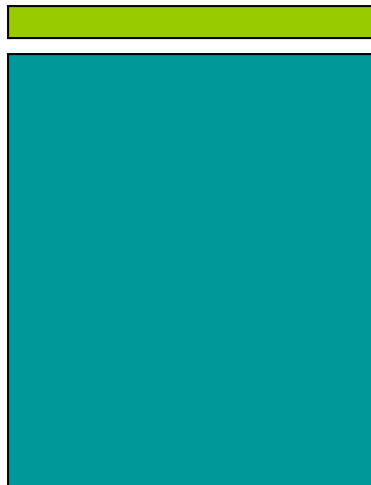


# Surfstyle

1155 Collins Avenue  
Miami Beach, Florida

traffic study



prepared for:  
CG Tides Village LLC

**Traf Tech**  
ENGINEERING, INC.

January 2014

January 29, 2014

Ms. Jeanine Rodriguez  
Project Manager  
CG Tides Village LLC  
3200 Stirling Road  
Hollywood, FL 33021

**Re: Surfstyle – Miami Beach, Florida  
Traffic Study**

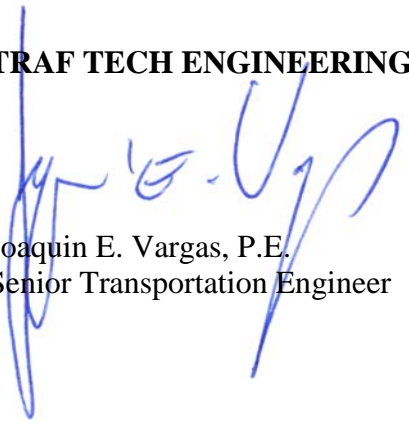
Dear Jeanine:

Traf Tech Engineering, Inc. is pleased to provide you with the results of the traffic study undertaken in connection with the proposed Surfstyle project located at 1155 Collins Avenue in the City of Miami Beach, Florida.

It has been a pleasure working with you on this project.

Sincerely,

**TRAF TECH ENGINEERING, INC.**



Joaquin E. Vargas, P.E.  
Senior Transportation Engineer

## TABLE OF CONTENTS

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<b>INTRODUCTION.....</b>	<b>1</b>
<b>INVENTORY .....</b>	<b>3</b>
Existing Land Use and Access .....	
Proposed Land Uses and Access .....	3
<b>EXISTING CONDITIONS.....</b>	<b>4</b>
Roadway System .....	4
Nearby Intersections.....	4
<b>TRAFFIC COUNTS .....</b>	<b>6</b>
<b>TRIP GENERATION.....</b>	<b>8</b>
<b>TRIP DISTRIBUTION AND TRAFFIC ASSIGNMENT.....</b>	<b>10</b>
<b>TRAFFIC ANALYSES.....</b>	<b>13</b>
Future Conditions Traffic Volumes.....	13
Level of Service (LOS) Analyses .....	14
<b>OTHER MODES OF TRANSPORTATION .....</b>	<b>19</b>
Miami-Dade Transit .....	19
Airport Shuttle Service.....	19
DecoBike.....	20
Pedestrian Network .....	20
<b>PARKING AND QUEUING ANALYSIS.....</b>	<b>21</b>
<b>SUMMARY &amp; CONCLUSIONS.....</b>	<b>23</b>

### LIST OF FIGURES

FIGURE 1 – Project Location Map.....	2
FIGURE 2 – Existing Lane Geometry .....	5
FIGURE 3 – Existing Traffic Counts – Peak Hour.....	7
FIGURE 4 – Project Trip Distribution .....	11
FIGURE 5 – Project Traffic Assignment .....	12
FIGURE 6 – Projected Current Year (2014) Traffic Volumes.....	15
FIGURE 7 – Background Traffic Conditions (Year 2015) .....	16
FIGURE 8 – Total Traffic with Project (Year 2015) .....	17

### LIST OF TABLES

TABLE 1 – Trip Generation Summary .....	9
TABLE 2 – Intersection Levels of Service.....	18

### Appendices

## INTRODUCTION

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Surfstyle is a proposed development to be located on the southeast corner of the intersection of Collins Avenue (State Road A1A) and 12<sup>th</sup> Street in the City of Miami Beach, Miami-Dade County, Florida. More specifically, the subject site is located at 1155 Collins Avenue. This project involves the redevelopment of the existing surface parking lot with retail space and a parking garage. The location of the project site is illustrated in Figure 1 on the following page.

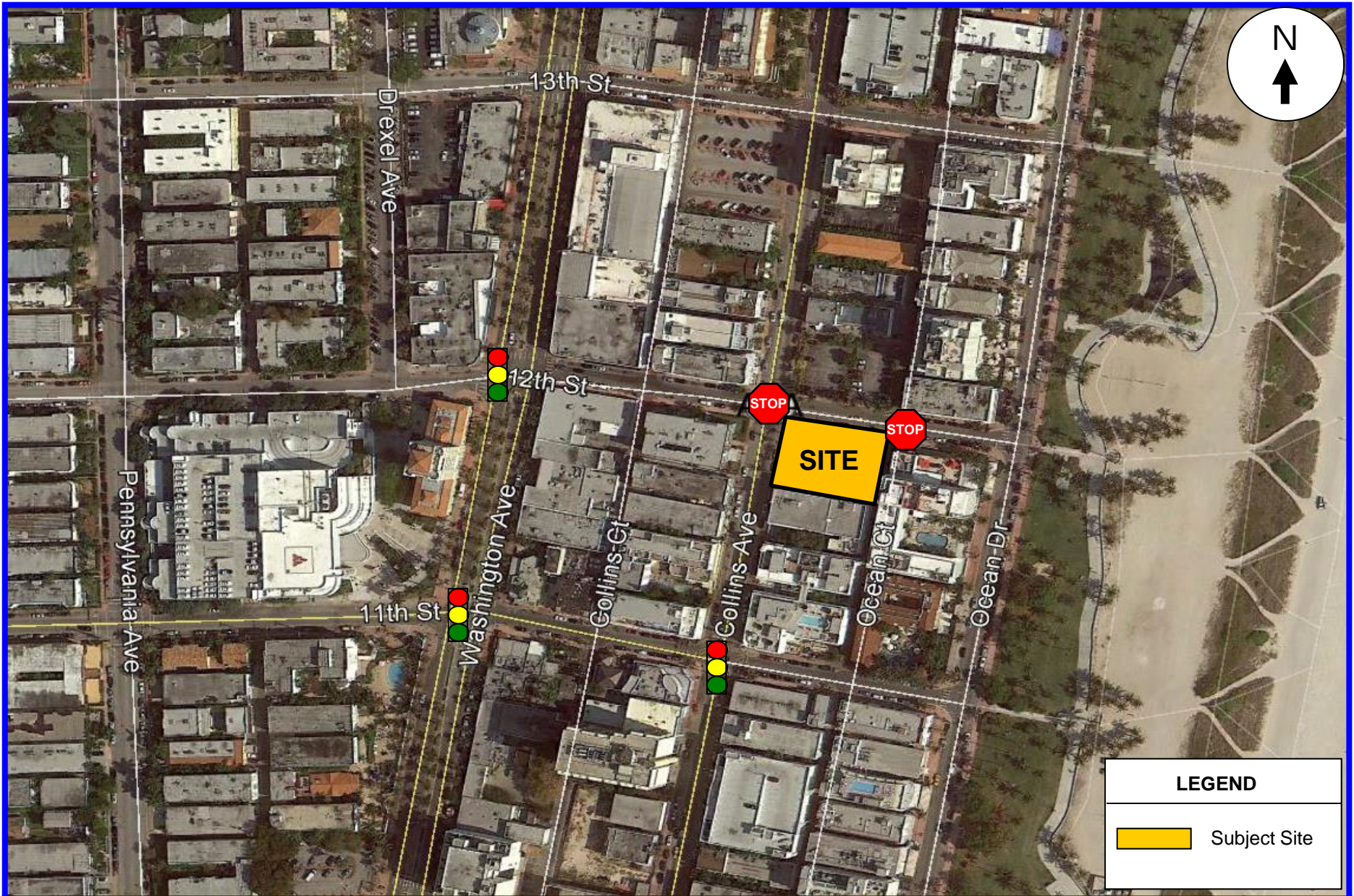
Traf Tech Engineering, Inc. has been retained by CG Tides Village LLC to conduct a traffic study<sup>1</sup> in connection with the redevelopment of this property. This study addresses trip generation and the traffic impacts created by the proposed project on the nearby transportation network. This study is divided into nine (9) sections, as listed below:

1. Inventory
2. Existing Conditions
3. Traffic Counts
4. Trip Generation
5. Trip Distribution and Traffic Assignment
6. Traffic Analyses
7. Other Modes of Transportation
8. Parking and Queuing Analysis
9. Summary & Conclusions

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<sup>1</sup> The methodology employed for this traffic impact study was discussed and agreed upon with City of Miami Beach staff on Friday, December 20, 2013. A summary of this meeting is presented in Appendix A.





## INVENTORY

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### **Existing Land Use and Access**

As mentioned previously, the subject site currently contains a 45-space surface parking lot. Access to this parking lot is presently provided by two (2) driveways located on 12<sup>th</sup> Street.

### **Proposed Land Uses and Access**

The proposed project will include 10,663 square feet of specialty retail space and a 148-space parking garage. Vehicular ingress to this project will be provided by a full access driveway to be located on 12<sup>th</sup> Street. Vehicular egress from the site will be provided via a left-turn only driveway onto Ocean Court. The proposed project is anticipated to be built and occupied by mid-2015. Appendix B contains the proposed site plan for the Surfstyle project.

## EXISTING CONDITIONS

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This section of the report addresses the transportation system located in the vicinity of the project site.

### **Roadway System**

The roadway system located near the site includes Collins Avenue (State Road A1A), Washington Avenue, Ocean Court, 11<sup>th</sup> Street, and 12<sup>th</sup> Street. Collins Avenue is a two-lane undivided, north-south state-maintained principal arterial facility with a posted speed limit of 30 miles per hour (mph). There is on-street parking along both sides of this section of Collins Avenue. Washington Avenue is a four-lane divided, north-south local principal arterial roadway with on-street parking on both sides of the roadway. Ocean Court is a one-lane northbound local roadway. 11<sup>th</sup> Street and 12<sup>th</sup> Street are city-maintained two-lane local roadways with metered on-street parking throughout.

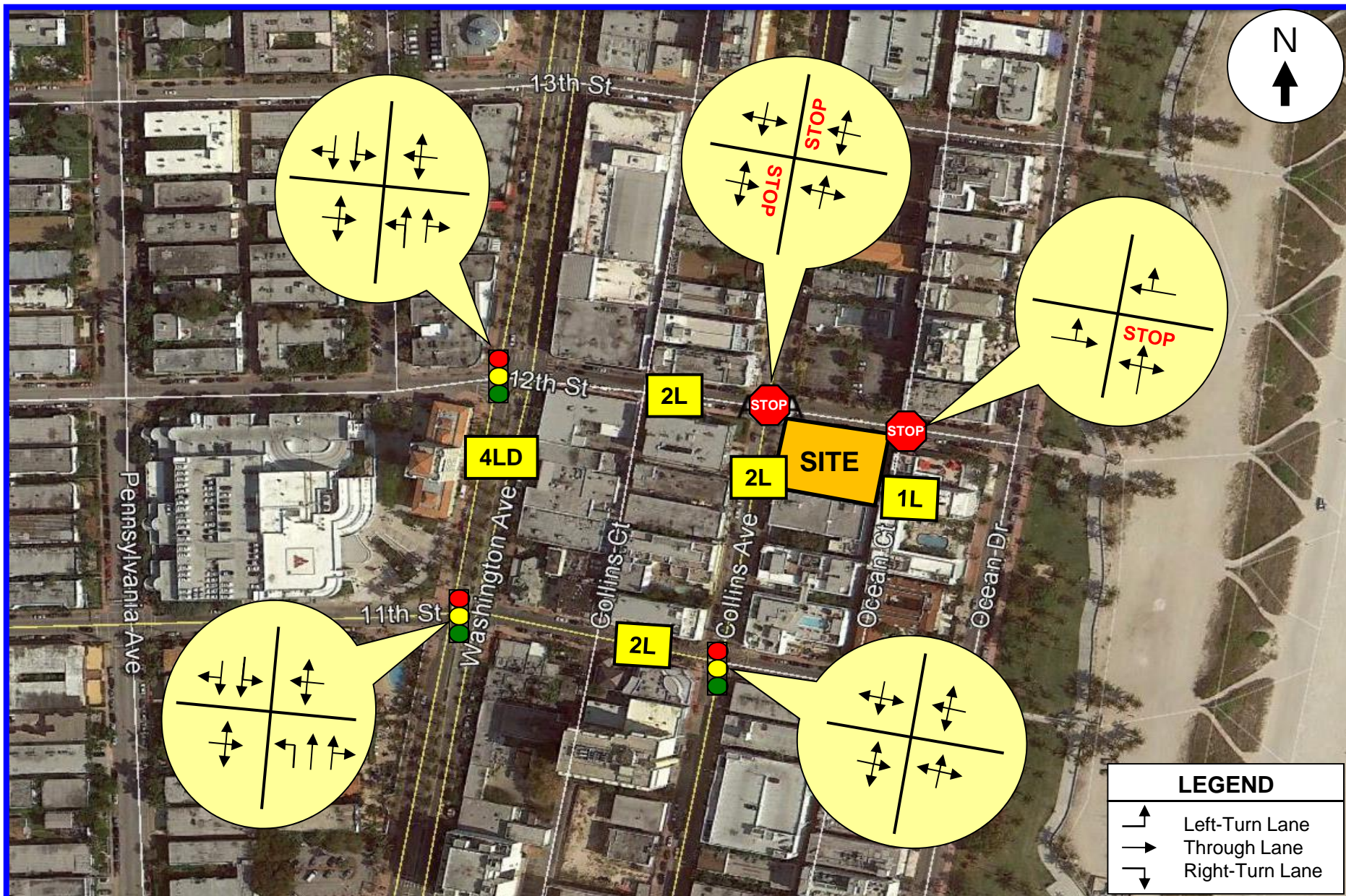
### **Nearby Intersections**

With the assistance of City of Miami Beach staff, five (5) nearby intersections were identified as the locations that will be impacted most by the proposed redevelopment project. These intersections are:

- Collins Avenue and 11<sup>th</sup> Street (signalized)
- Collins Avenue and 12<sup>th</sup> Street (stop-controlled)
- Washington Avenue and 11<sup>th</sup> Street (signalized)
- Washington Avenue and 12<sup>th</sup> Street (signalized)
- Ocean Court and 12<sup>th</sup> Street (stop-controlled)

Figure 2 shows the existing lane geometry of the five (5) intersections selected for analysis purposes. The number of lanes on the street system surrounding the project site is also depicted in this figure.





## TRAFFIC COUNTS

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As a result of the presently ongoing roadway construction along Collins Avenue, traffic counts performed for a previous analysis at this site were utilized for this analysis. Intersection turning movement counts at the following five (5) study intersections were obtained:

- Collins Avenue and 11<sup>th</sup> Street (signalized)
- Collins Avenue and 12<sup>th</sup> Street (stop-controlled)
- Washington Avenue and 11<sup>th</sup> Street (signalized)
- Washington Avenue and 12<sup>th</sup> Street (signalized)
- Ocean Court and 12<sup>th</sup> Street (stop-controlled)

Based upon a peak period analysis performed for this study area between February 16, 2012 (Thursday) and February 19, 2012 (Sunday), the peak period was determined to be on Saturdays between 8:30 PM and 10:30 PM. As a result, turning movement counts were performed on Saturday, March 3, 2012 during these peak hours. Figure 3 summarizes the results of the intersection turning movement counts undertaken during this time period. Appendix C contains the intersection turning movement counts, as collected in the field.

The signalized intersections within the project study area are maintained and operated by Miami-Dade County's Traffic Signals and Signs Division. The current signal timing plans for these intersections were obtained from the County and are included in Appendix D.





## TRIP GENERATION

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The trip generation for the Surfstyle project was based upon information contained in the Institute of Transportation Engineer's (ITE) *Trip Generation Manual (9<sup>th</sup> Edition)*. According to the subject ITE manual, the most appropriate land use category for the proposed development is: Land Use 826 – Specialty Retail Center. The trip generation rates used to determine the vehicle trips associated with the Surfstyle project are presented below.

### **ITE Land Use 826 – Specialty Retail Center**

#### Saturday Trip Generation

$$T = 42.04 (X)$$

Where  $T$  = number of Saturday trips and  $X$  = 1,000 SF of gross leasable area

#### PM Peak Hour of Generator

$$T = 5.02 (X) \text{ (56\% inbound and 44\% outbound)}^1$$

Where  $T$  = number of peak hour trips and  $X$  = 1,000 SF of gross leasable area

Given the urban location of the Surfstyle project, a multimodal reduction of 20% was applied to reflect reduced vehicle usage and greater reliance on walking, bicycling and public transportation. The proximity and operating characteristics of these alternative modes are described in subsequent sections of this report.

With 31 parking spaces required for the proposed retail component of this development, 117 parking spaces within the accompanying garage will be available for use of other nearby developments. According to the Urban Land Institute (ULI), public parking garages typically exhibit a turnover rate of approximately 20% during the peak hour. In order to conduct a conservative analysis, a 30% turnover rate was applied to the proposed Surfstyle parking garage. For the remaining 117 parking spaces, this translates to 35 trips during the PM peak hour. In addition, a trip credit, based upon field counts, was applied for the traffic associated with the existing surface parking lot.

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<sup>1</sup> In the absence of Saturday - PM peak hour trip generation data for this land use, the data for the PM peak of the generator on a weekday was utilized for this analysis.

Table 1 below summarizes the gross and net new vehicle trips associated with the proposed redevelopment of the Surfstyle site.

<b>TABLE 1</b> <b>Trip Generation Summary</b> <b>Surfstyle – Miami Beach, FL</b>					
<b>Land Use</b>	<b>Size</b>	<b>Daily Trips</b>	<b>PM Peak Hour Trips</b>		
			<b>Inbound</b>	<b>Outbound</b>	<b>Total</b>
Specialty Retail	10,663 SF	448	30	24	54
Multimodal Reduction	-20%	-90	-6	-5	-11
<b>Sub Total</b>	-	<b>358</b>	<b>24</b>	<b>19</b>	<b>43</b>
Parking Garage	117 Spaces	-	18	17	35
<b>Sub Total</b>	-	-	<b>42</b>	<b>36</b>	<b>78</b>
Existing Trips	-	-	-18	-16	-34
<b>Net New Trips</b>	-	-	<b>24</b>	<b>20</b>	<b>44</b>

Source: ITE Trip Generation Manual (9<sup>th</sup> Edition) and Traf Tech Engineering, Inc. (January 2014).

As indicated in Table 1 above, the net new external trips anticipated to be generated by the proposed Surfstyle project consists of approximately 44 trips during the PM peak hour of the generator (24 inbound and 20 outbound trips).



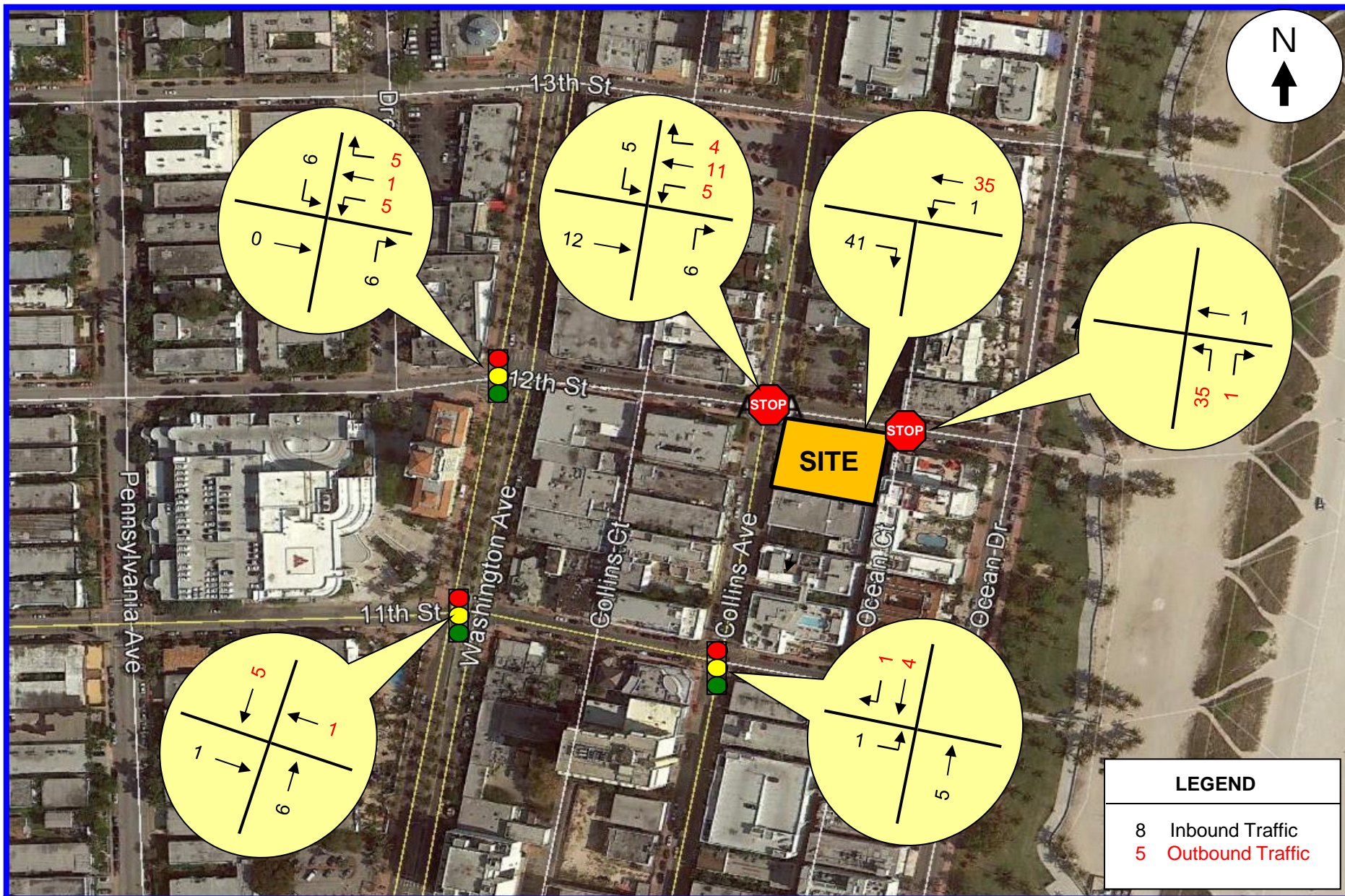
## **TRIP DISTRIBUTION AND TRAFFIC ASSIGNMENT**

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Consistent with the previous analysis conducted for this site, the trip distribution and traffic assignment for the project was based upon turning movement counts collected at the nearby study intersections. Figure 4 presents the project's trip distribution percentages and Figure 5 presents the project's PM peak hour traffic assignment.







## TRAFFIC ANALYSES

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This section of the study is divided into two (2) parts. The first part consists of developing the future conditions traffic volumes for the study area. The second part includes level-of-service analyses for existing and future conditions.

### **Future Conditions Traffic Volumes**

Two (2) sets of future traffic volumes were developed. The first set includes project build-out conditions without the proposed Surfstyle project and the second set adds the net new vehicle trips anticipated to be generated by the subject project.

**Peak Season Conversion Factor** – In order to develop year 2015 background traffic volumes, two separate analyses were undertaken. The first analysis converts the existing peak hour traffic counts collected in the field during the month of March to average peak season conditions. Based on FDOT's Peak Season Factor Category report, an adjustment factor of 0.98 is required to convert traffic counts collected in early March to average peak season conditions (please see Appendix E).

**Background Traffic Growth** – The second analysis includes a growth factor to project 2012 peak season traffic volumes to the 2015 build-out year. Based on traffic growth data published by the FDOT for the traffic count stations located near the project (Site #5170 – Collins Avenue north of 21<sup>st</sup> Street has exhibited a slightly negative change in traffic volumes over the past 5 years while Site #5159 – Collins Avenue 200 feet north of 5<sup>th</sup> Street has exhibited a 3.15% annual growth rate over the past 5 years) and upon long range traffic data contained in the latest SERPM model (annual growth rate of 0.83% per year for the period 2005 to 2035), an annual growth rate of 1.0% was applied for the purposes of this study. Background data for this analysis is included in Appendix E.

**Committed Traffic** – The traffic associated with the approved but not yet constructed project to be located at 1201 Collins Avenue has also been accounted for in this project. Excerpts from the traffic study for this project are included in Appendix F.

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**Planned Roadway Improvements** – The ongoing construction on Collins Avenue is associated with a Florida Department of Transportation (FDOT) Resurfacing, Restoration, and Rehabilitation (3R) project. Planned improvements within the project study area include:

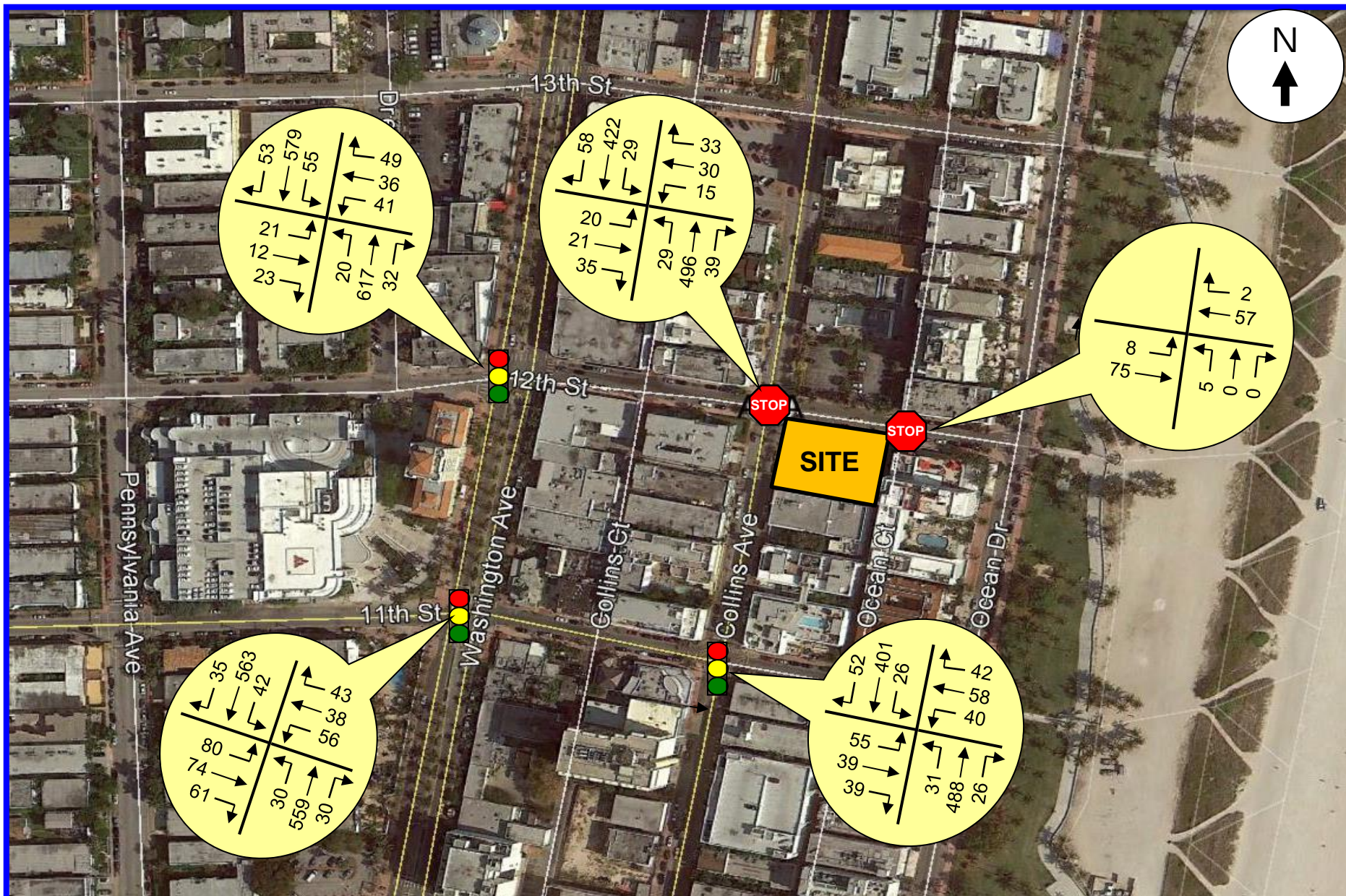
- Northbound and southbound left turn lanes at Collins Avenue and 11<sup>th</sup> Street
- Northbound and southbound left turn lanes at Collins Avenue and 12<sup>th</sup> Street
- Signalization at the intersection of Collins Avenue and 12<sup>th</sup> Street

**Future Traffic** – The new trips generated by the Surfstyle project (refer to Table 1 and Figure 5) were added to the 2015 background traffic in order to develop future total traffic conditions. The future traffic projections for the study intersections (peak season adjustments, growth rates, committed traffic, and project traffic) are presented in tabular format in Appendix G. Figure 6 presents the projected current year (2014) volumes for each of the study intersections. Figures 7 and 8 present the year 2015 future traffic volumes for the study area. Figure 7 includes background traffic only (without the proposed project) and Figure 8 includes the additional traffic anticipated to be generated by the Surfstyle project.

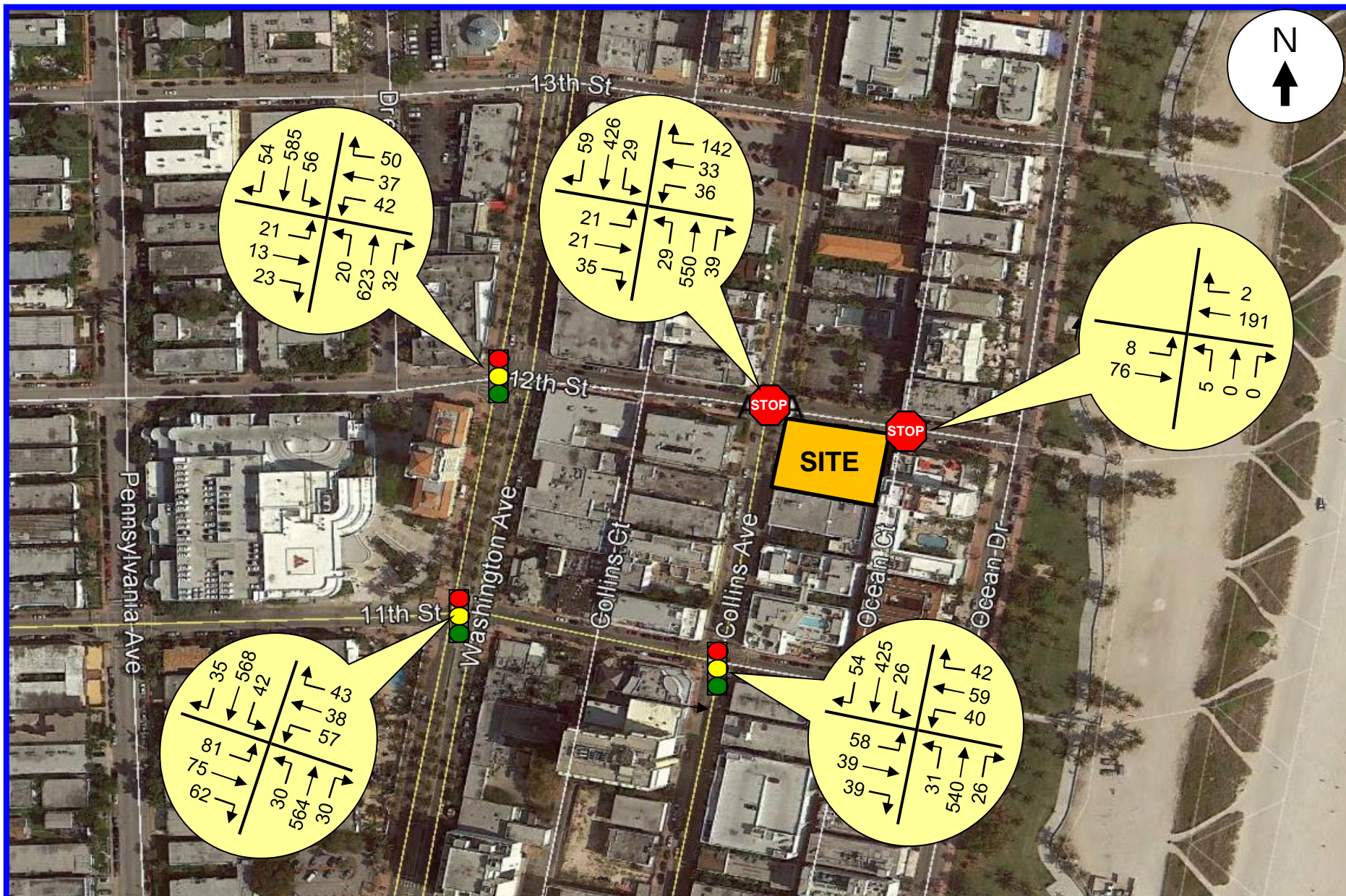
### **Level of Service (LOS) Analyses**

Intersection capacity/level of service (LOS) analyses were conducted for the five (5) study intersections. These analyses were undertaken following the capacity / level of service procedures outlined in the Highway Capacity Manual (HCM) using the SYNCHRO software. The results of the capacity analyses are summarized in Table 2. As indicated in Table 2, all study intersections are currently operating adequately and will continue to operate at a very good level of service in the year 2015 with the proposed project in place. The SYNCHRO printouts of the intersection capacity analyses are contained in Appendix H.

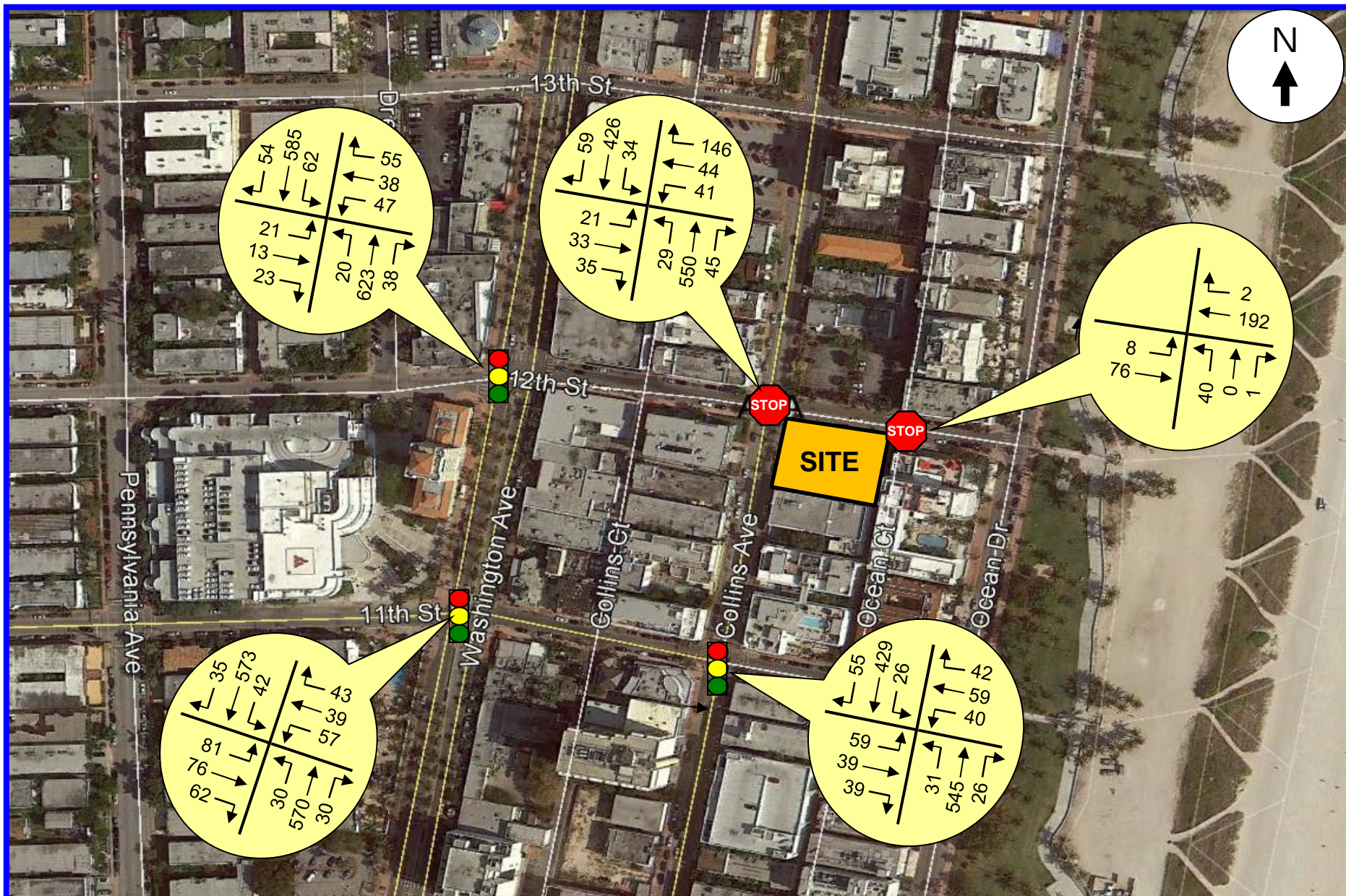














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<b>TABLE 2</b> <b>Intersection Levels of Service</b> <b>Surfstyle – Miami Beach, FL</b>			
<b>Intersection</b>	<b>2014 Existing</b>	<b>Future Traffic Conditions</b>	
		<b>2015 w/o Project</b>	<b>2015 With Project</b>
<b>Collins Ave / 11<sup>th</sup> Street</b>	B (17.9)	B (18.0)	B (18.1)
<b>Collins Ave / 12<sup>th</sup> Street</b>	D (33.7) *	B (14.4)	B (16.1)
<b>Washington Ave / 11<sup>th</sup> Street</b>	B (13.7)	B (13.8)	B (13.8)
<b>Washington Ave / 12<sup>th</sup> Street</b>	B (11.8)	B (11.9)	B (12.1)
<b>Ocean Ct / 12<sup>th</sup> Street</b>	A (7.5) *	A (8.5) *	A (8.8) *

Source: Highway Capacity Manual and SYNCHRO

Legend: B (11.9) = LOS (Average Delay – Seconds / Vehicle)

\* At stop-controlled intersections, the LOS on the critical side street is documented in this table.

## **OTHER MODES OF TRANSPORTATION**

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Throughout much of Miami Beach, and specifically within the immediate area of the proposed Surfstyle project, there are many convenient and cost-effective transportation alternatives for residents and visitors alike. As noted earlier in this report, a trip reduction factor of 20% was incorporated into this traffic analysis to reflect those patrons that are likely to avail themselves of these alternative travel modes as opposed to the automobile. Several of the more prominent modes in this area include bus transit services, the Deco Bike, the airport shuttle service and the sidewalk network throughout the surrounding area. Each of these is explained in further detail below.

### **Miami-Dade Transit**

Transit services on Miami Beach are provided by Miami-Dade Transit. There are numerous transit routes serving the immediate study area including 103 Route C, Route 150, Route 120 Beach MAX and the Route 123 SB Local. These transit routes provide frequent service and access to all of Miami-Dade County as well as connections to other destinations outside of the County. Miami-Dade Transit bus stops are provided between 11<sup>th</sup> Street and 12<sup>th</sup> Street along Washington Avenue.

### **Airport Shuttle Service**

Shuttle service to and from the Miami International Airport (MIA) is provided by Miami-Dade Transit via Route 150 which is also known as the Miami Beach Airport Flyer. This transit service is offered from 44<sup>th</sup> Street on the north to South Pointe Drive on the south. Within the Surfstyle project study area, this service is provided along Washington Avenue with designated stops at 9<sup>th</sup> Street and 13<sup>th</sup> Street. On weekdays, this service is provided every 30 minutes between 6:00 AM and 12:00 AM (midnight).

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## **DecoBike**

DecoBike is a bicycle sharing and rental program on Miami Beach. This program offers a network of over 100 solar-powered bicycle rental stations and a fleet of over 1,000 bicycles which can be rented 24 hours per day. Within the immediate area of the Surfstyle, there are at least three (3) convenient DecoBike rental stations. These stations are as follows:

- Station 119: 11<sup>th</sup> Street and Washington Avenue
- Station 118: 13<sup>th</sup> Street and Washington Avenue
- Station 115: 13<sup>th</sup> Street and Collins Avenue

## **Pedestrian Network**

Most of Miami Beach is considered a very walkable environment. Specifically within the project study area, each of the existing roadways has sidewalks on both sides and crosswalks are present at each of the major intersections. There are many attractive destinations within easy access to the Surfstyle and the project has been designed in such a manner as to provide direct access to this sidewalk network.

In summary, this project is located within an area that provides excellent access to alternative modes of transportation. It is expected that many of the patrons of the Surfstyle will utilize these services as opposed to driving passenger vehicles.

## PARKING AND QUEUING ANALYSIS

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Parking for the Surfstyle project will be provided via a 148-space mechanical parking garage. Access to the parking garage will be provided by a two-lane driveway on 12<sup>th</sup> Street and egress will be provided by a connection to Ocean Court. All parking will be valet only. The proposed design will provide a storage capacity for four (4) vehicles in advance of the mechanical elevators that will take vehicles to parking located on the third level, fourth level, and on the roof.

A queuing analysis has been conducted for the valet station associated with the mechanical garage. The vehicle storage (or queue) anticipated for this location was determined using information contained in ITE's *Transportation and Land Development*, Chapter 8 – Drive-In Facilities<sup>1</sup>. For this analysis, the following input variables were used:

- **Service Rate:** With a service rate of approximately two (2) minutes per vehicle, the two (2) mechanical elevators will be able to process approximately 60 vehicles in an hour.
- **Demand Rate:** Based upon the trip generation analysis, the inbound / outbound split of traffic is nearly 50% in / 50% out (42 inbound and 36 outbound). As a result, a lift that delivers a vehicle to a parking position will typically be able to return to the ground floor with another vehicle. As such, this analysis focuses on the peak hour demand of 42 inbound vehicles.

Using equation 8-9b and Table 8-11 of ITE's *Transportation and Land Development*, the maximum valet demand anticipated at the Surfstyle development, at the 90% confidence level, is four (4) vehicles. This queue (or storage) can be accommodated by the proposed number of service positions in advance of the mechanical elevator lifts. The results of this analysis are contained in Appendix I.

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<sup>1</sup> By Vergil G. Stover and Frank J. Koepke.

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During peak time periods, it is recommended that up to seven (7) valet attendants be stationed within the garage to ensure optimal operations. Of these valet attendants, it is recommended that four (4) be stationed within the vehicle drop-off / pick-up area to expedite the transition of the vehicle between the driver and the garage. The remaining three (3) attendants should be located on each of the three (3) floors of parking. This will expedite the maneuvering of the vehicles on and off of the mechanical elevator lifts.

## SUMMARY & CONCLUSIONS

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Surfstyle is a proposed development to be located on the southeast corner of the intersection of Collins Avenue (State Road A1A) and 12<sup>th</sup> Street in the City of Miami Beach, Miami-Dade County, Florida. More specifically, the subject site is located at 1155 Collins Avenue. This project involves the redevelopment of the existing surface parking lot with retail space and a parking garage.

Traf Tech Engineering, Inc. has been retained by CG Tides Village LLC to conduct a traffic study in connection with the redevelopment of this property. This study addresses trip generation and the traffic impacts created by the proposed project on the nearby transportation network. The conclusions of the traffic study are presented below:

- The net new external trips anticipated to be generated by the proposed Surfstyle project consists of approximately 44 trips during the PM peak hour of the generator (24 inbound and 20 outbound trips).
- All study intersections are currently operating adequately and will continue to operate at an acceptable level of service in the year 2015 with the proposed project in place.
- A review of the vehicle storage capacity provided at the subject valet drop-off / pick-up area indicates that up to four (4) vehicles can be accommodated. Based upon the queuing analysis performed for this site, this storage capacity is adequate to meet the anticipated peak demand. It is recommended that up to seven (7) valet attendants be stationed at this site during the peak time periods to ensure adequate operations and to minimize the possibility of occasional back-ups on the adjacent street system.

**TO:** Xavier Falconi, PE  
City of Miami Beach

**FROM:** Joaquin Vargas, PE  
Karl Peterson, PE  
Traf Tech Engineering

**DATE:** April 1, 2014

**SUBJECT:** Surfstyle Traffic Study  
Response to Peer Review Comments

---

Surfstyle is a proposed retail facility and parking garage planned to be located at 1155 Collins Avenue in the City of Miami Beach, Florida. A traffic study for this project was submitted to the City in February 2014. A peer review of this study was performed by The Corradino Group. The following are our responses to their comments:

**1. Page 9 / Table 1 – It is not clear how the trip credit for the existing surface parking lot was calculated. Please provide supporting documentation.**

***Response:** The traffic (i.e. trip credit) associated with the existing surface parking lot was obtained from traffic counts performed at the parking lot driveways. These counts, which are included in Appendix C of the report, indicate that the peak hour inbound volume is 18 vehicles and the peak hour outbound volume is 16 vehicles. This information has been extracted from the appendices and presented as Attachment A to this memorandum. We have also summarized the results of these counts in Figure A-1. The source of this information should have been more clearly explained in the report and we apologize for the confusion.*

**2. Page 21 / Demand Rate – The inbound and outbound trips should match the trips in Table 1. Please revise language and associated analysis.**

***Response:** We believe that the volumes (42 inbound and 36 outbound) presented in the Demand Rate analysis section are consistent with those documented in Table 1. The sum of the vehicle trips associated with the specialty retail component (less the multimodal reduction) and the parking garage are 42 inbound vehicles and 36 outbound vehicles. There is a credit for existing trips associated with the existing surface parking lot; however, that does not apply to the actual demand for the proposed parking garage.*

3. **Page 21 / Parking and Queuing Analysis – The valet/mechanical parking operation is not explained clearly. The valet operation should be explained with the help of a figure, highlighting the distance from valet stalls to the parking facility. The valet and mechanical parking operation is also not explained for ground, second and third floors. This should include time required for elevators to reach the top most floors, the distance from the elevators to the farthest parking space and the time required to park the vehicle in farthest mechanical parking. In short, clarification is needed for providing the proposed 7 valet attendants, as there may be more valet attendants needed to adequately perform these operations during peak hours.**

*Response: The queuing analysis is only applicable to the mechanical system. It was conservatively assumed that each vehicle will be parked or unparked within a 2-minute period. The purpose of the queuing evaluation was to ensure that the storage capacity available in advance of the lift station is adequate which the report addresses.*

4. **The delivery vehicle path is not mentioned in the report. If off-site parking is proposed for deliveries then permit must be obtained through the City Parking Department.**

*Response: Off-site parking for deliveries is not being sought for this project. Given the small scale nature of the retail component for the Surfstyle site, only small delivery vehicles will be used. These vehicles will make deliveries during off-peak periods and will utilize the driveway on 12<sup>th</sup> Street or the access point on the adjacent alley to access the first floor of the building where they will park for short durations.*

5. **Synchro Analysis – If required, revise synchro analysis based on above comment# 2. For all the three signalized intersections the cycle lengths and offsets are off when compared to MDC signal timing sheets. Please revise.**

*Response: No adjustments to the Synchro analyses are required as a result of comment #2. Each of the signalized intersection analyses have been reviewed and the cycle lengths have been confirmed and the offsets have been corrected. The revised analyses are included in Attachment B.*



August 6, 2018

Matthew Amster, Esq.  
Bercow Radell Fernandez & Larkin  
200 S. Biscayne Boulevard  
Suite 850  
Miami, Florida 33131

**Re: Valet Operations Plan for 1155 Collins Avenue - Miami Beach**

Dear Matt:

Pursuant to your request, Traf Tech Engineering, Inc. has prepared an updated valet analysis in connection with the previously approved Surfstyle project located on the south side of 12<sup>th</sup> Street between Collins Avenue and Ocean Court in the City of Miami Beach. Figure 1 on the following page shows the location of the project site. The purpose of this updated valet analysis is to document the anticipated number of valet attendants and the maximum on-site queueing anticipated with the new parking usage data, specifically after midnight use for 24/7 operation. The following is a summary of our findings.

**Background Information**

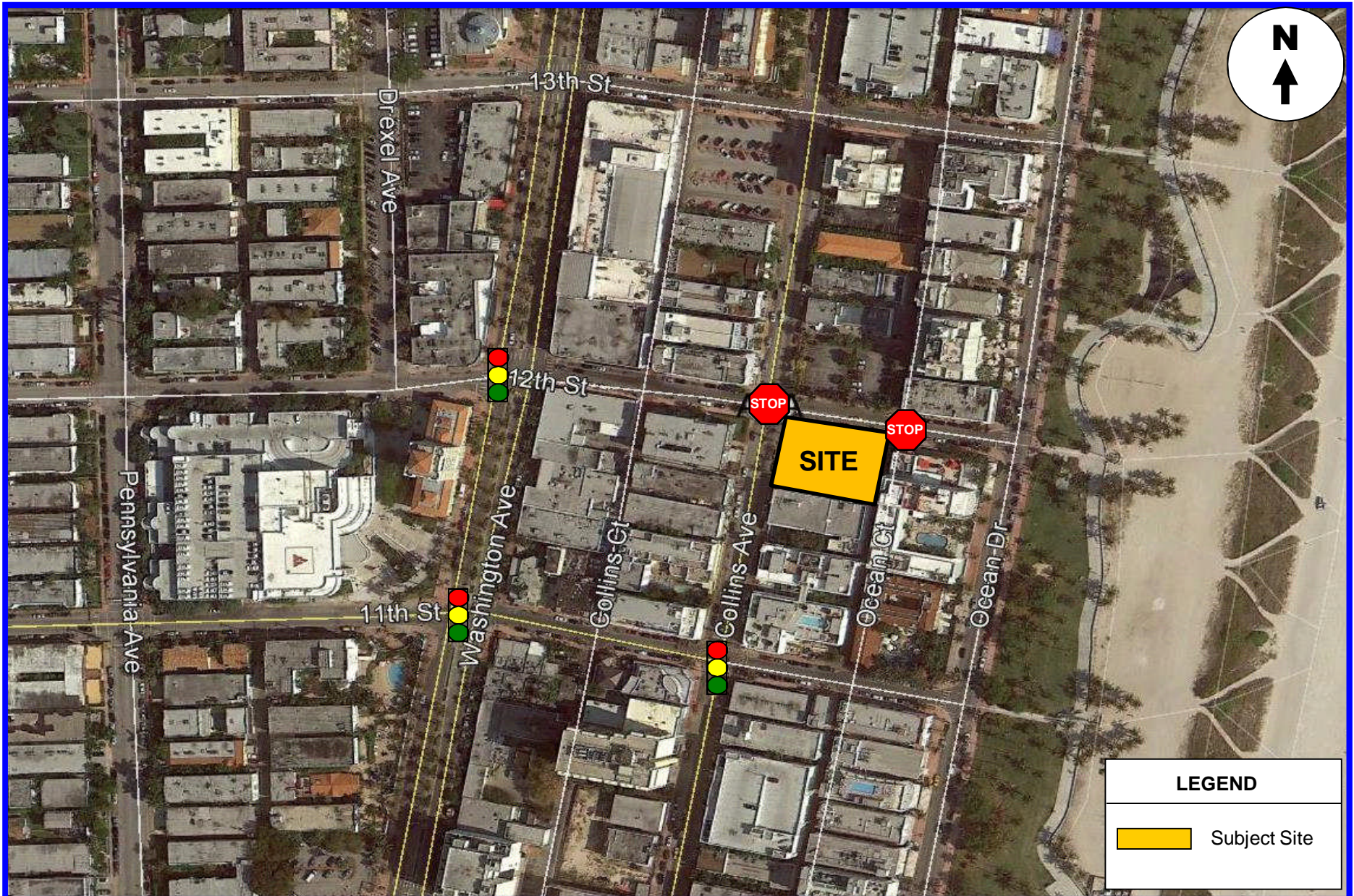
Parking will be provided via a 148-space mechanical parking garage. All parking will be valet only. The proposed design will provide a storage capacity for four (4) vehicles in advance of the mechanical elevators that will take vehicles to parking located on the second level, third level and on the roof (refer to Attachment A). The 148 parking spaces will be assigned as follows:

- 35 parking spaces for the retail use on the site
- 43 parking spaces dedicated to the Tides Village Hotel located on Ocean Drive (Attachment B shows the location of Tides Village Hotel and its relation to the 1155 Collins development)
- 70 parking spaces available to the public

**Trip Generation**

The trip generation associated with the three parking users was determined based on time-of-day parking utilization data published in the Urban Land Institute (ULI) *Shared Parking* (Second Edition) document. According to ULI, retail use has an hourly variation of up to 20% in parking usage. Similarly, hotel land uses have an hourly variation of up to 10%. Assuming that the peak parking of all three users (on-site retail, Tides Village Hotel and general public) coincides, the maximum hourly volume anticipated at the 1155 Collins garage is approximately 26 vehicles per hour (35 x 20% for on-site retail + 43 x 10% for the hotel users + 70 x 20% for general public).







A queuing analysis has been conducted for the valet station associated with the mechanical garage. The vehicle storage (or queue) anticipated for this location was determined using information contained in ITE's *Transportation and Land Development*, Chapter 8 – Drive-In Facilities<sup>1</sup>. For this analysis, the following input variables were used:

- **Service Rate:** With a service rate of approximately two (2) minutes per vehicle, the two (2) mechanical elevators will be able to process approximately 60 vehicles in an hour.
- **Demand Rate:** As presented in the trip generation above, a maximum of approximately 26 peak hour vehicles are anticipated at this facility.

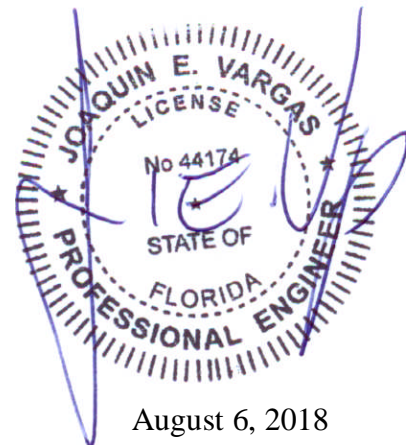
Using equation 8-9b and Table 8-11 of ITE's *Transportation and Land Development*, the maximum valet demand anticipated at the 1155 Collins development, at the 95% confidence level, is one (1) vehicle. This queue (or storage) can be accommodated by the proposed number of service positions in advance of the mechanical elevator lifts. The results of this analysis are contained in Attachment C.

During peak time periods, it is recommended that up to five (5) valet attendants be stationed within the garage to ensure optimal operations. Of these valet attendants, it is recommended that two (2) be stationed within the vehicle drop-off / pick-up area to expedite the transition of the vehicle between the driver and the garage. The remaining three (3) attendants should be located on each of the three (3) floors of parking. This will expedite the maneuvering of the vehicles on and off of the mechanical elevator lifts.

Please give me a call if you have any questions.

**TRAF TECH ENGINEERING, INC.**

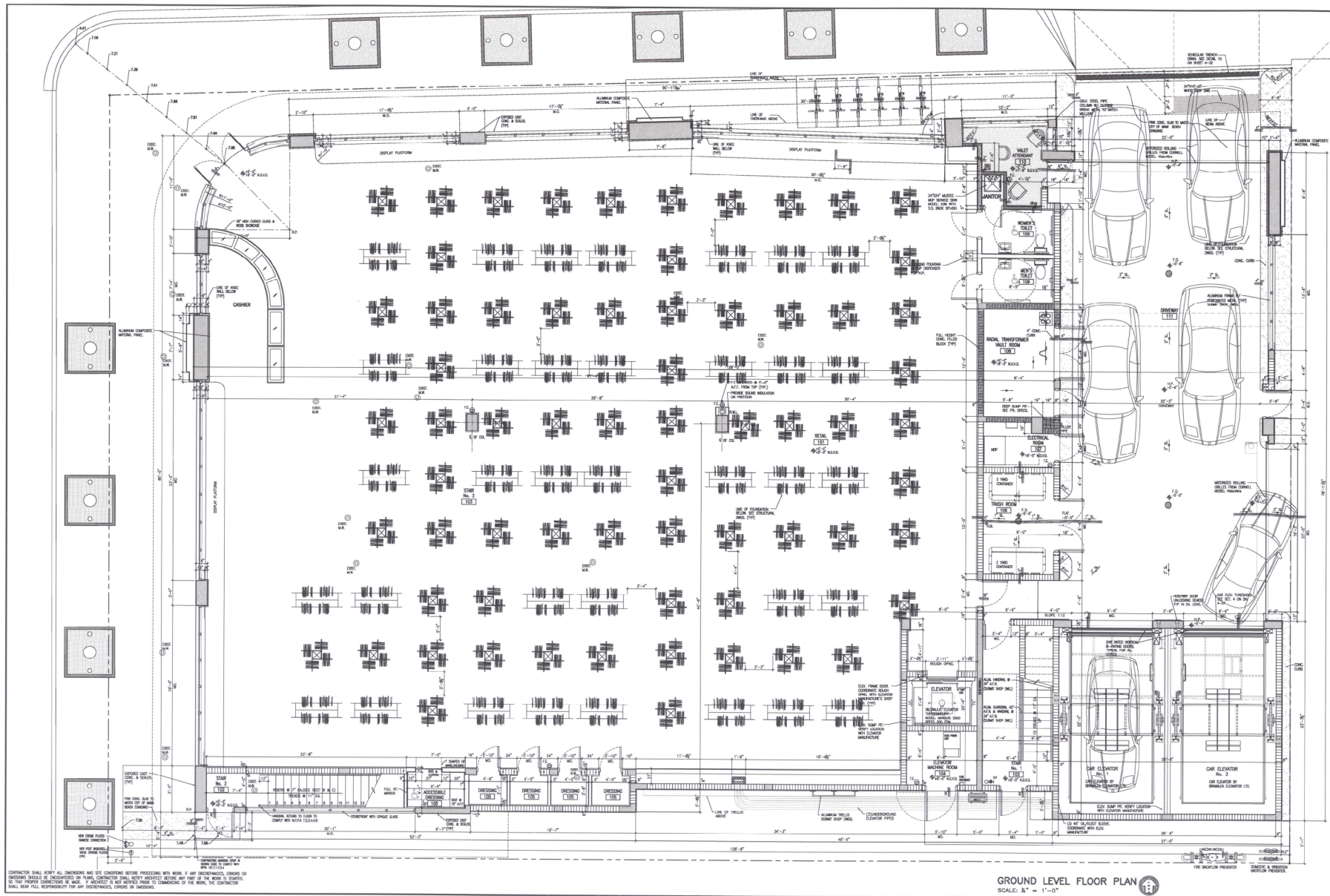
**Joaquin E. Vargas, P.E.**  
Senior Transportation Engineer



<sup>1</sup> By Vergil G. Stover and Frank J. Koepke.

**ATTACHMENT A**

**Ground Floor Plan**



CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND USE CONDITIONS BEFORE PROCEEDING WITH WORK. IF ANY DISCREPANCIES, ERRORS OR OMISSIONS ARE DETECTED, CONTRACTOR SHALL NOTIFY ARCHITECT BEFORE ANY PART OF THE WORK IS STARTED. NO PART OF THIS DRAWING IS TO BE USED IN ANY MANNER WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. THE USER THEREOF SHALL BE RESPONSIBLE FOR ANY DISCREPANCIES, ERRORS OR OMISSIONS.

These drawings and copies thereof furnished by the architect are his property and are not to be used on other work and are to be returned upon completion of work. If this drawing or any part thereof is used in any manner without the consent of the architect, the user thereof becomes responsible to the architect for full completion.

NEW RETAIL/PARKING BUILDING FOR:

**SURFSTYLE**

1155 Colline Ave.

Miami Beach, FL 33139



CHARLES H. BENSON & ASSOCIATES ARCHITECTS, P.A.  
architect planner interiors

1685 WASHINGTON AVE. 2ND FLOOR, MIAMI BEACH, FLORIDA 33139 Ph: 305-432-4181 Fax: 305-432-4151  
ARCHITECTURE LICENSE # A14022 NCARB CERTIFICATE # 42,136

DRAWN BY  
Charles H. Benson

SCALE  
AS SHOWN

PROJECT NUMBER

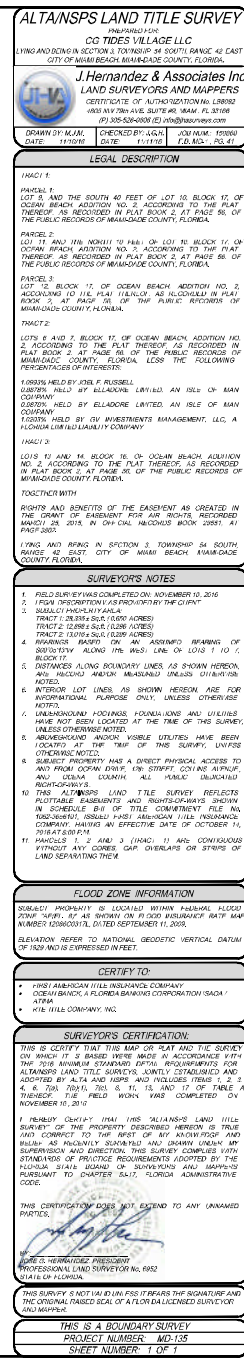
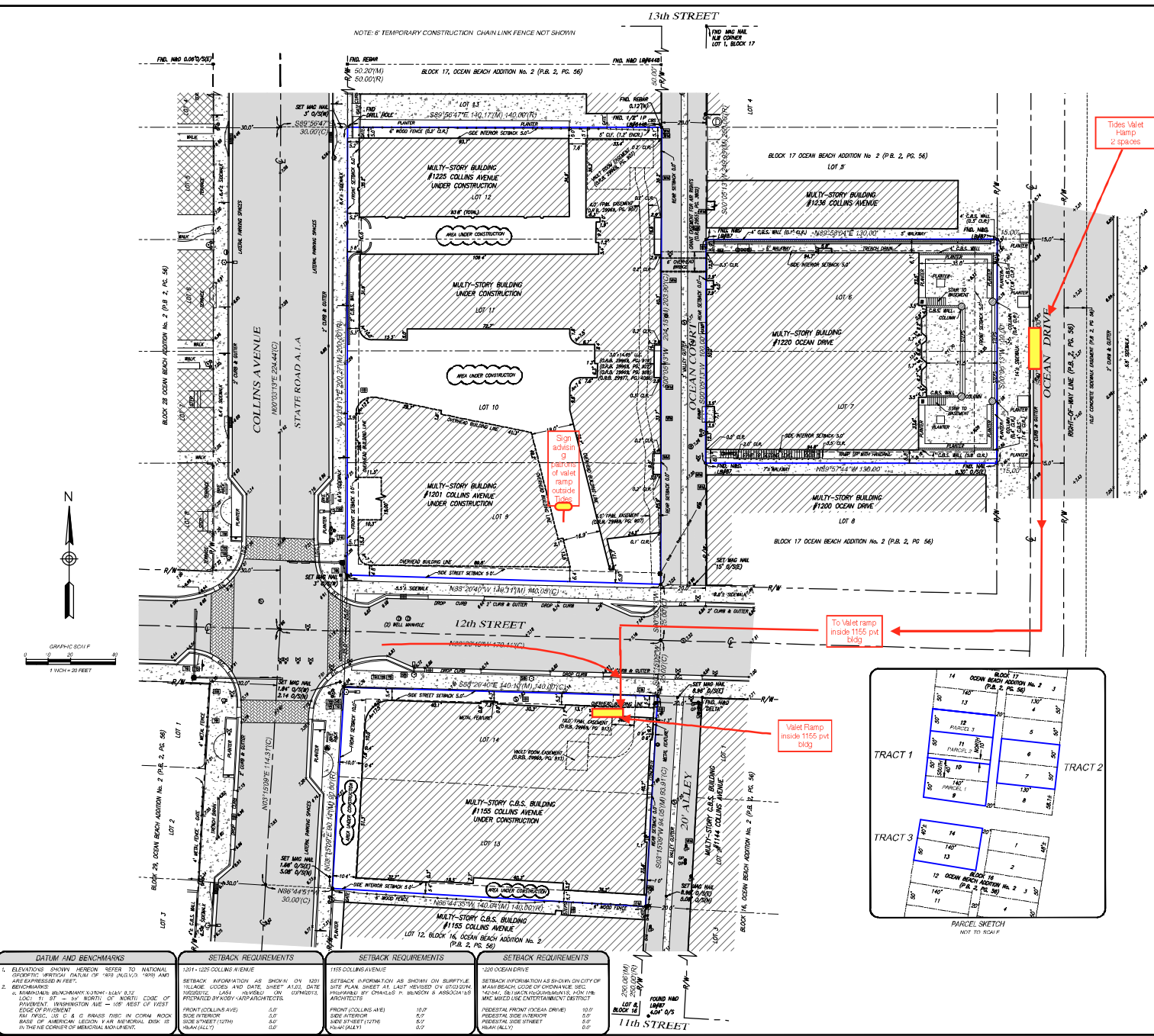
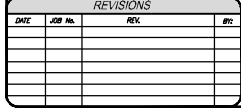
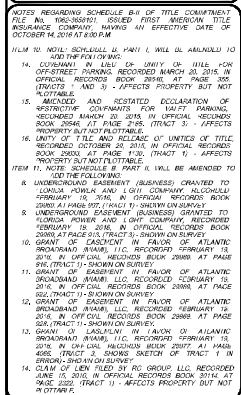
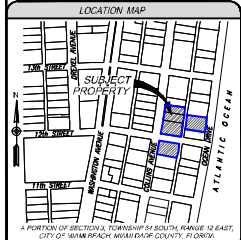
21315

A-2  
or  
12

# **ATTACHMENT B**

## **Project Location Map and Tides Village Hotel Site**





# **ATTACHMENT C**

## **Valet Analysis**



location, a 5% probability of back-up onto the adjacent street is judged to be acceptable. Demand on the system for design is expected to be 110 vehicles in a 45-minute period. Average service time was expected to be 2.2 minutes. Is the queue storage adequate?

Such problems can be quickly solved using Equation (8-9b) given in Table 8-10 and repeated below for convenience.

$$M = \left[ \frac{\ln P(x > M) - \ln Q_M}{\ln \rho} \right] - 1$$

where:

$M$  = queue length which is exceeded  $p$  percent of the time

$N$  = number of service channels (drive-in positions)

$Q$  = service rate per channel (vehicles per hour)

$\rho = \frac{\text{demand rate}}{\text{service rate}} = \frac{q}{NQ}$  = utilization factor

$q$  = demand rate on the system (vehicles per hour)

$Q_M$  = tabled values of the relationship between queue length, number of channels, and utilization factor (see Table 8.11)

TABLE 8-11

Table of  $Q_M$  Values

	$N = 1$	2	3	4	6	8	10
0.0	0.0000	0.0000	0.0000	0.0000			
0.1	.1000	.0182	.0037	.0008	.0000	0.0000	0.0000
.2	.2000	.0666	.0247	.0096	.0015	.0002	.0000
.3	.3000	.1385	.0700	.0370	.0111	.0036	.0011
.4	.4000	.2286	.1411	.0907	.0400	.0185	.0088
.5	.5000	.3333	.2368	.1739	.0991	.0591	.0360
.6	.6000	.4501	.3548	.2870	.1965	.1395	.1013
.7	.7000	.5766	.4923	.4286	.3359	.2706	.2218
.8	.8000	.7111	.6472	.5964	.5178	.4576	.4093
.9	.9000	.8526	.8172	.7878	.7401	.7014	.6687
1.0	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000

$$\rho = \frac{q}{NQ} = \frac{\text{arrival rate, total}}{(\text{number of channels})(\text{service rate per channel})}$$

$N$  = number of channels (service positions)

### Solution

Step 1:  $Q = \frac{60 \text{ min/hr}}{2.2 \text{ min/service}} = 27.3$  services per hour

Step 2:  $q = (110 \text{ veh/45 min}) \times (60 \text{ min/hr}) = 146.7$  vehicles per hour

Step 3:  $\rho = \frac{q}{NQ} = \frac{146.7}{(6)(27.3)} = 0.8956$

Step 4:  $Q_M = 0.7303$  by interpolation between 0.8 and 0.9 for  $N = 6$  from the table of  $Q_M$  values (see Table 8-11).

Step 5: The acceptable probability of the queue,  $M$ , being longer than the storage, 18 spaces in this example, was stated to be 5%.  $P(x > M) = 0.05$ , and:

$$M = \left[ \frac{\ln 0.05 - \ln 0.7303}{\ln 0.8956} \right] - 1 = \left[ \frac{-2.996 - (-0.314)}{-0.110} \right] - 1$$

$$= 24.38 - 1 = 23.38, \text{ say } 23 \text{ vehicles.}$$

0.4 ----- 0.2286  
0.433 ---- X  
0.4 ----- 0.3333

X = 0.2632

### Queuing Analysis based on ITE Procedures

$q = 26$  veh/hr (demand rate)

$Q = 30$  veh/hr (service rate)

$N = 2$  (service channels)

$$p = \frac{q}{NQ} = 0.4333 \text{ (N = two)}$$

$$Q_M = 0.2632 \text{ (for N = 2)}$$

Using Acceptable Probability of 5% (95% Confidence Level)

$$M = \left( \frac{\ln(x > M) - \ln(Q_M)}{\ln(p)} \right) - 1$$

$$M = \left( \frac{\ln(0.05) - \ln(0.2632)}{\ln(0.4333)} \right) - 1$$

$$M = \left( \frac{-2.9957 - (-1.3348)}{-0.8363} \right) - 1$$

$$M = 1.98 - 1 = 0.98, \text{ say 1 vehicle}$$

**Planning Board Application**

**PB18-0223**

**1155 Collins Avenue**



**View to SE from Collins Avenue & 12<sup>th</sup> Street**



**View E from Collins Avenue**



**Planning Board Application**  
**PB18-0223**  
**1155 Collins Avenue**



**View to NE from Collins Avenue**



**View to South from 12<sup>th</sup> Street**



**Planning Board Application**  
**PB18-0223**  
**1155 Collins Avenue**



**View to South from 12<sup>th</sup> Street**



**View of Entrance at NE Corner of Building**

**Planning Board Application**

**PB18-0223**

**1155 Collins Avenue**



**View of Entrance and Adjacent Alley**



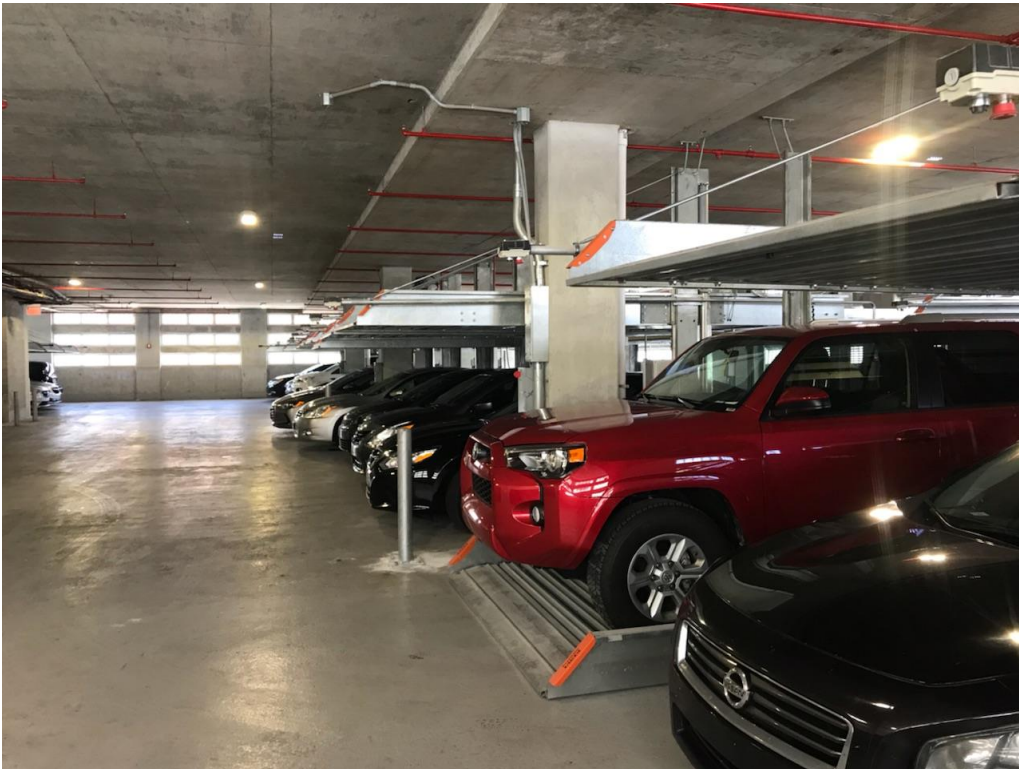
**View of Inside of Garage**



**Planning Board Application**  
**PB18-0223**  
**1155 Collins Avenue**



**View of Inside of Garage**



**View of Inside of Garage**