CITY OF MIAMI BEACH
BUILDING DEPARTMENT
1700 CONVENTION CENTER DRIVE
2ND FLOOR - CITY HALL
MIAMI BEACH, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I, (we) have been retained by: Claro Development to perform special inspector services under the Florida Building Code at the 4385 Collins Avenue project on the below listed structures as of 10/15/10 (date). I am a professional engineer licensed in the State of Florida.

Process Number: B1100002 **Master Permit (IF APPLICABLE):**

- ○ Special Inspector for Pilings, FBC 1822.1.20
- ○ Special Inspector for Soil Compaction, FBC 1820.3.1
- ○ Special Inspector for Precast Attachments, FBC 1927.12.2 (By P.E. or R.A..)
- ○ Special Inspector for Reinforced Masonry, FBC 2122.4
- ⊗ xx Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A..)
- ○ Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R. A..)
- ○ Special Inspector for

NOTE: Only the marked boxes apply.

The following individual's) employed by this firm or me are authorized representatives to perform inspections*

1. Jose Fernandez, PE
2. Peter Koerber
3. Federico Garcia
4.

***NOTE: FBC 2001 HVZ sections 1927.12.2, 2218.2, 2319.17.4.2 requires either a Registered professional Engineer or Registered Architect to perform the actual inspections.**

I, (we) will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, (we) understand that a Special Inspection Log for each building must be displayed in a convenient location on the site for reference by the City of Miami Beach Building Department Inspector. All mandatory inspections, as required of the Florida Building Code, Inspection performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. Further, upon completion of the work under each building permit, I will submit to the Building Inspector at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.

Architect/Engineer Signature:

Architect/Engineer Name Printed: Mustafa Cankat, PE

Address: 1900 SW 57 AVE?. #1, Miami, FL 33155

Phone Number: (305) 266-9777

Owner/Agent Signature: Ceeta Polak - Agent

Owner/Agent Name Printed:

Building Department Accepted By: Ferrer 12/6/11

M. Cankat
Signed and Sealed

#18632
License Number

Date: 10/15/10

CANKAT-ESSMAN, INC.
1900 SW 57 Avenue, #1
Miami, FL 33155
Tel: (305) 266-9777
Fax: (305) 266-0584
mus@cankatessman.com
www.cankatessman.com

October 20, 2010

City of Miami Beach
Building Department
1700 Convention Center Drive
2nd Floor City Hall
Miami Beach, FL

Attention: Mr. R. L. Valdes, PE
Structural Plan Reviewer

Re: 4385 Collins Ave., Miami Beach, FL
Soho Beach House / Court Yard Pergola
Structural B 1100002
Our Job No. 10017

Dear Sir:

The following are our responses to your Structural Review Comments, dated 10/7/10.

ITEM #1:
Comment: PROVIDE FOUNDATION FOR TRELLIS COLUMNS. NOTE THAT DESIGN CRITERIA REFERS TO SOIL BEARING PRESSURE BUT THE MASTER PERMIT USED PILE FOUNDATION. IF TRELLIS IS SUPPORTED BY A CONCRETE STRUCTURE BY OTHERS (SLAB ON GRADE, STRUCTURAL SLAB OR CONTINUOUS OR INDIVIDUAL FOOTINGS), PROVIDE EVIDENCE OF SUCH.
RESPONSE: Trellis columns are supported on existing 8" reinforced concrete slab over pile foundation. Calculation page foundation information is corrected. See attached partial foundation plan of the area where the trellis columns are installed. See calculation page 11 maximum downward reaction (gravity + wind) is 2.05 kips. Punching shear at column base is less than 10 psi and distributed load is within the capacity of structural slab.

ITEM #2:
Comment: SPECIAL INSPECTIONS ARE REQUIRED FOR STRUCTURAL STEEL BOLTED AND WELDED CONNECTIONS. PROVIDE THREE FULLY EXECUTED COPIES OF SPECIAL INSPECTOR FORM.
RESPONSE: Special Inspector forms are provided. Please see attached.

ITEM #3:
Comment: SECTION 1/S-2. PROVIDE SIZE AND CONNECTIONS FOR HANGERS SUPPORTING TRELLIS FROM "EXISTING BEAMS BY OTHERS".

RESPONSE: Please see Section 6/S-2 for tubular hanger section size and weld size connecting them at top and bottom.

ITEM #4:
Comment: CALCULATIONS INDICATE A MOMENT CONNECTION BETWEEN BEAM AND POST AS AN ALL-AROUND 3/16" FILLET WELD. PROVIDE DETAIL FOR CONNECTION 2.
RESPONSE: Stability of the hanging pergola below the long bar structure has been originally provided in transverse direction (N-S) by connecting the new HSS 6x4x3/16 beams to concrete columns of the bar structure at each end in longitudinal direction (E-W) hangers were welded only. We revised Section 2/S-2 by adding cross bracing members to provide stability without relying on moment connection.

Yours truly,

M. Cankat

Mustafa Cankat, PE

Atts.

CANKAT-ESSMAN, INC.
CONSULTING ENGINEERS
1900 SW 57 Ave., Suite 1
MIAMI, FL 33155
T:305.266.9777 F:305.266.0584

STRUCTURAL CALCULATIONS

GENERAL INFORMATION

Project :	SOHO BEACH HOUSE Courtyard Pergola
Project Address:	4385 Collins Ave Miami Beach FL 33140
Date	9/21/2010
Our Job No.	10017
Client:	SOHO BEACH HOUSE

INDEX

TOTAL (21) PAGES

Mustafa Cankat, PE
Florida License #: 18632
C.A #: 404419

M. Cankat

10/27/2010

STRUCTURAL CALCULATIONS

Applicable Codes:

* Florida Building Code 2007
* ACI 318-99 Reinforced Concrete
* ACI 530-98 Reinforced Masonry
* ASTM C-90 Hollow unit masonry
* ASTM C-476 Grout
* AISC
* ASCE 7-05

Wind Criteria (as per ASCE 7-05)

Basic Wind speed 146 (3 second gust) High Velocity Wind Zone
Height <60 Ft.
Exposure C
Importance factor I= 1
Kd= 1
kdt= 0.85

Design Loads:

	Dead Load	S.I. DL	Live Load
Roof	10 psf		30 psf
Floor			
Terrace, Balcony /Stair	25 psf		100 psf

Pitch of roof /12 degrees
Eave height(z) Ft
Building width (L) Ft
Mean roof height(H) Ft

Pergola columns are supported on existing 8" concrete structural slab over Pile Foundations.
Assumed minimum concrete properties.

Concrete:	Foundation	Superstructure	Reinforcing Steel	
fc'=	3000 psi	4000 psi	fy=	60000 psi
E=	3321 ksi			

Structural Wood
All structural lumber shall be No.2 common grade Southern Pine or better grade

Structural Steel
Tubular sections: ASTM A-500 Gr B
All Others: ASTM A-36

WIND ANALYSIS

ASCE 7-05, MONOSLOPE FREE ROOFS, C&C $\Theta <= 7.5°$ **SOHO Beach**

h(feet)	9.25	mean roof height	$\Theta°$	0.00	OK< 7.5
Exposure	2	1 for B, 2 for C,1 for D	a(feet)	3.00	
K_{zt}	1.00	6.5.7	Exposure	C	
K_d	0.85	Table 6-4	α	9.5	Table 6-2
V	146	mph	z_g	900	Table 6-2
I	1.00	Table6-1	$K_{h(CASE\ 1)}$	0.85	Table 6-3
G	0.85	6.5.8	q_h	39.37	$.00256 K_z K_{zt} K_d V^2 I$
p(in/ft)	0.00	roof pitch	$q_h G$	33.47	psf
L(feet)	18.83	least horizontal dim.	h/L	0.49	OK
Flow	1	Enter1 for clear flow			

roof angle	sqft.	A_f	C_N Zone 3		Zone 2		Zone 1	
0°	9	$<=a^2$	2.4	-3.3	1.8	-1.7	1.2	-1.1
	36	$>a^2<=4a^2$	1.8	-1.7	1.8	-1.7	1.2	-1.1
	36	$>4a^2$	1.2	-1.1	1.2	-1.1	1.2	-1.1

roof angle	sqft	A_f	C_N Zone 3		Zone 2		Zone 1	
$\Theta°$	9	$<=a^2$	2.4	-3.3	1.8	-1.7	1.2	-1.1
	36	$>a^2<=4a^2$	1.8	-1.7	1.8	-1.7	1.2	-1.1
	36	$>4a^2$	1.2	-1.1	1.2	-1.1	1.2	-1.1

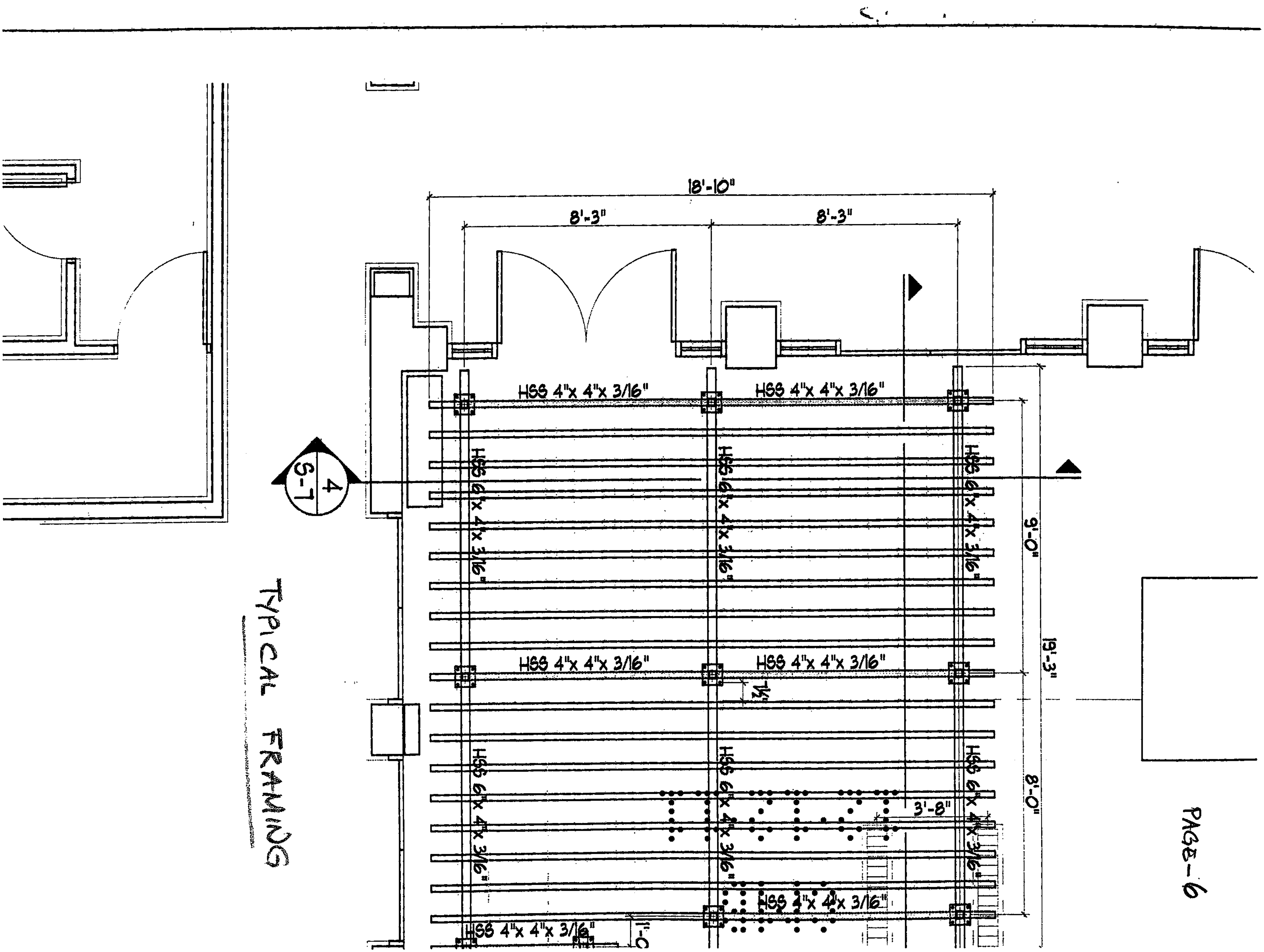
roof angle	sqft	A_f	C_N Zone 3		Zone 2		Zone 1	
7.5°	9	$<=a^2$	3.2	-4.2	2.4	-2.1	1.6	-1.4
	36	$>a^2<=4a^2$	2.4	-2.1	2.4	-2.1	1.6	-1.4
	36	$>4a^2$	1.6	-1.4	1.6	-1.4	1.6	-1.4

roof angle	sqft	A_f	$q_h G C_N$(psf) Zone 3		Zone 2		Clear Flow Zone 1	
$\Theta°$	9	$<=a^2$	80.32	-110.44	60.24	-56.90	40.16	-36.81
	36	$>a^2<=4a^2$	60.24	-56.90	60.24	-56.90	40.16	-46.86
	36	$>4a^2$	40.16	-36.81	40.16	-36.81	40.16	-36.81

4x4 WD
STL FRAME

FRAMING

STEEL BEAM/POST


18'-10"
8'-3"
8'-3"
HSS 4"x 4"x 3/16"
HSS 4"x 4"x 3/16"
HSS 6"x 4"x 3/16"
HSS 6"x 4"x 3/16"
HSS 6"x 4"x 3/16"
4
S-7
9'-0"
19'-3"
HSS 4"x 4"x 3/16"
HSS 4"x 4"x 3/16"
1½"
HSS 6"x 4"x 3/16"
HSS 6"x 4"x 3/16"
HSS 6"x 4"x 3/16"
8'-0"
3'-8"
HSS 4"x 4"x 3/16"
HSS 4"x 4"x 3/16"
TYPICAL FRAMING
PAGE-6

CANKAT ESSMAN 1900 SW 57th Ave. No:1 mIAMI, fl 33155 (305)266-9777	STEEL BEAM DESIGN SOHO BEACH HOUSE 9' LONG TUBULAR BEAM	Page 7 of Project No: 11/1/04 Design: M.C. Date: 09/02/10 Checked: F.O Date:

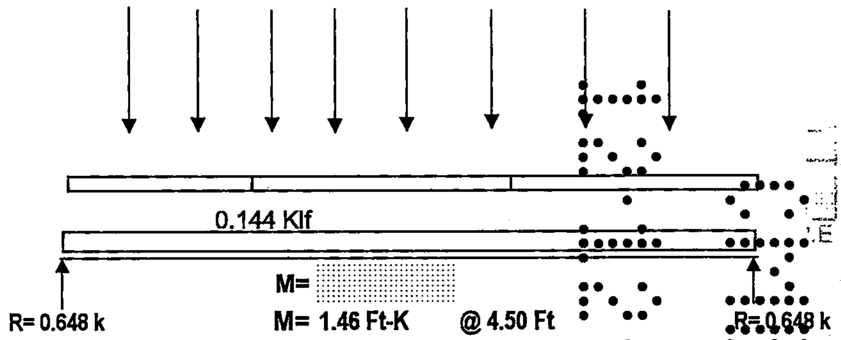
\\Secondchance\SharedDocs\Soho Beach\[steelbeam.xls]SECTION — Programmed by: Fuat Omarli,P.E.

BEAM HSS 6x4 x 3/16

Beam Data Material: 46 Ksi
Span length: 9.00 Ft
Section: Compact

		Distances from left Start	End	Total	Appl. Point	Moment
Uniform Load:	0.144 Klf		9.0 Ft			
Partial uniform Load						
Partial uniform Load						
Partial uniform Load			9.0 Ft			

Point Load	P(K)	x(Ft) from left	Dist to mid span	Dist to point of inflection
P1				
P2				
P3				
P4				
P5				
P6				
P7				
P8				
P9				
P10				



M=
M= 1.46 Ft-K @ 4.50 Ft
1.46 Ft-K
1.44 Ft-K

RESULT

M= 1.46 Ft-K x 12.00 = 17.5 in-K

$S_{req'd} = M_{max}/F_b =$ 17.5 / 30.36 = **0.58 in3**

Section Properties of Laterally Supported Beam

TYPE: HSS

SIZE: HSS6X4X3/16

WEIGHT	12.0 Lbs/ft			
AREA	3.28 in2			
DEPTH	6.00 in			
WIDTH	4.000 in			
t web				
t flange				
t	0.188 in			
Ix	16.40 in4			
Sx	5.46 in3	>	0.58 in3	OK
r x	2.23 in			
Iy	8.76 in4			
Sy	4.38 in3			
ry	1.63 in			

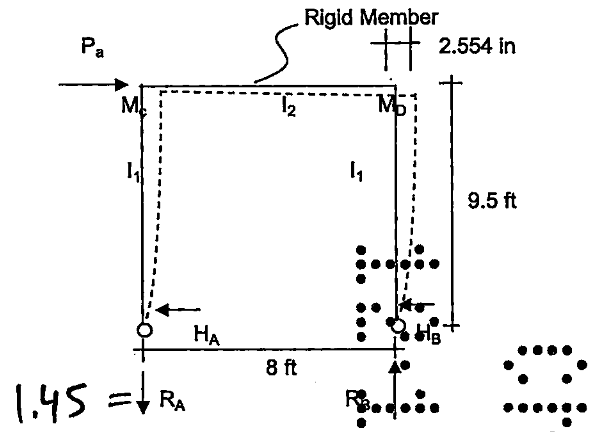
Deflection Check: As per FBC '02 L/360

$\Delta = 5/48 * M * L^2 / E * I =$ 0.04 in < 0.30 in
OK

Vibration Check:

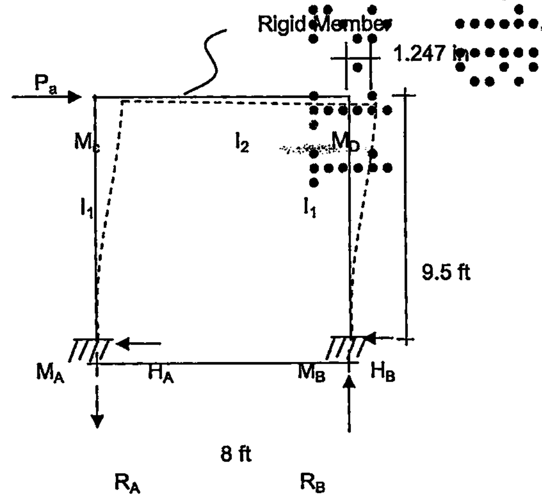
$f = 3.5\sqrt{1/\Delta}$ = 3.5 $\sqrt{22.37}$ = 16.56 OK >5

Lateral Force, P:	1.2 K	
Wall length,L:	8.25 Ft ✓	
Wall height,h:	9.5 Ft	
Flexural mod. of Elasticity,E:	29000 Ksi	
I_2 horiz.	12 in^4	$k=I_2\ h / I_1\ L$
I_1 Vert.	2 in^4	k= 6.909



PIN SUPPORTS

H_A =	0.6 K ✓
H_B=	0.6 K
M_C=	-5.7 K-Ft ✓
M_D=	-5.7 K-Ft
$\Delta= M_C *L2/ 6EI$ =	2.554 in

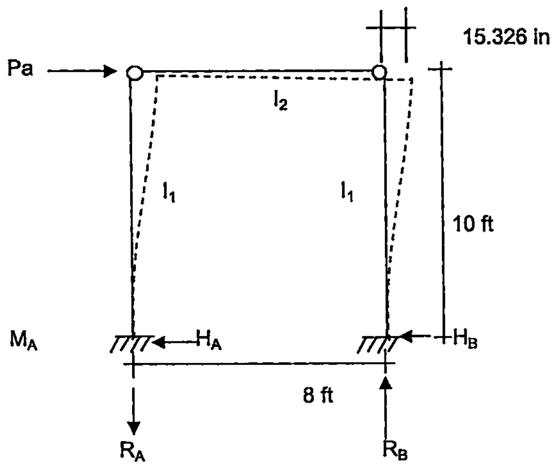


FIXED SUPPORTS

H_A =	0.6 K
H_B=	0.6 K
$R_A= -R_B= -3PH/L * k/(6k+1)$=	-0.67 K ✓
$M_A= -M_B=-PH/2 *[(3K+1)/(6K+1)]$=	2.92 K-Ft
$M_C=H*h-M_A$=	2.78 K-Ft
$M_D=-H*h+M_B$=	-2.78 K-Ft
$\Delta= M_C *L2/ 6EI$ =	1.247 in

FIXED SUPPORTS AND PINNED AT TOP

H_A =	0.6 K
H_B=	0.6 K
$R_A= -R_B= PH/L$ =	0.0 K
$M_A=H*h$=	5.70 K-Ft
$M_B=H*h$=	5.70 K-Ft
$\Delta= P*L^3/ EI$ =	15.326 in



CANKAT ESSMAN	STEEL BEAM DESIGN	Page 9	of
1900 SW 57th Ave. No:1	SOHO BEACH HOUSE	Project No:	11/1/04
mIAMI, fl 33155	18' LONG TUBULAR BEAM	Design: F.O	Date: 09/02/10
(305)266-9777		Checked: MC	Date:

\\Secondchance\SharedDocs\Soho Beach\[steelbeam.xls]SECTION

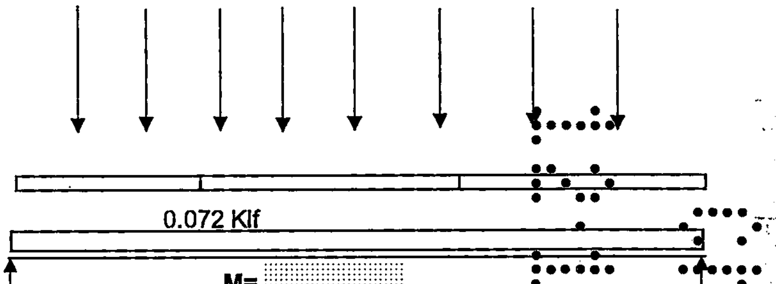
Programmed by: Fuat Omarli,P.E.

HSS 6x4 x3/16

SINGLE SPAN W/LONG SPAN CONDITON

Beam Data Material	46 Ksi						
Span length:	18.00 Ft						
Section	Compact	Distances from left					
		Start	End	Total	Appl. Point	Moment	
Uniform Load:	0.072 Klf		18.0 Ft				
Partial uniform Load							
Partial uniform Load							
Partial uniform Load			18.0 Ft				

Point Load	P(K)	x(Ft) from left	Dist to mid span	Dist to point of inflection
P1				
P2				
P3				
P4				
P5				
P6				
P7				
P8				
P9				
P10				



R= 0.648 k

M=

M= 2.92 Ft-K @ 9.00 Ft

2.92 Ft-K

2.91 Ft-K

R= 0.648 k

RESULT

M= 2.92 Ft-K x 12.00 = 35.0 in-K

$S_{req'd}$ = M_{max}/F_b= 35.0 / 30.36 = **1.15 in3**

Section Properties of Laterally Supported Beam

TYPE: HSS

SIZE: HSS6X4X3/16

WEIGHT	12.0 Lbs/ft			
AREA	3.28 in2			
DEPTH	6.00 in			
WIDTH	4.000 in			
t web				
t flange				
t	0.188 in			
Ix	16.40 in4			
Sx	5.46 in3	>	1.15 in3	OK
r x	2.23 in			
Iy	8.76 in4			
Sy	4.38 in3			
ry	1.63 in			

Deflection Check: As per FBC '02 L/360

$\Delta = 5/48 * M * L^2 / E * I$ = 0.36 in < 0.60 in

OK

Vibration Check:

$f = 3.5\sqrt{1/\Delta}$ = 3.5 $\sqrt{2.80}$ = 5.85 OK >5

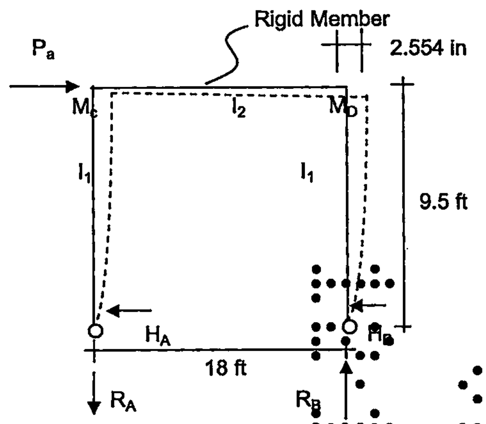
CANKAT ESSMAN
1900 SW 57th Ave. No: 1
Miami, 33155
(305)266-9777

PROJECT: SOHO BEACH HOUSE
CLIENT:
SUBJECT: Frame 2

JOB NO:
TITLE:

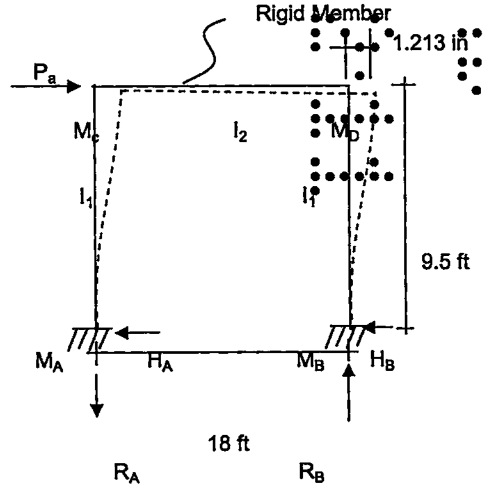
Designed: FO
Checked: MC

Lateral Force, P:	1.2 K	
Wall length,L:	18 Ft	
Wall height,h:	9.5 Ft	
Flexural mod. of Elasticity,E:	29000 Ksi	
I_2 horiz.	12 in^4	$k=I_2\,h / I_1\,L$
I_1 Vert.	2 in^4	k= 3.167



PIN SUPPORTS

H_A = 0.6 K
H_B= 0.6 K
M_C= -5.7 K-Ft
M_D= -5.7 K-Ft
Δ= M_C *L2/ 6EI = 2.554 in



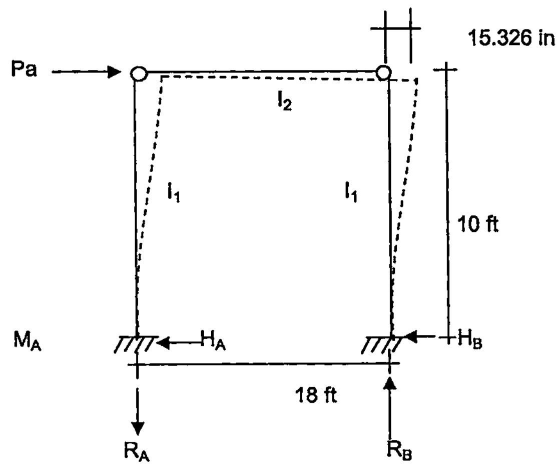
FIXED SUPPORTS

H_A = 0.6 K
H_B= 0.6 K
R_A= $-R_B$= -3PH/L * k/(6k+1)= -0.30 K
M_A= $-M_B$=-PH/2 *[(3K+1)/(6K+1)]= 2.99 K-Ft
M_C=H*h-M_A= 2.71 K-Ft
M_D=-H*h+M_B= -2.71 K-Ft
Δ= M_C *L2/ 6EI = 1.213 in

FIXED SUPPORTS AND PINNED AT TOP

H_A = 0.6 K
H_B= 0.6 K
R_A= $-R_B$= PH/L = 0.0 K

M_A=H*h= 5.70 K-Ft
M_B=H*h= 5.70 K-Ft
Δ= $P*L^3$/ EI = 15.326 in



cankat - essman inc.

PROJECT SOHO BEACH HOUSE COURTYARD PERGOLA
LOCATION MIAMI BEACH, FL
SUBJECT FRAMING

JOB NO 10017
DATE 9/21/2010
BY MC

$q_{wind} = 60$ psf (for rafter), 40 psf (for frame)

$q_{rafter} = 60 \times 0.25 = 15$ plf

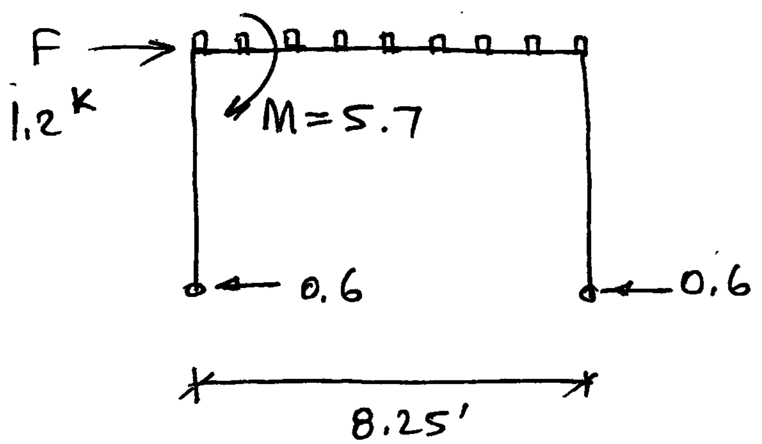
Trib. width = 8'

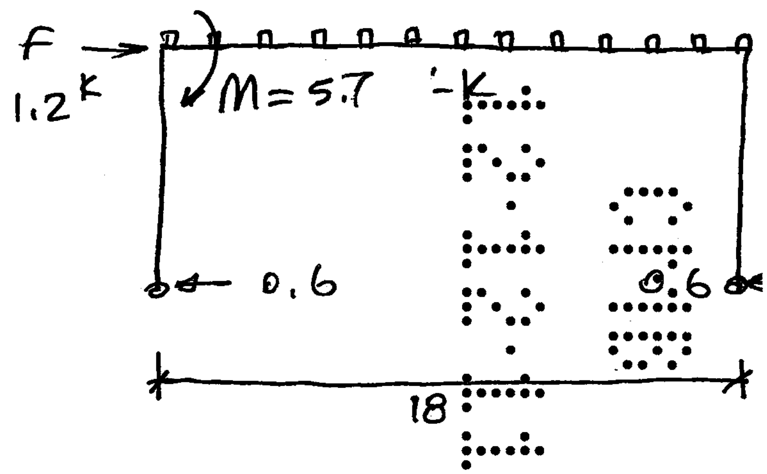
$F = 0.015 \times 8 \times 10$ pcs $= 1.2$

$q_{frame} = 40 \times 0.5 = 20$ plf

Trib width = 4'

$F = 0.015 \times 4 \times 20$ pcs $= 1.2$





	FRAME-1		FRAME-2	
	0.65 ↓	↓ 0.65	0.65 ↓	0.65 ↓
	1.45 ↑	↓ 1.45	0.56 ↑	0.56 ↓
Total	↑ 0.8 (Up)	↓ 2.05 (down)	0.09 ↓ (down)	1.21 ↓ (down)

USE FOR
EXP. BOLT
DESIGN

4"x4" PT. WOOD RAFTERS @ 12" o/c

SOUTHERN PINE #2 OR BETTER

$L_{max} = 11'-3"$

$A = 12.25$ sq in

$S_{x-x} = 7.146 \text{ in}^3$

$I_{x-x} = 12.51 \text{ in}^4$

* $F_b = 1{,}500$ psi

$F_t = 825$ psi

$F_v = 175$ psi

$F_{c\perp} = 565$ psi

$E = 1.6 \times 10^6$ psi

*OR FOR 4" WIDE CONSTRUCTION GRADE

$F_b = 1{,}100$ psi

$W_w = 60$ psf (CONSERVATIVE)

@ EA. RAFTER $W_w = 60 \times 3.5/12 = 17.5$ plf.

$M_w = 17.5 \times \overline{11.25}^2 / 8$

$M_w = 277$ ft-lb.

$f_{bx} = \dfrac{277 \times 12}{7.14} = 465 \text{ psi} < 1{,}100 \text{ psi}$ O.K

$V = 17.5 \times 11.25/2$

$V = 66$ lbs. ↑

$v = \dfrac{4}{3} \dfrac{66}{3.5 \times 3.5} = 7 \text{ psi} \ll 175 \text{ psi}$

MAX. CANTILEVER ~~END~~ CHECK $L = 2'-7"$

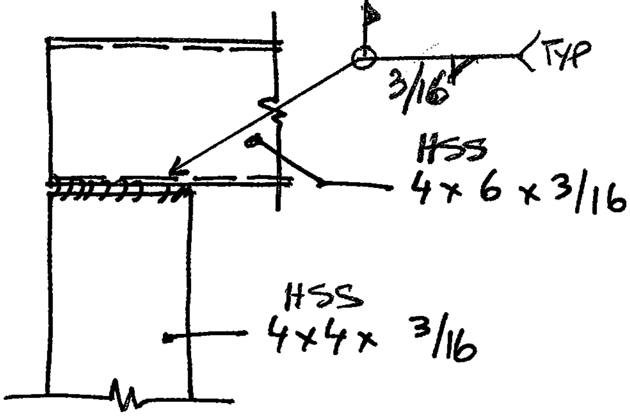
$M_{CANT} = 17.5 \times \overline{2.58}^2 / 2$

$M_{CANT} = 58 \text{ ft-lb} < 277 \text{ ft-lb}$ DO NOT GOVERN

NOTE: NO ADJUSTMENT FACTORS ARE USED SINCE STRESSES ARE VERY LOW ... OK ✓

CONNECTIONS

MOMENT CONNECTION @ TOP OF POST W/ BEAM.



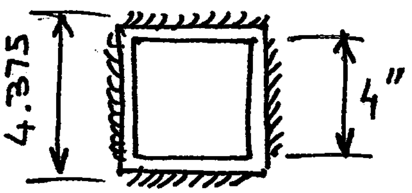
$M = 5.7$ '-k

$S = 4.209$ in^3 (*)

$f_w = \frac{5.7 \times 12}{4.209} = 16.25$

$F_{w_{allow}} = 21$ ksi $> f_w$

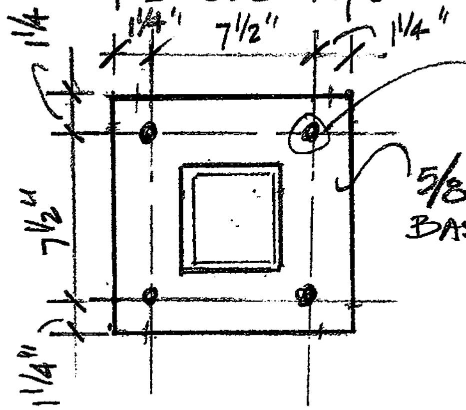
$F_{W_{ALLOW}} = 0.3 \times 70$ ksi $= 21$ ksi



BASE CONNECTION W/ EXPANSION BOLTS

FROM PAGE 11 MAXIMUM REACTIONS @ BASE

V = 0.6 kips
T = 0.8 kips



TOTAL (4) 5/8" HILTI-KWIK BOLT 3 W/ 4" EMBEDMENT

5/8 x 10" x 10" BASE PL.

4 BOLTED COLUMN BASE

$V' = 0.6/4 = 0.15$ kips/BOLT

$T = 0.8/4 = 0.20$ kips/BOLT

CONT'D ON PAGE 16

*) See NEXT PAGE FOR WELD SECTION MODULUS. (PAGE 15)

CANKAT ESSMAN 1900 SW 57th Ave. Miami, FL 33155 Tel: (305) 266-9777	Project: SOHO BEACH HOUSE Subject: 4X4 POST WELD	Engineer: FO Date: 2-Sep Checker: MC Date: 2-Sep	Page: 15 Project #

PROPERTIES OF SECTION DESCRIBED BY NODES COORDINATES

Units: in

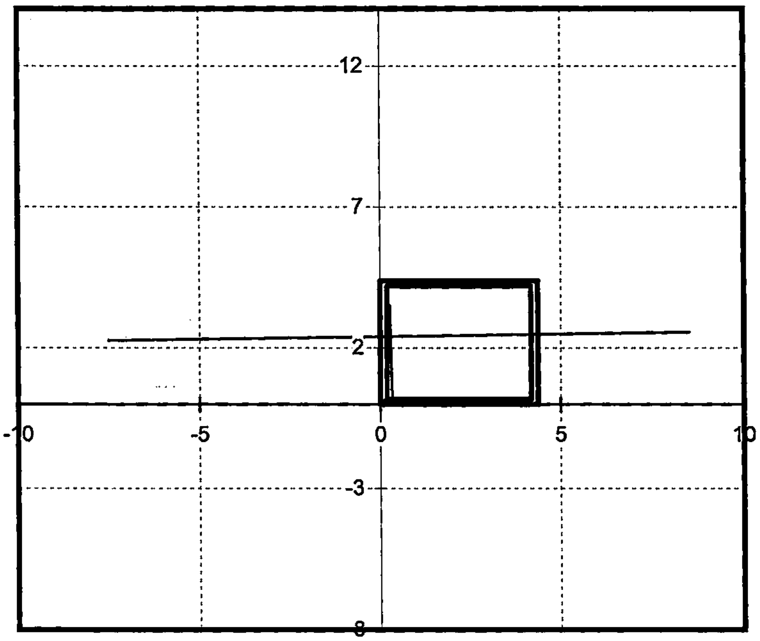
Nodes Coordinates	X	Y
1		
2		4.375
3	4.375	4.375
4	4.375	
5		
6	0.188	0.188
7	4.188	0.188
8	4.188	4.188
9	0.188	4.188
10	0.188	0.188
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		
26		
27		
28		
29		
30		
31		
32		
33		
34		

Section Properties About Centroidal Axis Parallel to Original Axis

ϕ =	45.00000 deg	X-dim =	4.3750 in
A =	3.14063 in^2	Y-dim =	4.3750 in
Xcg =	2.18495 in	Sx(top) =	4.1994 in^3
Ycg =	2.18495 in	Sx(bot) =	4.2092 in^3
Ixo =	9.19694 in^4	Sy(left) =	4.2092 in^3
Iyo =	9.19694 in^4	Sy(right)=	4.1994 in^3
Ixyo =	-0.00002 in^4		

Section Properties About Principal Axis

ϕ =	45.00000 deg	Sxp(top) =	2.9729 in^3
Ixp =	9.19696 in^4	Sxp(bot) =	2.9729 in^3
Iyp =	9.19691 in^4	Syp(left) =	2.9764 in^3
Ixyp =	0.00000 in^4	Syp(right)=	2.9694 in^3
J =	18.39387 in^4		



MIN. EDGE DIST { FOR TENSION $\Longrightarrow$ $1.75 \times 4 = 7''$
FOR SHEAR $\Longrightarrow$ $3 \times 4 = 12''$
BOLT SPACING (T & V) $\Longrightarrow$ $3 \times 4 = 12''$

ALL COLUMNS
WITH THE EXCEPTION OF COLUMNS NEXT TO TRENCH
HAVE MORE THAN 12" EDGE DISTANCE.

CHECK COLUMN BASE NEXT TO TRENCH.
EDGE DISTANCE = 5" (WORST CONDITION)
& $h_{nominal} = 4''$ (EMBEDMENT)

ADJUSTMENT FACTORS
TENSION / SHEAR (Spacing) $7^{1/2}$ $\Longrightarrow f = 0.96$

TENSION (Edge Distance) = 5" $\Longrightarrow f_{RN} = 0.9$

SHEAR (Edge Distance) = 5" $\Longrightarrow f_{RV} = 0.61$

TENSION CAP / PER BOLT (CONCRETE $f'_c = 3000$ psi)

$T = 2.87 \times 0.96 \times 0.9$
$T = 2.47 > 0.2$ kips ok.

SHEAR CAP / PER BOLT

$V = 4.89 \times \underset{\text{20\% REDUCTION}}{0.80} \times 0.96 \times 0.61$

$V = 2.29$ kips > 0.15 kips ok.

COMBINED TENSION & SHEAR

$$\left(\frac{0.2}{2.47}\right) + \left(\frac{0.15}{2.29}\right) = 0.14 \ll 1.0 \text{ ok.}$$

WOOD RAFTER TO STEEL TUBE CONNECTION
PULL OUT @ END CONNECTION (FROM PAGE 12) 66 lbs $\ll$ (0.25 x 4297 = 1074 lbs.)*

* SEE APPENDIX ITW BUILDEX 1/4-20 x 3 SELF DRILLING SCREWS

ALL OTHER CONNECTION, REACTIONS ARE LOW & BY INSPECTION. OK.

APPENDIX

B100002 PAGE - 18

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Hilti, Inc.
5400 S. 122nd East Avenue
Tulsa OK 74146

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

NOTICE: In addition to the requirements of this permit there may be additional restrictions applicable to this property that may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management districts, state agencies, or federal agencies. The City of Miami Beach assumes no responsibility for accuracy of or results from plans which are approved subject to compliance with all Federal, State and Local Laws, Rules, and Regulations.

This product is approved as described herein, and has been designed to comply with the Florida Building Code including the High Velocity Hurricane Zone.
DESCRIPTION: Kwik Bolt 3 Concrete Expansion Anchor.
APPROVAL DOCUMENT: Drawing No. **543-003-C**, Sheets 1 of 1, titled "Hilti Kwik Bolt 3 Mechanical Expansion Anchor for Concrete and Masonry Elements" dated 11/12/98 with last revision on 10/31/06, prepared by T. E. Lunn PE Engineering, signed and sealed by T. E. Lunn PE, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance (NOA) number and approval date by the Miami-Dade County Product Control Division.
MISSILE IMPACT RATING: None

LABELING: Each unit or box shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved or MDCPCA", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consist of this page 1, evidence page as well as approval document mentioned above.
The submitted documentation was reviewed by **Candido F. Font, P.E.**

PRODUCT CONTROL ACCEPTED

12/28/06

NOA No 06-0810.13
Expiration Date: December 28, 2011
Approval Date: December 28, 2006

NOTICE OF ACCEPTANCE: EVIDENCE PAGE

A **DRAWINGS:**

1. Drawings prepared by T.E. Lunn PE Engineering, titled "Hilti Kwik Bolt 3 Mechanical Expansion Anchor for Concrete and Masonry Elements"; Drawing No. 543-003-C, dated 11/12/98 with last revision on 10/31/06, sheet 1 of 1, signed and sealed by T.E. Lunn PE.

B **TEST:**

	Laboratory No.	Test Report.	Date.	Signature
1.	CEL 51792C	ASTM E488	05/22/06	L.W. Mattis PE.
2.	STTS 99-0513.01	ASTM G85 Annex 5	08/25/05	M. Mosbat PE.

C **CALCULATIONS:**

N/A

D **QUALITY ASSURANCE.**

1. Building Code Compliance Office.

E **MATERIAL CERTIFICATIONS:**

N/A

F **STATEMENTS:**

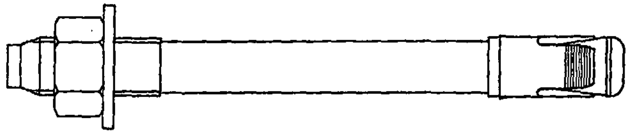
1. No interest letter issued by T.E. Lunn on 07/20/06, signed and sealed by T.E. Lunn, PE.
2. Code compliance letter issued by T.E. Lunn on 07/20/06 signed and sealed by T. E. Lunn, PE.

12/28/06

Candido F. Font, P.E.
Senior Product Control Examiner
NOA No 06-0810.13
Expiration Date: December 28, 2011
Approval Date: December 28, 2006

CMB

DESCRIPTION:
HILTI Kwik Bolt 3 is a stud type expansion anchor with a single piece wedge which expands in concrete and CMU when torque is applied. they are supplied with a nut and washer.



Anchor Diameter (in.)	Fracture Load (lb)
1/4	2,900
3/8	7,200
1/2	12,400
5/8	19,600
3/4	28,700

MATERIAL CHARACTERISTICS:
1" carbon steel studs, conform to AISI 11L41.
Nuts are carbon steel conforming to ASTM A563 Grade A and meet dimensional requirements of ANSI B18.2.2.
Washers are made from carbon steel conforming to ASTM F844 and meet dimensional requirements of ANSI 18.22.1 Type A plain.
All 3/8", 1/2", 5/8", and 3/4" wedges are made from carbon steel.
All 1/4" and 1" wedges are made from stainless steel.

DESIGN VALUES:

Dia. (in.)	Min. Embed. (in.)	3000 psi		CMU Grouted	
		Tension	Shear	Tension	Shear
1/4	2	623	458	–	–
3/8	2-1/2	1355	1460 [6]	780	1155
1/2	3-1/2	2185	2343 [7]	905	1723
5/8	4	2870	4885 [8]	1295	1723
3/4	4-3/4	3635	6173	–	–
1	6	4808	9200	–	–

GENERAL NOTES:
1) Design values in pounds are equal to ultimate load divided by 4.
2) Edge distance shall be 1.75 times the minimum embedment for tension and 3 times the minimum embedment for shear. Anchor center to center distance shall be 3 times the minimum embedment.
3) Anchor installation shall be made in accordance with Hilti's published installation instructions in the Product Technical Guide. Limit one anchor per cell.
4) Anchor installation in grouted concrete masonry units require a minimum 12" distance and 8" spacing. Limit one anchor per cell.
Avoid installation within 1-3/8" of any vertical mortar joint. See Figure 1.
5) Values shown are for shear plane through the anchor body.
6) When the shear plane is acting through the anchor threads, reduce the shear by 10%.
7) When the shear plane is acting through the anchor threads, reduce the shear by 6%
8) When the shear plane is acting through the anchor threads, reduce the shear by 20%.

Anchor Installation is Allowed in all Non-Shaded Areas

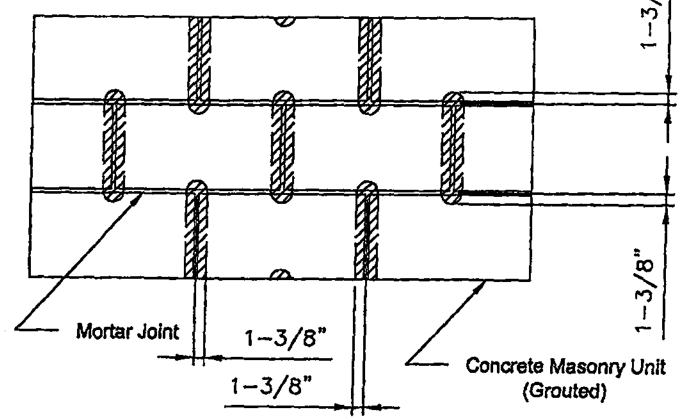


FIG. 1

T.E. LUNN, PE
ENGINEERING
23 THURSTON DR.
PALM BEACH GARDENS, FL 33418
OFC: (561 625-6455
FAX: (561)625-4883
EMAIL: telunn@gte.net
FLORIDA REG. # 26119

11.6.06

HILTI, INC.
5400 S 122nd E Ave
Tulsa, OK 74146

HILTI KWIK BOLT 3
MECHANICAL
EXPANSION ANCHOR
FOR CONCRETE
AND MANSONRY
ELEMENTS

REVISION DATE: 31-10-06
DATE: 11-12-98
DRAWING: 543-003-C
SCALE: 1" = 1"
SHEET NO. 1 OF 1

FOR OFFICE USE

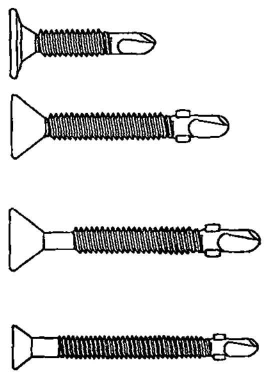
Approved as complying with the Florida Building Code
Date 12/28/06
NOA# 06-0810.13
Miami Dade Product Control Division
By

PAGE-21

TEKS® Self-Drilling Fasteners

Product Report No. 4080!

Selector Guide



Part Number	Description	Head Style	Drill Point	Drill & Tap Capacity	Wood Attachment Range	Applications
1077000	10-24 x 3/4"	Wafer	#3	.036-.175	1/4"-3/8"	• Plywood, mansard, fascia roofing, flooring to steel framing.
1079000	10-24 x 1"	Wafer	#3	.036-.175	1/4"-1/2"	
1082000	10-24 x 1-7/16"	PFH w/w*	#3	.060-.175	1/4"-3/4"	
1087000	10-16 x 1-13/16"	PFH	#3	.036-.175	1/4"-3/4"	
1092000	12-24 x 2-1/4"	PFH w/w*	#4	.125-.250	3/4"-1-3/8"	• Plywood, 2 X 4's header to steel framing.
1094000	12-24 x 2-3/4"	PFH w/w*	#4	.125-.250	3/4"-1-5/8"	
1096000	1/4-20 x 3"	PFH w/w*	#4	.125-.250	3/4"-2"	• 2 X 4's to steel framing.

*w/w with wings

Performance Data

PULLOUT VALUES (average lbs. ultimate)

Fastener Dia.	Fastener Pt.	26	24	22	20	18	16	14	12	3/16	1/4
#10	3				299	499	708	967	1474		
#12	4.5						683	923	1508	3865	4101
1/4	4						788	1116	1803	4297	

SHEET STEEL GAUGES

Gauge No.	12	14	16	18	20	22	24	2
Decimal Equivalent	.105"	.075"	.060"	.048"	.036"	.030"	.024"	.0

FASTENER VALUES

Fastener (dia-tpi)	Tensile (lbs. min.)	Shear (avg. lbs. ult.)	Torque (min. in. lbs.)
10-16 T/3	1936	1400	61
10-24 T/3	2702	1500	65
12-24 T/4	3165	2200	150
1/4-20	3860	2700	168

SHEAR VALUES (average lbs. ultimate)

Fastener Dia.	Fastener Pt.	Steel Gauge (lapped) 20	18	16	14	12	1/8	1/4
#10	3	728	1266	1540	1522			
#12	4.5					2641	2887	2897
1/4	4					2650	2820	

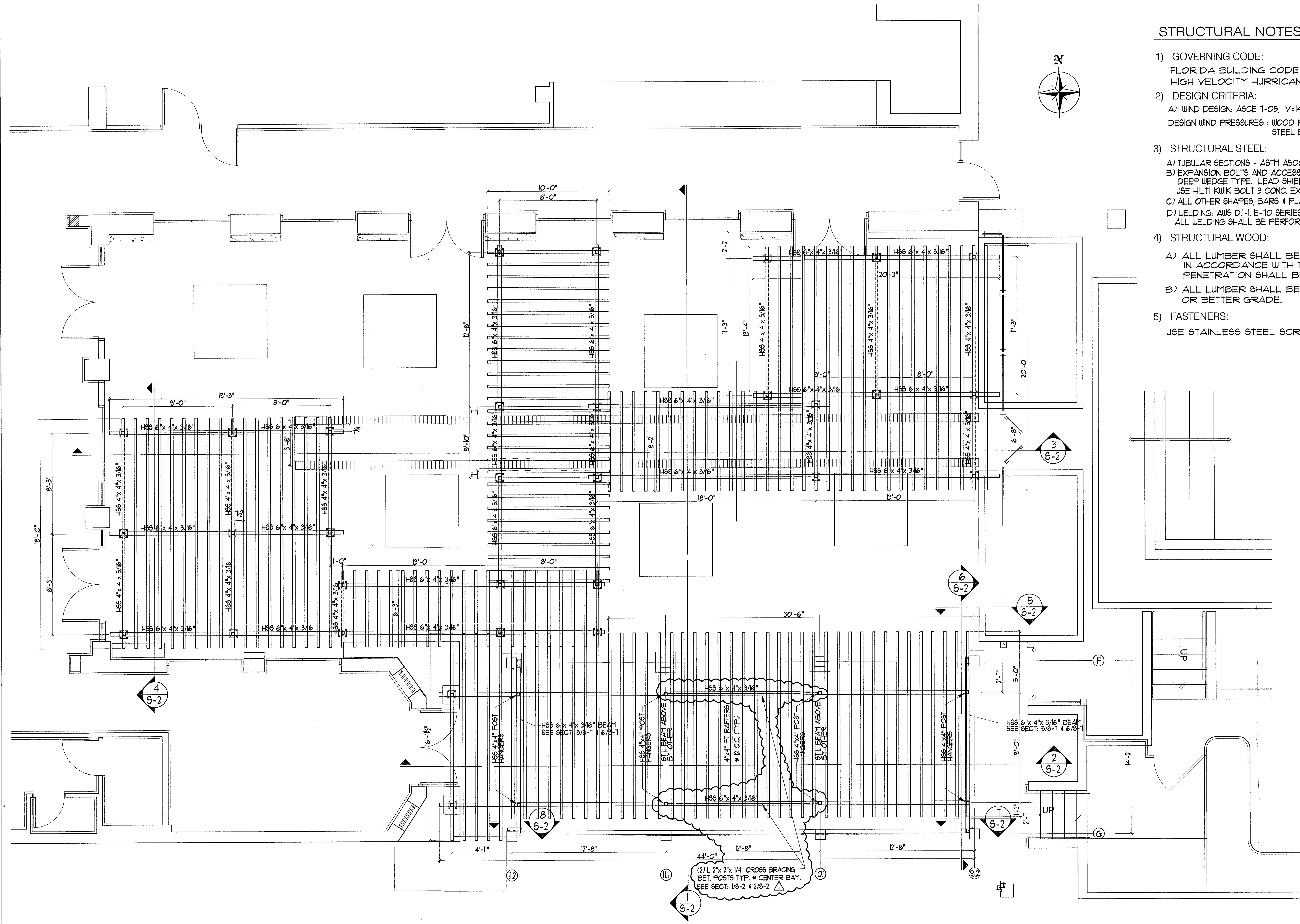
The values listed are ultimate averages achieved under laboratory conditions and ap
Buildex manufactured fasteners only. Appropriate safety factors should be applied to t
values for design purposes.

Installation Guidelines

- A standard screwgun with a depth sensitive nosepiece should be used to install Teks. For optimal fastener performance, the screwgun should be a minimum of 4 amps and have a RPM range of 0-2000.
- Adjust the screwgun nosepiece to properly seat the fastener.
- Worn or damaged bit tip should be replaced.
- The fastener is fully seated when the head is flush with the work surface.
- Overdriving may result in torsional failure of the faster or stripout of the substrate.
- The fastener must penetrate beyond the metal structu minimum of 3 pitches of thread.
- All #10 diameter "Winged" parts must be driven into a minimum of 16 GA steel thickness; all 1/4 and #12 diameter "Winged" parts must be driven into a minimu of 1/8" steel in order to break the wings consistently.

ITW Buildex

BUILDING IDEAS THAT WORK ™
1349 West Bryn Mawr Avenue
Itasca, Illinois 60143
630/595-3500 FAX: 630/595-3549



PLAN OF PERGOLA AT GROUND FLOOR

SC: 1/4" = 1'-0"

STRUCTURAL NOTES

1) GOVERNING CODE:

FLORIDA BUILDING CODE 2007 WITH 2009 AMENDMENTS / HIGH VELOCITY HURRICANE ZONE SECTIONS.

2) DESIGN CRITERIA:

A) WIND DESIGN: ASCE 7-05, V=146 MPH, EXP: C

DESIGN WIND PRESSURES : WOOD RAFTERS ZONE (2) & ZONE (3) +60 PSF (-) 57 PSF
STEEL BEAMS ZONE (2) & ZONE (3) +40 PSF (-) 37 PSF

3) STRUCTURAL STEEL:

A) TUBULAR SECTIONS - ASTM A500 (GRADE B)
B) EXPANSION BOLTS AND ACCESSORIES SHALL BE GALVANIZED STEEL DEEP WEDGE TYPE. LEAD SHIELDS ARE NOT ACCEPTABLE. USE HILTI KWIK BOLT 3 CONC. EXPANSION ANCHORS OR APPROVED EQUAL.
C) ALL OTHER SHAPES, BARS & PLATES - ASTM A 36
D) WELDING: AWS D.1-1, E-70 SERIES ELECTRODES ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS

4) STRUCTURAL WOOD:

A) ALL LUMBER SHALL BE SOUTHERN PINE PRESSURE TREATED IN ACCORDANCE WITH THE REQUIREMENTS OF AWPA STANDARD C-1. PENETRATION SHALL BE TO 85% OF SAPWOOD.

B) ALL LUMBER SHALL BE No 2 COMMON GRADE SOUTHERN PINE OR BETTER GRADE.

5) FASTENERS:

USE STAINLESS STEEL SCREWS UNLESS OTHERWISE NOTED ON SECTIONS.

NOTICE: In addition to the requirement of this permit there may be additional restrictions applicable to this property that may be found in the Public Records of this County and there may be additional permits required from other government entities such as water management districts, state agencies, or federal agencies. The City of Miami Beach assumes no responsibility for accuracy of or results from these plans which are approved subject to compliance with all Federal, State, and Local Laws, Rules, and Regulations.

PUBLIC WORKS
PLAN REVIEW NOTICE
Phone 305-673-7080 Fax 305-673-7028

THIS PLAN REVIEW CONSTITUTES APPROVAL FOR OBTAINING BUILDING PERMITS ONLY.

All construction and/or use of equipment in the right-of-way and/or easements, requires a separate Public Works Department permit prior to start of construction.

Permit Requirements: Proof of existing sidewalk/swale area conditions (pictures) and/or posting of sidewalk/roadway bonds (Public Works Inspection of the right-of-way will be required prior to final sign-off on the C.C. / C.O., or the release of bonds.)

Approved/Reviewed By: [signature] Date: 11-15-11

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:

BUILDING:	[signature] 12/6/14
ZONING:	[signature]
DRB/HPB:	
CONCURRENCY:	
PLUMBING:	
ELECTRICAL:	
MECHANICAL:	
FIRE PREVENTION:	[signature] 12/12/14
ENGINEERING:	
PUBLIC WORKS:	[signature] 11-15-11
STRUCTURAL:	[signature] 12/6/14
ELEVATOR:	

REVISIONS	BY
1 CROSS BRACING ADDED BLDG DEPT COMMENTS	JMF 10/18/2010

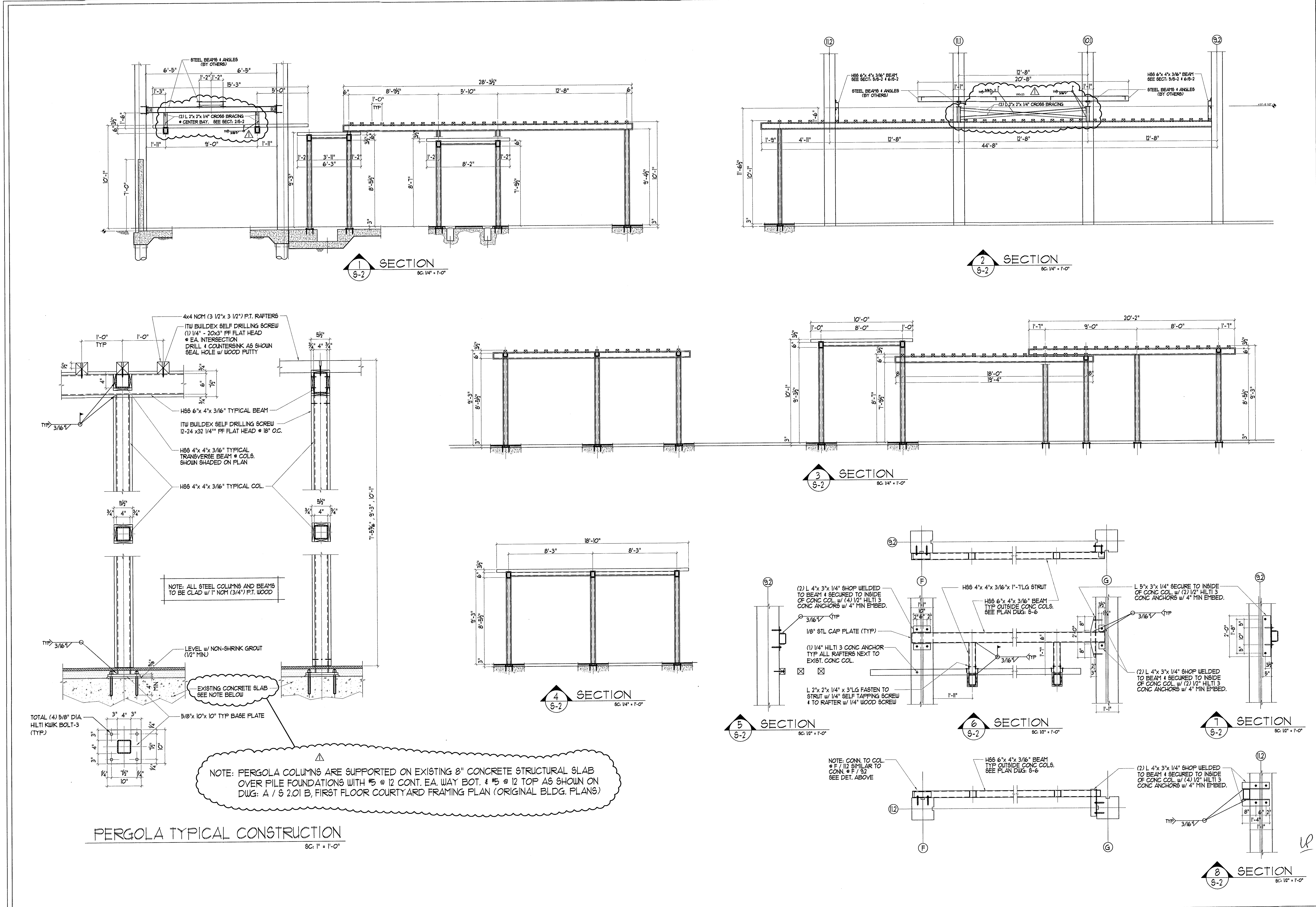
COURT YARD & LONG BAR PERGOLA
SOHO BEACH HOUSE
4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140

PLAN VIEW & GENERAL NOTES

Seal: [signature] 10/27/2010

cankat-essman,inc.
consulting engineers
STATE OF FLORIDA CA No: 00004419
1900 S.W. 57th AVE. SUITE 1, MIAMI, FL 33155
PHONE:(305) 266-9777 FAX:(305) 266-0584
LAWRENCE J. ESSMAN, P.E. FL. REG. NO. 6878
MUSTAFA CANKAT, P.E. FL. REG. NO. 18632

Date: SEPT 21, 2010
Scale: AS NOTED
Drawn: J.M.F.
Job: 10017
Dwg # S-1
Sheet 1 of 2



REVISIONS	BY
⚠ CROSS BRACING ADDED BLDG DEPT COMMENTS	JMF 10/18/2010

COURT YARD & LONG BAR PERGOLA

SOHO BEACH HOUSE

4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140

SECTIONS

Seal

10/27/2010

cankat-essman, inc.
consulting engineers
STATE OF FLORIDA CA No: 00004419
1900 S.W. 57th AVE. SUITE 1, MIAMI, FL. 33155
PHONE:(305) 266-9777 FAX:(305) 266-0584
LAWRENCE J. ESSMAN, P.E. FL. REG. NO. 6878
MUSTAFA CANKAT, P.E. FL. REG. NO. 18632

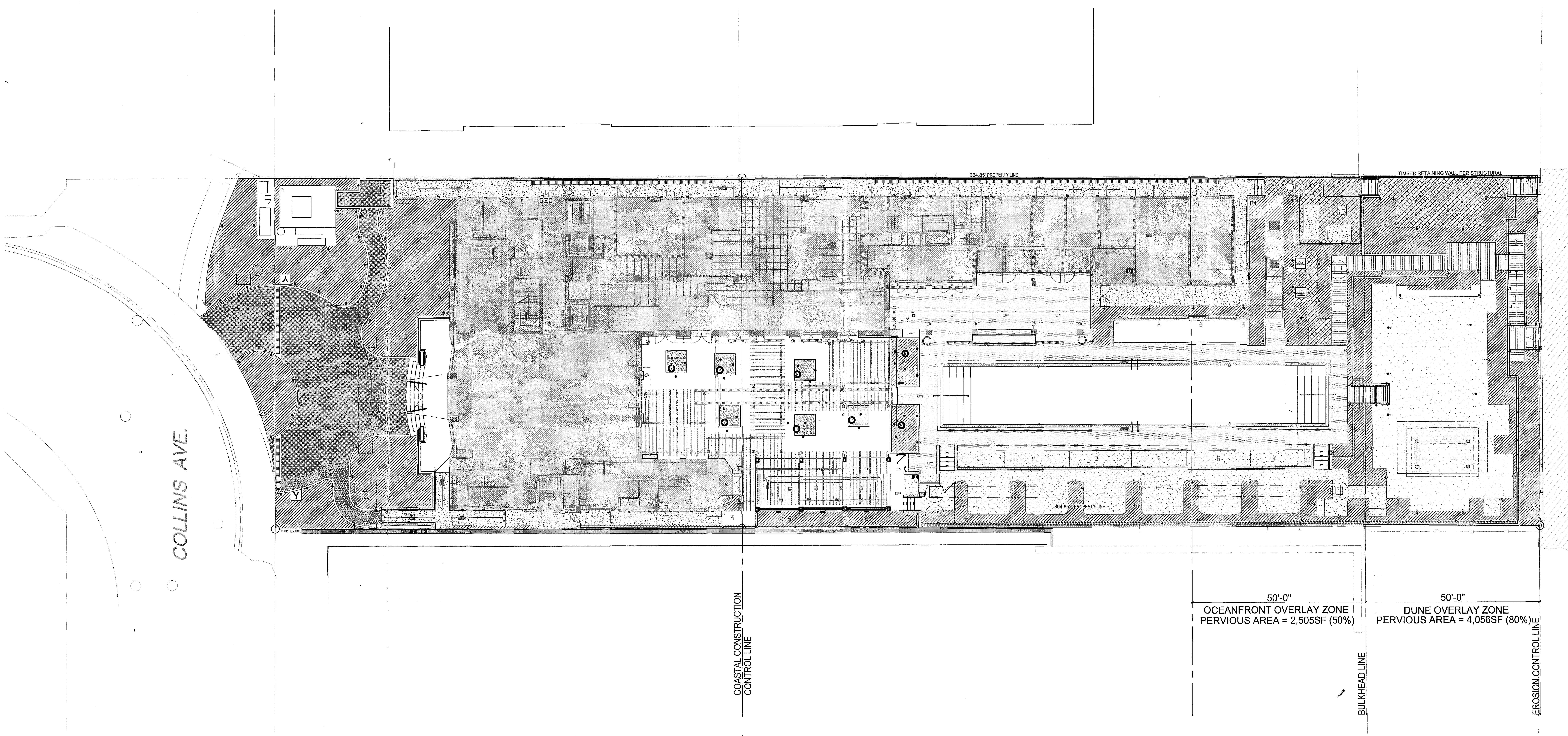
Date: SEPT 21, 2010

Scale: AS NOTED

Drawn: J.M.F.

Job: 10017

Dwg # S-2



PROPOSED SITE PLAN

NOT TO SCALE

ALLAN T. SHULMAN ARCHITECT, P.A
(AR 0012763)
100 NE 38TH STREET, SPACE #2 MIAMI, FL 33137
TEL: 305.438.0609 FAX: 305.438.0170

JUNGLES
LANDSCAPE ARCHITECT
RAYMOND JUNGLES INCORPORATED
Landscape Architect ASLA
242 SW 5th Street, Miami, FL 33130
PH (305)858.6777 FAX (305)856.0742
www.raymondjungles.com

REVISIONS

REVISIONS

10.19.10

N

SOHO BEACH HOUSE

4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140
JOB NO. 06023
RENOVATION AND ADDITION

L-1.00

10.19.10

PARTIAL HARDSCAPE PLAN

SCALE: 1/8"=1'-0"

ALLANT. SHULMAN ARCHITECT, P.A
(AR 0012763)
100 NE 38TH STREET, SPACE #2 MIAMI, FL 33137
TEL: 305.438.0609 FAX: 305.438.0170

JUNGLES
LANDSCAPE ARCHITECT
RAYMOND JUNGLES INCORPORATED
Landscape Architect ASLA
242 SW 5th Street, Miami, FL 33130
PH (305)858.6777 FAX (305)858.0742
www.raymondjungles.com

REVISIONS

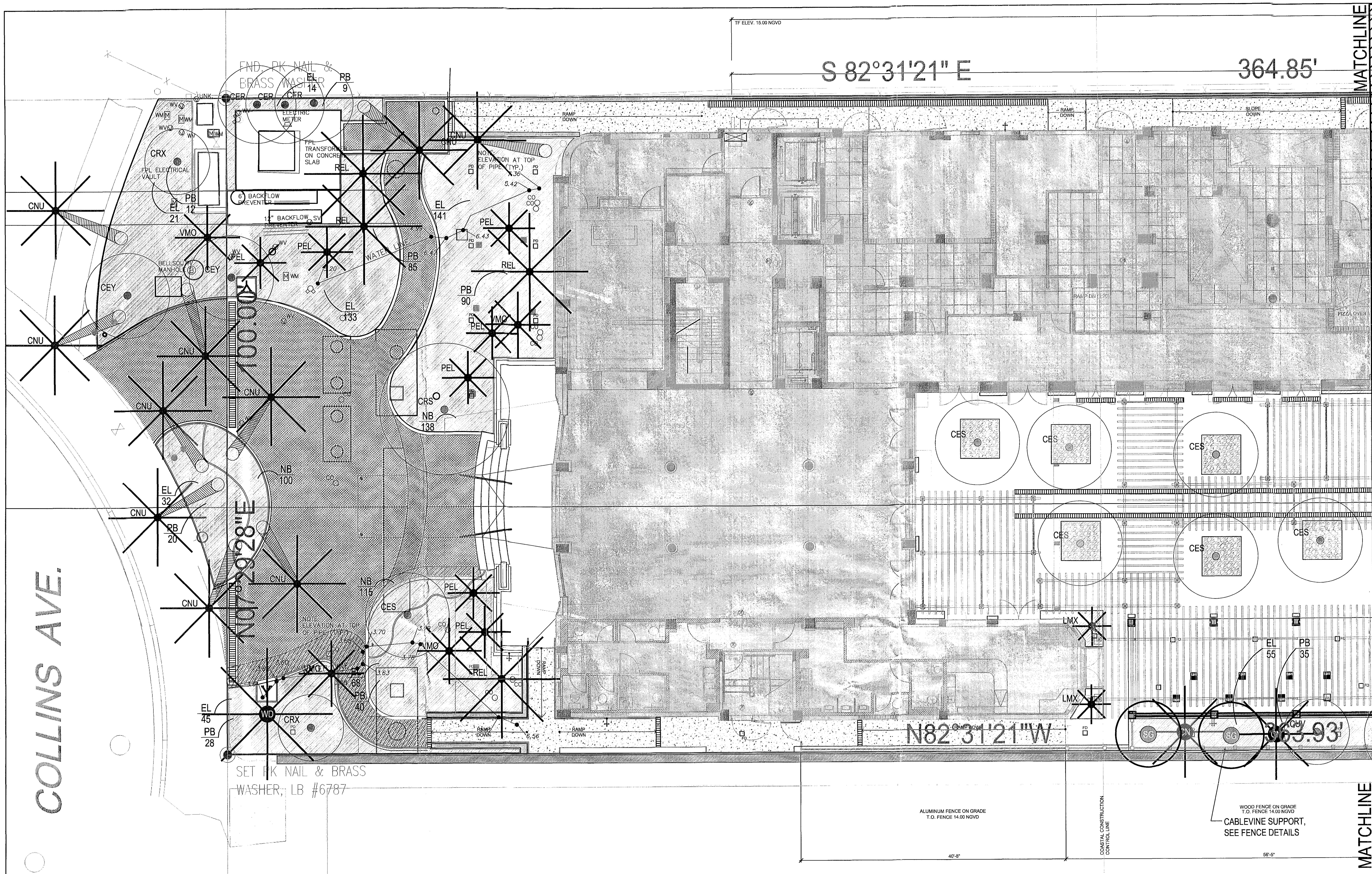
REVISIONS

10.19.10

N

SOHO BEACH HOUSE
4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140
JOB NO. 06023
RENOVATION AND ADDITION

L-1.01
10.19.10

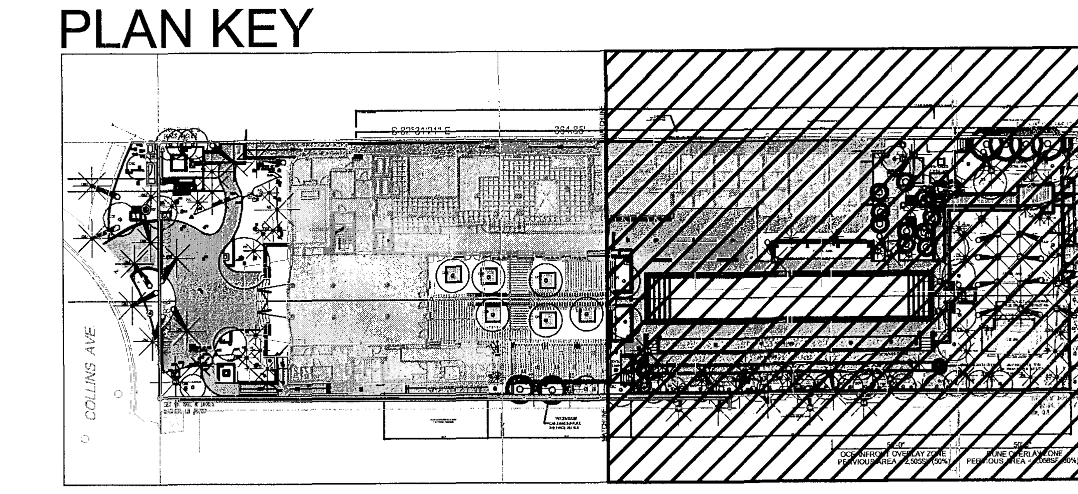


SOHO BEACH HOUSE PLANT LIST

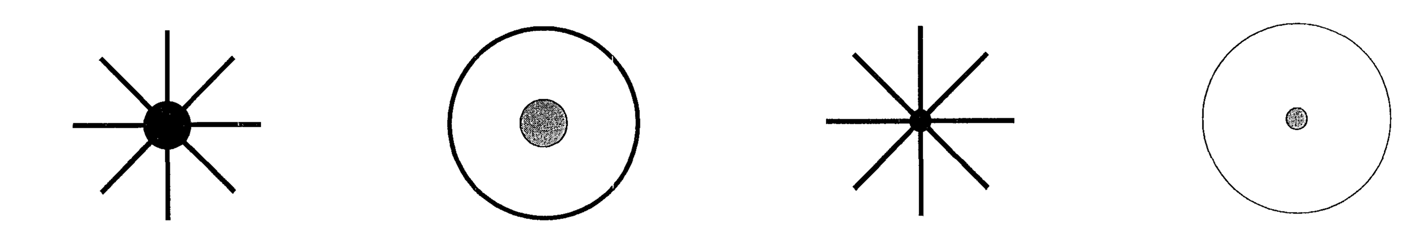
15-Jan-10

QTY	ABRV.	BOTANICAL NAME	COMMON NAME	SPECIFICATIONS
TREES				
1	CRO	*Clusia rosea*	Autograph Tree	36" DBH, 14' ht. x 14 sp.
1	CRS	*Clusia rosea*	Autograph Tree	16' - 18' ht. x 16' - 18' sp. Specimen (Landward of CCCL)
2	CRX	*Clusia rosea*	Autograph Tree	10' - 12' ht. x 10' sp., 12" DBH
25	CUV	*Coccoloba uvifera*	Sea Grape	12-14 ht. x 12' sp.
3	CER	*Conocarpus erectus*	Green Buttonwood	12' - 14' ht. x 8' sp.
14	CES	*Conocarpus erectus var. sericeus*	Silver Buttonwood	12' - 14' ht., x 10' -12' sp. Multi-trunk, 4 - 6" caliper, 7' CT
2	CEY	*Conocarpus erectus var. sericeus*	Silver Buttonwood	12' - 14' ht., x 10' -12' sp. Multi-trunk, 2 - 3" caliper
1	PAQ	*Pachira aquatica* (See NOTE 2)	Water Chestnut	10' -12' ht. x 6' x 8' sp.
PALMS				
10	CNU	*Coco nucifera*	Coconut Palm	
2	CMA	*Copernicia macroglossa*	Petticoat Palm	6' - 8' ht.
3	LLO	*Latania loddigesii*	Blue Latan Palm	14' ht., 8' GW
2	LMX	*Leucothrinax morrisii*	Sivler Thatch Palm	6' G.W.
7	PEL	*Ptychosperma elegans*	Solitaire Palm	4 @ 14' -16' ht. (singles) 4 @ 14' - 16' ht. multiple trunks
4	REL	*Roystonea eleta*	Royal Palm	Double trunk
11	SPA	*Sabal palmetto*	Cabbage Palm	14' Booted trunk, 18" DBH (Landward of CCCL)
20	TRX	*Thrinax radiata*	Green Thatch Palm	14' ht., single and multiple trunks
4	VMO	*Veitchia montgomeryana*	Montgomery Palm	18' - 24' ht., 6" - 8" DBH
NATIVE UNDERSTORY SHRUBS				
67	CC	*Capparis cynophallophora*	Jamaica Caper	6' - 8' ht. x 6' - 8' sp.
24	EF	*Eugenia foetida*	Spanish Stopper	10' - 12' ht. x 4' sp., full to base
ACCENTS				
13	AD	*Aechmea 'Dean'*	Same	
44	CG	*Clusia guttifera 'Nana'*	Same	3 gal.
13	CF	*Cordyline fruticosa 'Black Magic'*	Cordyline	4' - 6' ht. x 4' - 6' sp. Full
4	CA	*Crinum augustum 'Queen Emma'*	Same	6' ht. x 6' sp.
3	HC	*Heliconia caribaea* (See NOTE 2)	Same	8' - 10' ht. x 6' - 8' sp., Pale Yellow Flowers
1	HR	*Heliconia rostrata* (See NOTE 2)	Lobster Claw	6' - 8' ht. x 6' sp.
25	MD	*Monstera deliciosa* (See NOTE 1)	Mexican Breadfruit	36" x 36"
1	PS	*Philodendron*	TBD	Specimen Philodendron
85	PR	*Philodendron rojo 'Congo'*	Same	3 gal.
5	PW	*Philodendron 'Weeks Hybrid'* (See NOTE 2)	Same	36" x 36" (See NOTE 1)
31	TD	*Tripsacum dactyloides*	Fakahatchee Grass	36" x 36"
18	UP	*Uniola paniculata*	Sea Oats	3-gal. 24" o.c.
GROUNDCOVERS				
1060	AV	*Aloe vera*	Medicinal Aloe	6" OC
19	CI	*Chrysobalanus icaco*	Horizontal Cocoplum	24" x 24"
1110	EL	*Ernodea littoralis*	Golden Creeper	24" OC
60	GP	*Gaillardia pulchella*	Blanket Flower	12" OC
78	HL	*Hymenocallis latifolia*	Spider Lily	36" OC
353	NB	*Neoregelia 'Bossa Nova'* (See NOTE 1)	Same	12" OC
695	PB	*Philodendron 'Burle Marx'* (See NOTE 1)	Same	36" OC
584	PO	*Portulaca oleracea*	Sea Purslane	12" OC
790	PO	*Portulaca oleracea* (See NOTE 4)	Sea Purslane	6" OC
19	SW	*Sanseveria 'Whales Fin'*	Same	18" x 18"
80	ZP	*Zamia pumila*	Coontie	3 gal. 18" OC
VINES				
1	BA	*Bougainvillea 'Alabama Sunset'*	Same	7 gal. train to trunk of Coconut Palm
1	PV	*Pyrostegia venusta*	Flame Vine	
21	SV	*Solandra*	Chalice Vine	
18	IPC	*Ipomea pes-caprae*	Railroad Vine	
14	SF	*Stephanotis floribunda*	Bridal Wreath	
RELOCATES				
1	CR	*Clusia rosea*	Autograph Tree	Relocated from on-site
2	SG	*Coccoloba uvifera*	Sea Grape	Relocated from on-site
31	CN	*Cocos nucifera*	Coconut Palm	Relocated from on-site
4	CE	*Conocarpus erectus*	Green Buttonwood	Relocated from on-site
12	BR	*Jacquinia arborea*	Bracelet-Wood	Relocated from on-site (8' ht x 5' sp.)
1	WD	*Phoenix sylvestris*	Wild Date Palm	Relocated from on-site
10	TR	*Thrinax radiata*	Green Thatch Palm	Relocated from on-site
MISC.				
Zoysia Sod (3820 sqft.)				
Reinforced Zoysia Sod (815 sqft) - Install per manufactures instructions				
Fertilizer: Florikote Coated Magnesium, Florikote13-5-11, Florikote Coated Micronutrients. Supplier: Florikan (800) 322- 8666				
Accent plant allowance $5,000 W.P.A. total (W.P.A. = Wholesale Purchase Allowance) Species to be determined by Landscape Architect				
Coquina SV #2, 6" min. over well compacted sub-grade or existing beach sand. Supplier: Sutton Brick & Stone (954) 472- 7646				
Mulch - 2" deep Florimulch topped with 1" Pinus elliottii needles in planting areas				
Soil Ammendments: Add organic amendment per lab recommendation as topping layer on uncompcated fill, 8" typ., depth varies. See plant schedule. Mix organic amendment with existing backfill to depth indicated in plant schedule.				
NOTES				
NOTE 1	*Create an organic layer of 4-inches and 2-inches of a transitional zone 50% organic and 50% existing uncompacted fill*			
NOTE 2	*Plant to be located in field by Landscape Architect*			
NOTE 3	*Contractor to provide irrigation to all turf areas.*			
NOTE 4	*Quantity reflects Sea Purslane at 6" o.c. on tiki bar rooftop only*			

PLAN KEY



PLANT SYMBOL KEY



NOTE: FINAL LOCATION OF PLANTS TO BE DETERMINED IN THE FIELD BY LANDSCAPE ARCHITECT. CONTRACTOR TO STAKE TREE LOCATIONS AND CONFIRM SOIL DEPTH /UTILITY CONFLICTS. PRIOR TO DIGGING, CONTRACTOR TO SCHEDULE REVIEW OF FIELD CONDITIONS WITH LANDSCAPE ARCHITECT TO ADJUST TREE LOCATION TO RESOLVE ANY CONFLICTS

PARTIAL CANOPY PLANTING PLAN

SCALE: 1/8"=1'-0"

ALLANT. SHULMAN ARCHITECT, P.A
(AR 0012763)
100 NE 38TH STREET, SPACE #2 MIAMI, FL 33137
TEL: 305.438.0609 FAX: 305.438.0170

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LANDSCAPE ARCHITECT
RAYMOND JUNGLES INCORPORATED
Landscape Architect ASLA
242 SW 5th Street, Miami, FL 33130
PH (305)858.6777 FAX (305)858.0742
www.raymondjungles.com

REVISIONS

REVISIONS

10.19.10

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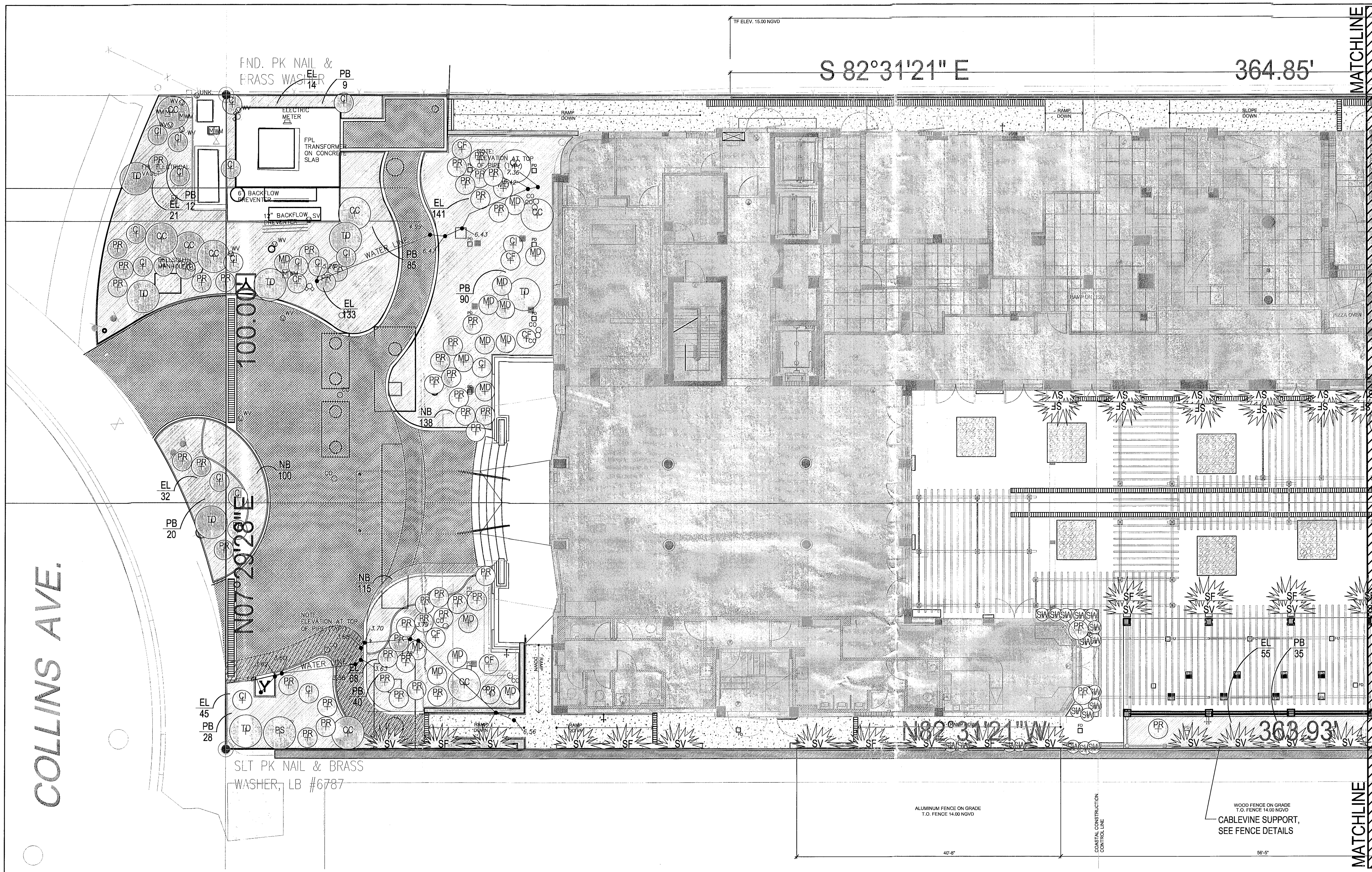
SOHO BEACH HOUSE
4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140
JOB NO. 06023
RENOVATION AND ADDITION

L-3.01
10.19.10

NOTE: PLANTING SCHEDULE, SEE SHEET L-3.01



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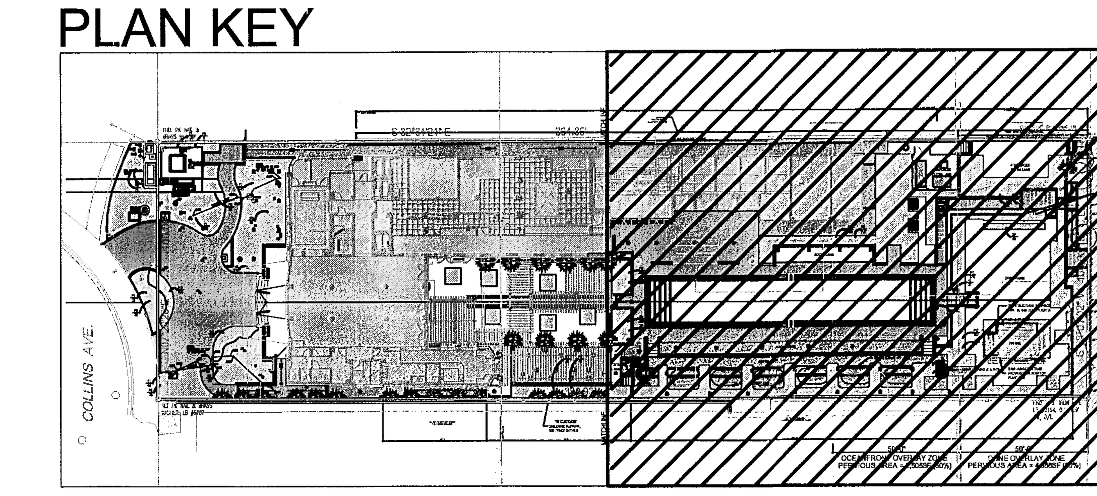
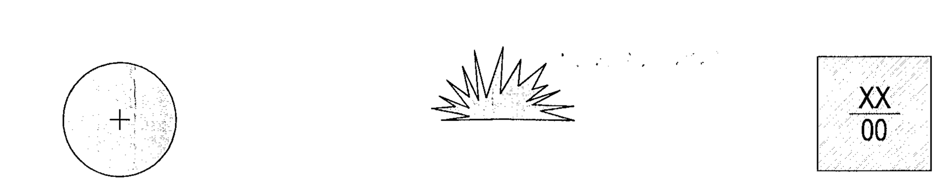


PLANT SYMBOL KEY



PROSPOSED SHRUB / ACCENT | PROPOSED VINE | PROPOSED GROUNDCOVERS

NOTE: FINAL LOCATION OF PLANTS TO BE DETERMINED IN THE FIELD BY LANDSCAPE ARCHITECT. CONTRACTOR TO STAKE TREE LOCATIONS AND CONFIRM SOIL DEPTH /UTILITY CONFLICTS. PRIOR TO DIGGING, CONTRACTOR TO SCHEDULE REVIEW OF FIELD CONDITIONS WITH LANDSCAPE ARCHITECT TO ADJUST TREE LOCATION TO RESOLVE ANY CONFLICTS

PARTIAL UNDERSTORY PLANTING PLAN

SCALE: 1/8"=1'-0"

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SOHO BEACH HOUSE
4385 COLLINS AVENUE, MIAMI BEACH, FL, 33140
JOB NO. 06023
RENOVATION AND ADDITION

L-3.03
10.19.10

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CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:

BUILDING: 12/6/11
ZONING: 12/6/11
DERM:
CONCURRENCY:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION: TArmstrong 12/12/11
ENGINEERING:
PUBLIC WORKS: 11.15.11
STRUCTURAL: Perez 12/6/11
ELEVATOR: