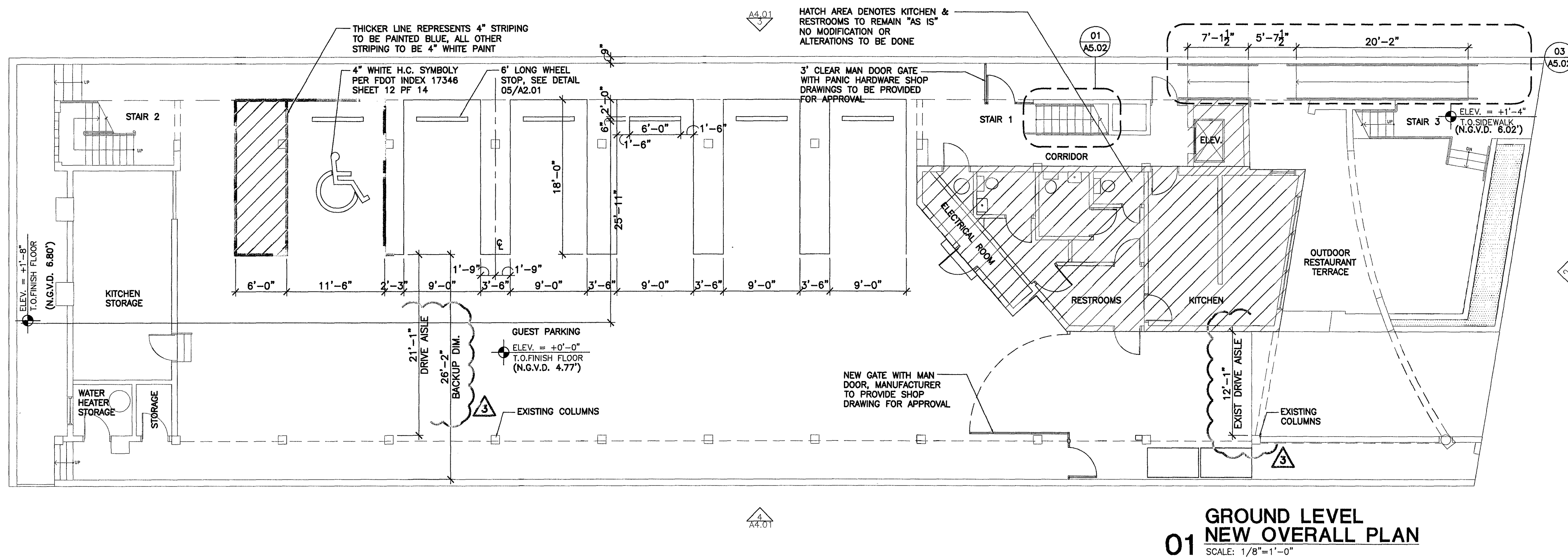
INTERIOR IMPROVEMENT NOTES

-ANY EXISTING PARTITIONS DAMAGED DURING DEMOLITION TO BE PATCH & RE-FINISH TO ITS ORIGINAL CONDITIONS.

-CONTRACTOR SHALL CAREFULLY VERIFY IN FIELD ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO COMMENCING WORK. ANY DISCREPANCIES MUST BE NOTIFIED TO ARCHITECT.

-ALL EXISTING CONDITIONS TO REMAIN UNLESS OTHERWISE NOTED, TYP.

-GC TO VERIFY DOORS MARKED AS (E) TO BE 90 MIN. FIRE RATED. IF NOT REPLACE WITH 90 MIN. RATED DOORS, PER PHASING PLAN A1.03



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HOTEL EVA

INTERIOR IMPROVEMENTS

1506 COLLINS AVENUE
MIAMI BEACH,
FLORIDA 33139

NEW OVERALL FLOOR PLANS

JOB NUMBER
14010.00

DATE
05-23-14

SHEET NUMBER
A2.02

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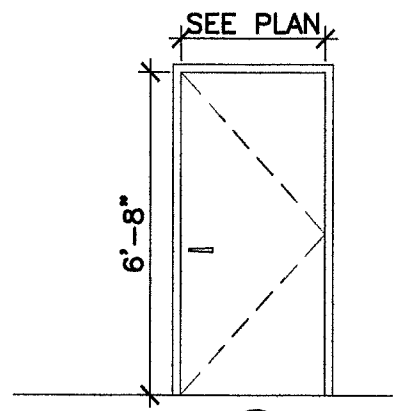
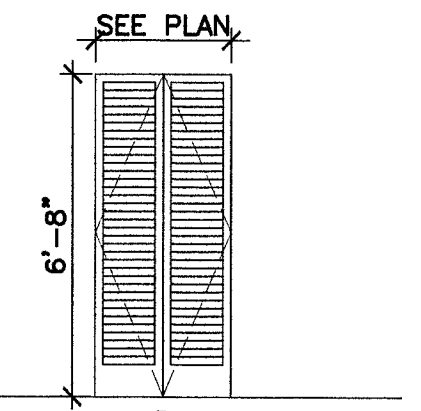
REVISIONS	DATE	ITEM
3	01-19-15	BUILDING COMMENTS
2	11-19-14	BUILDING COMMENTS
1	10-06-14	CITY COMMENTS/ CHANGE OF SCOPE
	05-23-14	ISSUED FOR PERMIT

**HOTEL EVA
INTERIOR
IMPROVEMENTS**
1506 COLLINS AVENUE
MIAMI BEACH,
FLORIDA 33139

PROJ. NUMBER: 14010.00
DRAWING BY: VS
SCALE: AS SHOWN
SHEET TITLE: ENLARGE FLOOR PLANS

ENLARGE FLOOR PLANS

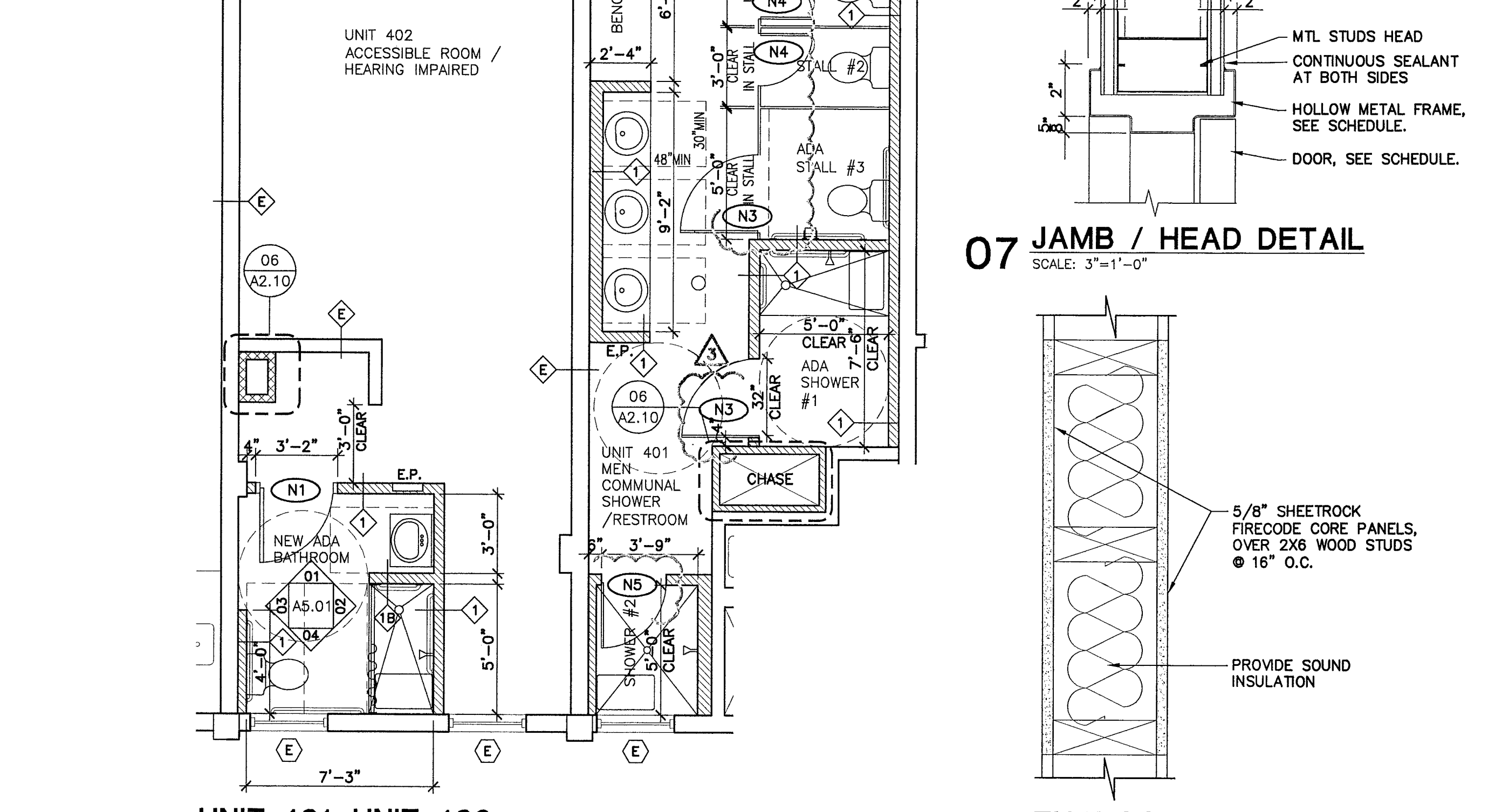
JOB NUMBER: 14010.00
DATE: 05-23-14
SHEET NUMBER: A2.10

NEW DOOR SCHEDULE												
DOOR NO.	DIMENSIONS			TYPE	MATERIAL	DOOR		HARDWARE SET NO	FRAME			REMARKS
	WIDTH	HEIGHT	THICK.			FIN	MATERIAL		FINISH	JAMB DET.	HEAD DET.	
N1	3'-0"	6'-8"	1 3/8"	A	SOLID CORE WOOD	PAINT GRADE	SET #1	HOLLOW METAL	-	06/A2.10	06/A2.10	BI-FOLD WITH TRACK ON TOP
N2	2'-10"	6'-8"	1 1/2"	B	SOLID CORE WOOD	PAINT GRADE	-	-	-	-	-	
N3	3'-0"	4'-10"	1"		SOLID POLYMER PLASTIC	WHITE	SET #2	HARDWARE SET:				
N4	2'-0"	4'-10"	1"		SOLID POLYMER PLASTIC	WHITE	SET #3	SET #1 (1) CLOSER, (3) BUTT HINGES, (1) PRIVACY SET, (1) WALL STOP				
N5	2'-6"	4'-10"	1"		SOLID POLYMER PLASTIC	WHITE	SET #3	SET #2 (2) CONTINUOUS HINGE (1) ADA CONCEAL LATCH SET (1) ADA DOOR PULL (1) BUMPER HOOK, (1) OUTSWING STRIKE & KEEPER (1) SELF-CLOSER				
								SET #3 (2) PARTITION HINGE TOP & BOTTOM, (1) CONCEAL LATCH SET, (1) BUMPER HOOK, (1) INSWING STRIKE & KEEPER				
DOOR TYPES						DOOR NOTES						
						GENERAL NOTE: --ALL EGRESS DOORS SHALL NOT REQUIRE A KEY OR SPECIAL KNOWLEDGE TO OPEN FOR EGRESS. --ALL DOORS ALONG THE CORRIDOR TO HAVE SELF-CLOSING & LATCHING HARDWARE PER FFPC 2010 29.3.6.2.3. --DOOR HANDLES,PULLS, LATCHES LOCKS & OTHER OPENING DEVICE SHOULD HAVE A SHAPE THAT IS EASY TO OPERATE WITH ONE HAND, WITHOUT GRASPING, PINCHING OR TWISTING OF THE WRIST, THEY SHOULD BE MOUNTED NO HIGHER THAN 48" ABOVE FINISH FLOOR.						
						LEGEND: (E) GC TO VERIFY EXISTING DOORS TO BE 90 MIN. FIRE RATED. IF NOT REPLACE WITH 90 MIN. RATED DOORS, PER PHASING PLAN A1.03 (E) EXISTING WINDOWS TO REMAIN (E) EXISTING WALLS TO REMAIN						
SOLID CORE WOOD DOOR						SOLID CORE WOOD DOOR						

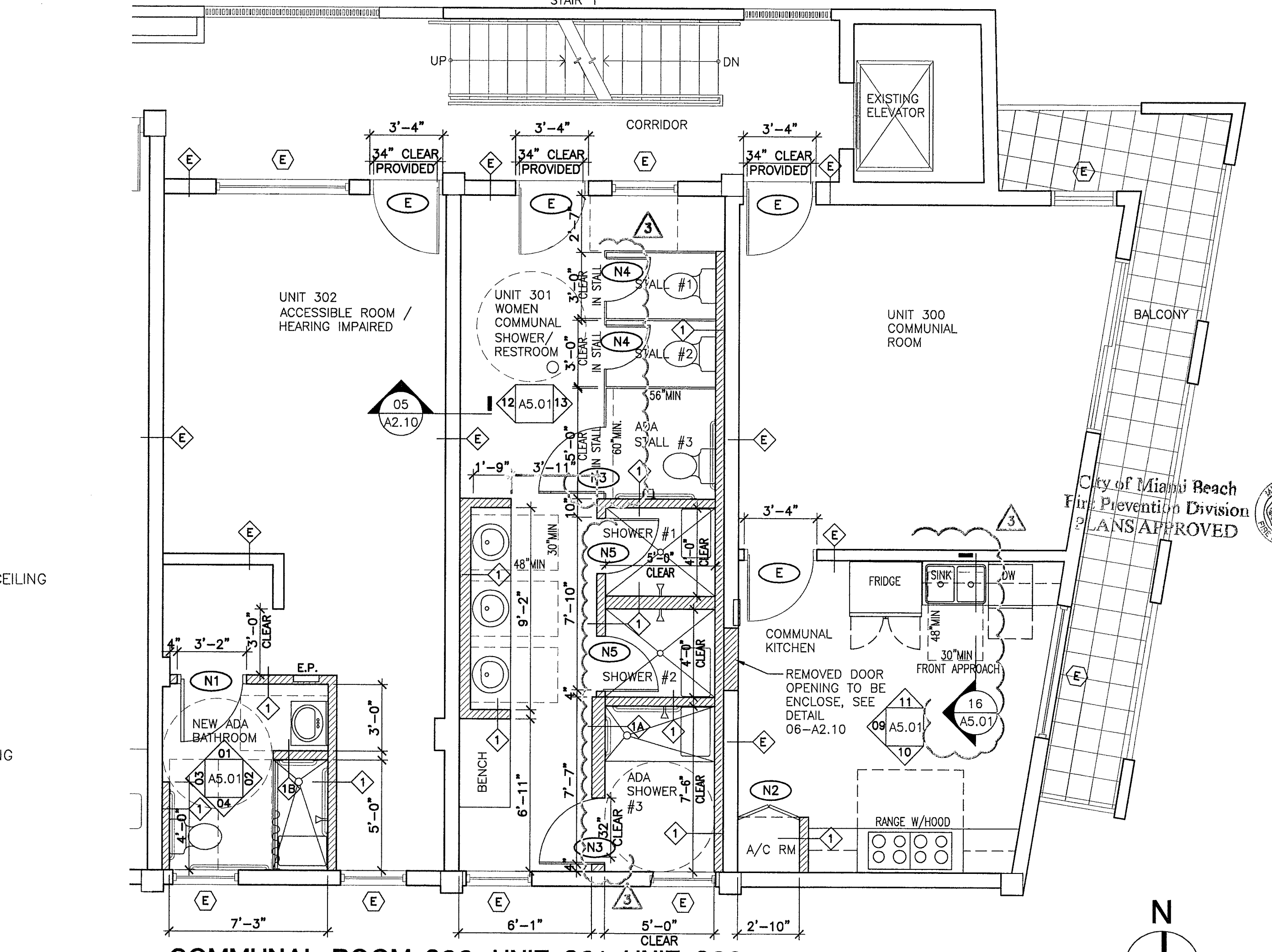
DOOR TYPES
SEE PLAN
A SOLID CORE WOOD DOOR
B SOLID CORE WOOD DOOR

DOOR NOTES
GENERAL NOTE:
-ALL EGRESS DOORS SHALL NOT REQUIRE A KEY OR SPECIAL KNOWLEDGE TO OPEN FOR EGRESS.
-ALL DOORS ALONG THE CORRIDOR TO HAVE SELF-CLOSING & LATCHING HARDWARE PER FPPC 2010 29.3.6.2.3.
-DOOR HANDLES, PULLS, LATCHES, LOCKS & OTHER OPENING DEVICES SHOULD HAVE A SHAPE THAT IS EASY TO OPERATE WITH ONE HAND, WITHOUT GRASPING, PINCHING OR TWISTING OF THE WRIST, THEY SHOULD BE MOUNTED NO HIGHER THAN 48" ABOVE FINISH FLOOR.

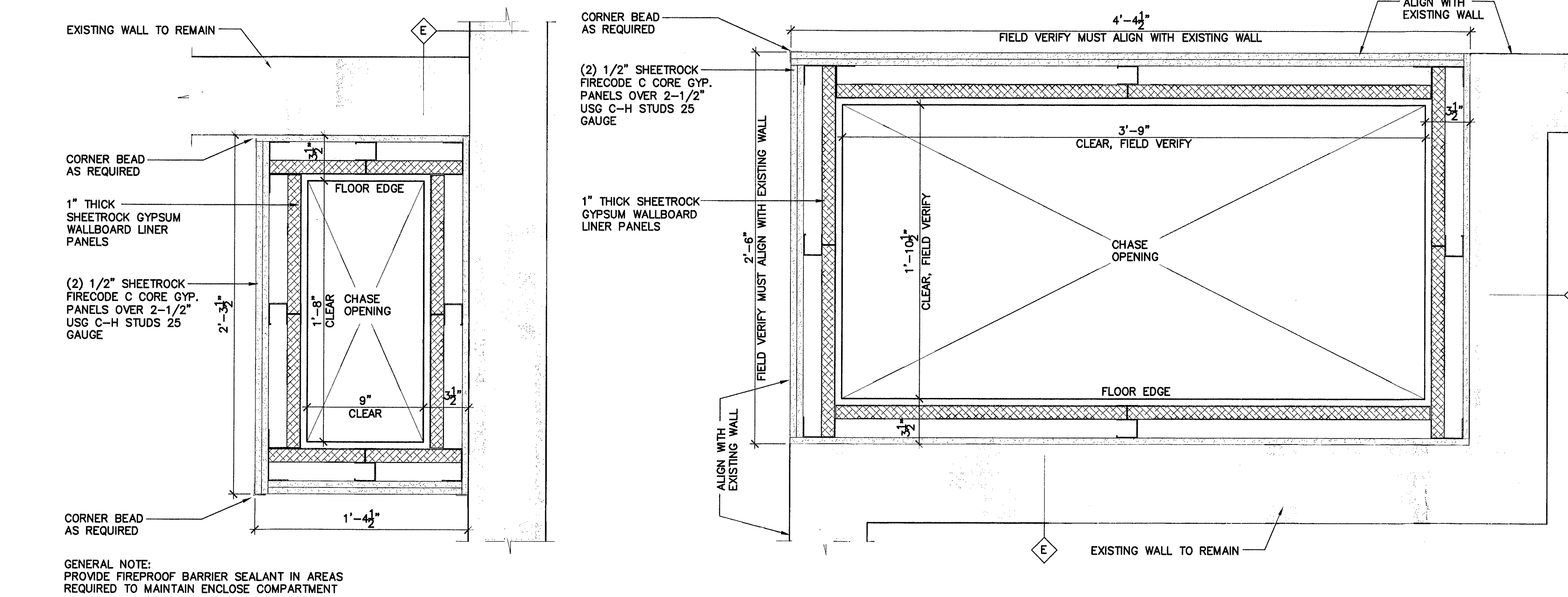
LEGEND:
GC TO VERIFY EXISTING DOORS TO BE 90 MIN. FIRE RATED. IF NOT REPLACE WITH 90 MIN. RATED DOORS, PER PHASING PLAN A1.03
E EXISTING WINDOWS TO REMAIN
E EXISTING WALLS TO REMAIN



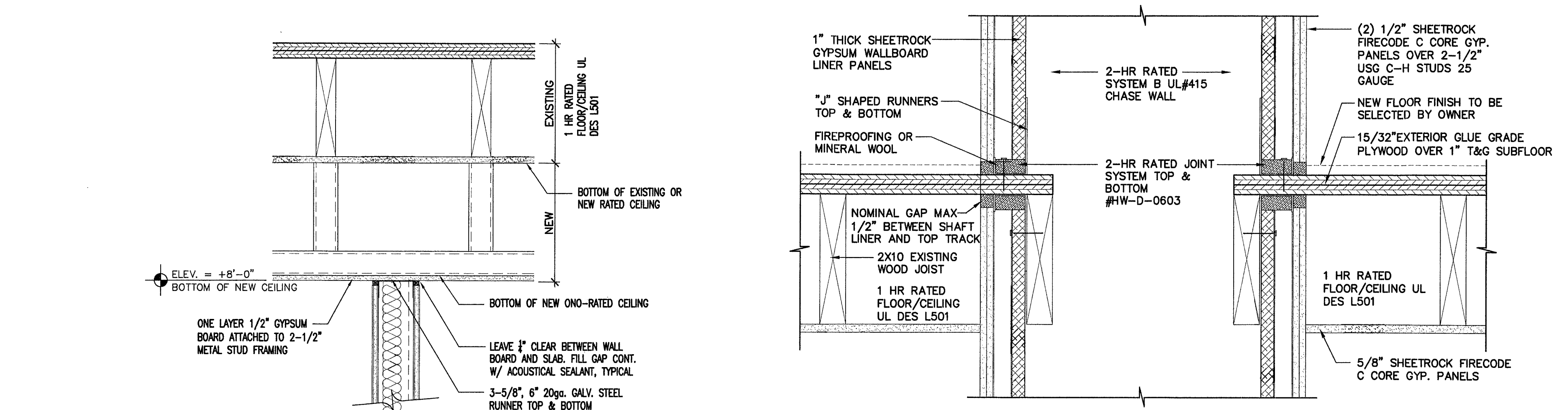
**04 UNIT 401, UNIT 402
ENLARGE 4TH LEVEL FLOOR PLAN**
SCALE: 1/4"=1'-0"



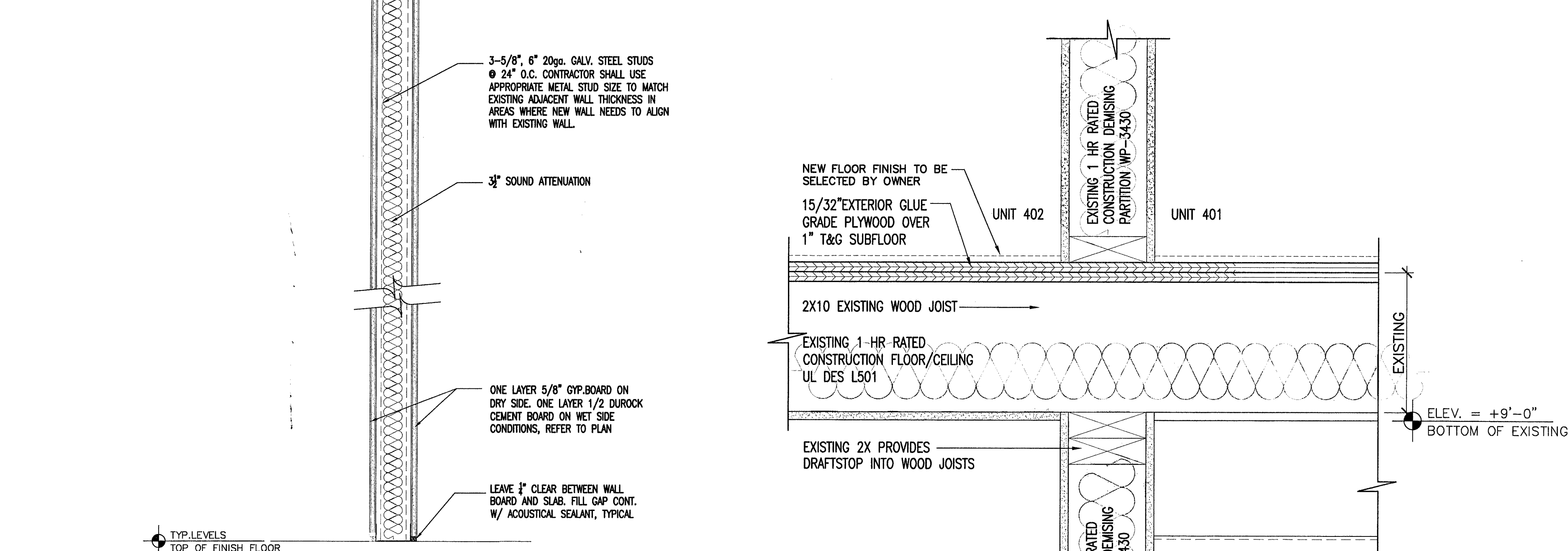
**01 COMMUNAL ROOM 300, UNIT 301, UNIT 302
ENLARGE FLOOR PLAN**
SCALE: 1/4"=1'-0"



**02 CHASE WALL
2 HR RATED SYSTEM B UL#U415**
SCALE: N.T.S.



**03 CHASE WALL
2 HR RATED SYSTEM B UL#U415**
SCALE: N.T.S.

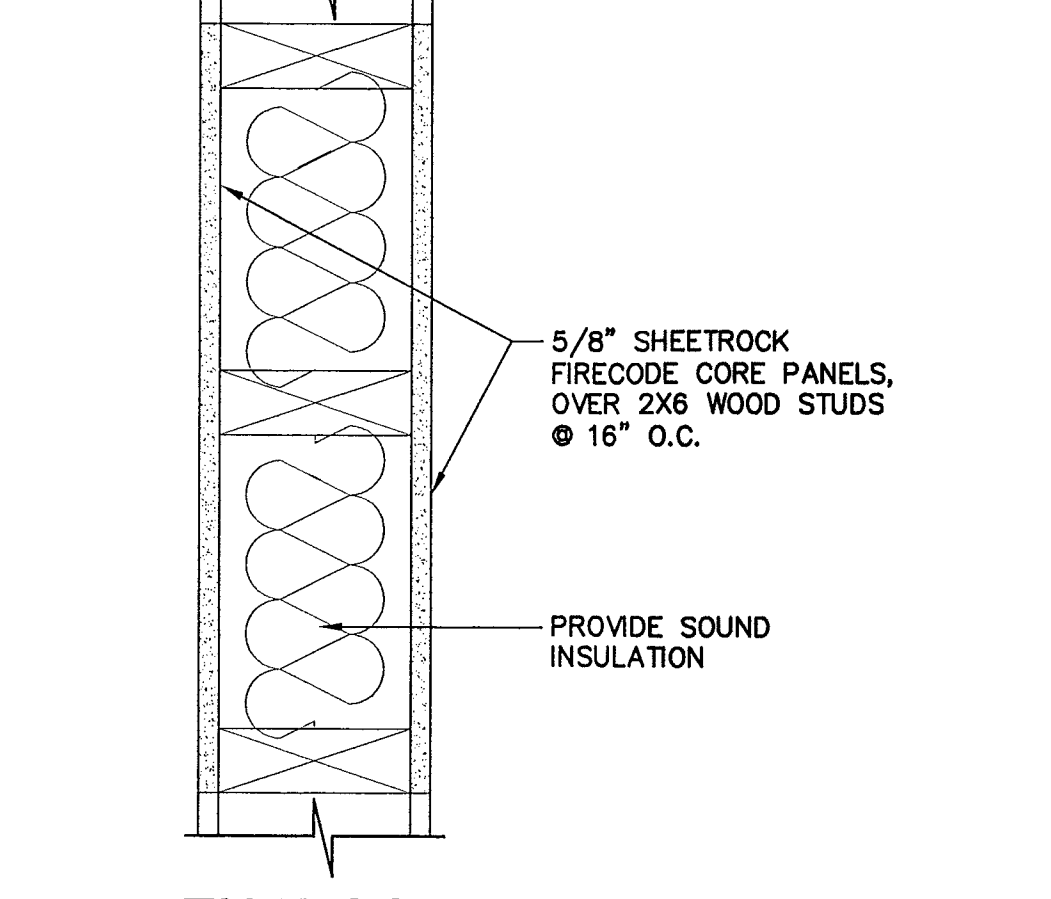


**04 CHASE WALL SECTION
2 HR RATED SYSTEM B UL#U415**
SCALE: N.T.S.

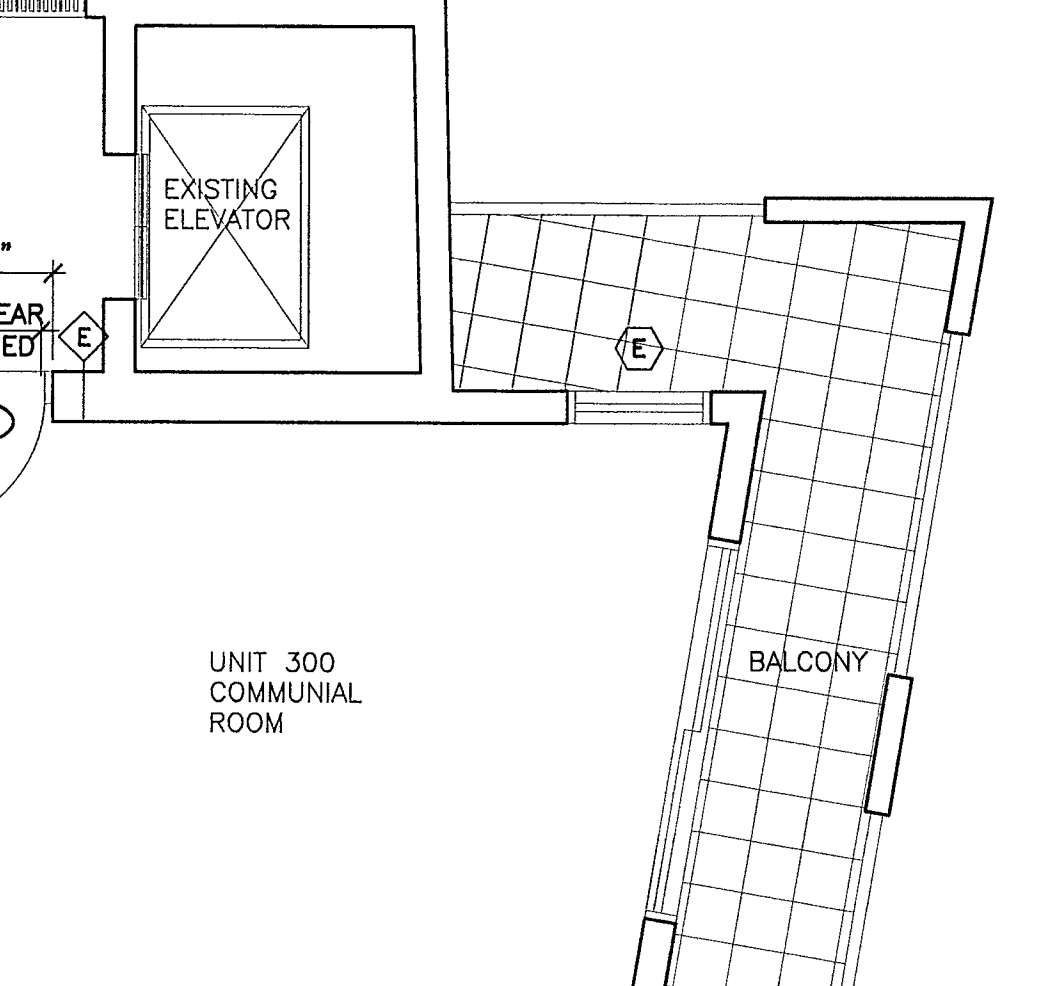


**05 EXISTING 1 HR RATED
DEMISING WALL UL DESIGN L501**
SCALE: N.T.S.

07 JAMB / HEAD DETAIL
SCALE: 3/4"=1'-0"



**06 ENCLOSED WALL PLAN
1 HR RATED UL#305**
SCALE: N.T.S.



08 WALL TYPE 3-5/8"
SCALE: 1-1/2"=1'-0"
NON-RATED

09 WALL TYPE 6"
SCALE: 1-1/2"=1'-0"
NON-RATED

10 WALL TYPE 3-5/8"
SCALE: 1-1/2"=1'-0"
DO NOT PROVIDE SOUND ATTENUATION.

11 WALL TYPE 3-5/8"
SCALE: 1-1/2"=1'-0"
DO NOT PROVIDE SOUND ATTENUATION.

12 WALL TYPE 3-5/8"
SCALE: 1-1/2"=1'-0"
DO NOT PROVIDE SOUND ATTENUATION.

13 WALL TYPE 3-5/8"
SCALE: 1-1/2"=1'-0"
DO NOT PROVIDE SOUND ATTENUATION.