June 15, 2017

Park Central Hotel c/o Carli Koshal, Esq. Bercow Radell & Fernandez, P.A. 200 S. Biscayne Boulevard, Suite 850 Miami, Florida 33131

#### Re: Park Central Hotel (626-650 Ocean Drive) – Traffic Study

Dear Carli:

Per your request, Traf Tech Engineering, Inc. conducted a traffic evaluation associated with the proposed improvements to the existing Park Central Hotel (626 - 650 Ocean Drive) located in the City of Miami Beach in Miami-Dade County, Florida. Figure 1 on the following Page shows the location of the project site. This report documents the existing roadway characteristics, projected trip generation and traffic impacts to the surrounding street system as a result of the proposed improvements, traffic circulation, pedestrian circulation and pedestrian facilities analysis (sidewalks and crosswalks). The following is a summary of our findings:

#### **Existing Roadway Conditions**

The roadway system adjacent to the project site includes Ocean Drive, a two-lane northsouth facility with on-street parking on both sides of the street and a posted speed limit of 30 miles per hour. North of the site is the signalized intersection of 7<sup>th</sup> Street with pedestrian crosswalks on the north, south and west legs of the intersection. Valet stations are provided on the west side of Ocean Drive serving the numerous restaurants and hotel located within South Beach.

#### **Trip Generation Estimate**

A trip generation analysis was performed using the trip generation rates published in the Institute of Transportation Engineer's (ITE) *Trip Generation Manual* (9<sup>th</sup> Edition). The trip generation analysis was undertaken for daily, AM peak hour and PM peak hour conditions. The analysis was based on the following assumptions:

#### EXISTING LAND USE

o Hotel (127 rooms)

#### PROPOSED LAND USES

- o Hotel (120 rooms)
- Terrace Improvements (110 new seats, other seats are at previous existing seating areas)

According to ITE's *Trip Generation Manual* (9<sup>th</sup> Edition), the trip generation rates used for the existing and proposed land uses are:

8400 North University Drive, Suite 309, Tamarac, Florida 33321

Tel: (954) 582-0988 Fax: (954) 582-0989



## **PROJECT LOCATION MAP**

FIGURE 1 Park Central Hotel Miami Beach, Florida



ENGINEERING, INC.

HOTEL (ITE Land Use 310)

Daily Trip Generation T = 8.17 (X)Where T = number of daily trips, X = number of hotel rooms

*PM Peak Hour of the Adjacent Street* T = 0.60 (X) (51% inbound and 49% outbound) Where T = number of peak hour trips, X = number of hotel rooms

RESTAURANT (ITE Land Use 931)

Daily Trip Generation T = 2.86 (X)Where T = number of daily trips, X = number of seats

*PM Peak Hour of the Adjacent Street* T = 0.26 (X) (67% inbound and 33% outbound)Where T = number of peak hour trips, X = number of seats

Using the above-listed equations from the ITE document, a trip generation analysis was undertaken for the existing and proposed land uses. The results of this effort are documented in Tables 1 and 2. As indicated in the tables, the proposed hotel improvements are projected to generate approximately 224 new daily trips and approximately 22 new peak hour trips (16 inbound and 6 outbound). Therefore, the proposed hotel improvements are anticipated to have minimal traffic impact to the surrounding street system (one new peak hour trip every two minutes and 43 seconds). Figures 2 and 3 illustrate the traffic circulation for new trips (arrival, parking, retrieval and departing trips) assuming valet usage of the parking garage located on 7<sup>th</sup> Street just west of Collins Avenue.

#### **Pedestrian Counts**

Pedestrian counts were collected on Friday, May19, 2017 near the intersection of Ocean Drive and 7<sup>th</sup> Street. The pedestrian counts included pedestrian counts travelling north-south along the west sidewalk of Ocean Drive south of 7<sup>th</sup> Street. Additionally, pedestrian counts crossing Ocean Drive at the signalized intersection of 7<sup>th</sup> Street were also undertaken. The pedestrian counts indicate that during peak 15-minute period the sidewalk volume includes approximately 185 pedestrians during the peak 15-minutes traveling north and south on the west side of Ocean Drive. The data also shows that approximately 150 pedestrians during a one-hour period. The pedestrian counts are contained in Appendix B.

#### Pedestrian Circulation

A sidewalk that varies between 5.4 feet and 10.9 feet is provided on the west side of Ocean Drive and adjacent and near to the 626-650 Ocean Drive site (refer to Photos depicted in Appendix B).



The sidewalk provides north-south pedestrian mobility within the immediate area of the project. From the sidewalk, access to the 626-650 Ocean Drive site is provided via pedestrian access path/stairs including new ramp railings immediately west of the sidewalk. Moreover, a signalized pedestrian crossing is provided at 7<sup>th</sup> Street located approximately 100 feet north of the site.

#### Pedestrian Facilities Analysis (Sidewalks and Crosswalks)

Based on the pedestrian counts contained in Appendix B, approximately 185 pedestrians/15-minutes travel north and south along the west side of Ocean Drive. As shown in the signal timing plans contained in Appendix C for the signalized located at 7<sup>th</sup> Street, the subject pedestrian crossing operates with an off-line signal cycle of 52 seconds, which results in approximately 69 pedestrian crossing opportunities per hour. Hence, the signalized pedestrian crossing at 7<sup>th</sup> Street has 69 opportunities per hour to accommodate 150 pedestrians per hour (sufficient pedestrian capacity is available at the subject signalized pedestrian crossing).

The traffic counts contained in Appendix B show a maximum of 185 pedestrians during the peak 15-minute period traveling north-south along the west side of Ocean Drive. With a sidewalk width of 5.4 feet (5 feet, 5 inches), the resulting pedestrian flow rate is approximately 2.284 pedestrians/minute/foot of sidewalk width (185 pedestrians per peak 15-minute period divided by 15 divided by 5.4). According to the 2010 Highway Capacity Manual (refer to Appendix D), the sidewalk adjacent to the site has adequate capacity to accommodate the peak pedestrian traffic recorded within this area.

#### Summary

The Park Central Hotel improvements are projected to generate approximately 79 new daily trips and approximately eight (8) new peak hour trips (five inbound and two outbound). Therefore, the proposed hotel improvements are anticipated to have a De-Minimus impact to the surrounding street system (one new peak hour trip every seven and one-half minutes). Adequate traffic and pedestrian circulation is provided for the project. Sufficient pedestrian capacity is available at the signalized pedestrian crossing located at 7<sup>th</sup> Street (approximately 100 feet from the site). Finally, the sidewalk located adjacent to the site has adequate capacity to accommodate the peak pedestrian traffic.

Sincerely,

#### TRAF TECH ENGINEERING, INC.

Joaquin E. Vargas, P.E Senior Transportation Engineer



TABLE 1 Trip Generation Summary (Current Use) 626 - 650 Ocean Drive							
			PM Peak Hour				
Land Use	Size	Daily Trips	Total Trips	Inbound	Outbound		
Hotel (LUC 310)	127	1,038	76	39	37		
External Trips		1,038	76	39	37		

Source: ITE Trip Generation Manual (9th Edition)

TABLE 2 Trip Generation Summary (Proposed Uses) 626 - 650 Ocean Drive								
PM Peak Hour								
Land Use	Size	Daily Trips	Total Trips	Inbound	Outbound			
Hotel (LUC 310)	120	980	72	37	35			
Terrace Seats (LUC 931)	110	315	29	19	10			
Internal Trips (1)		-33	-3	-1	-2			
External Trips		1,262	98	55	43			

Source: ITE Trip Generation Manual (9th Edition)

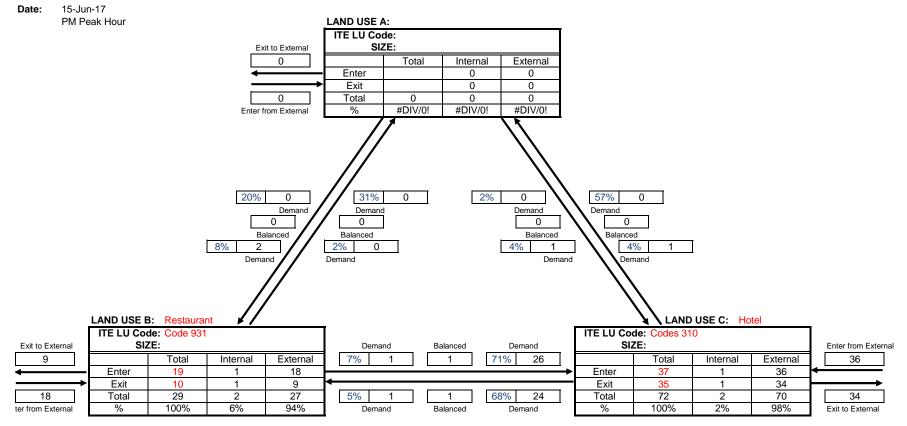
Difference in Trips	224	22	16	6

(1) See internal calculations on the following page



#### PROPOSED LAND USES Trip Generation and Internal Capture Summary

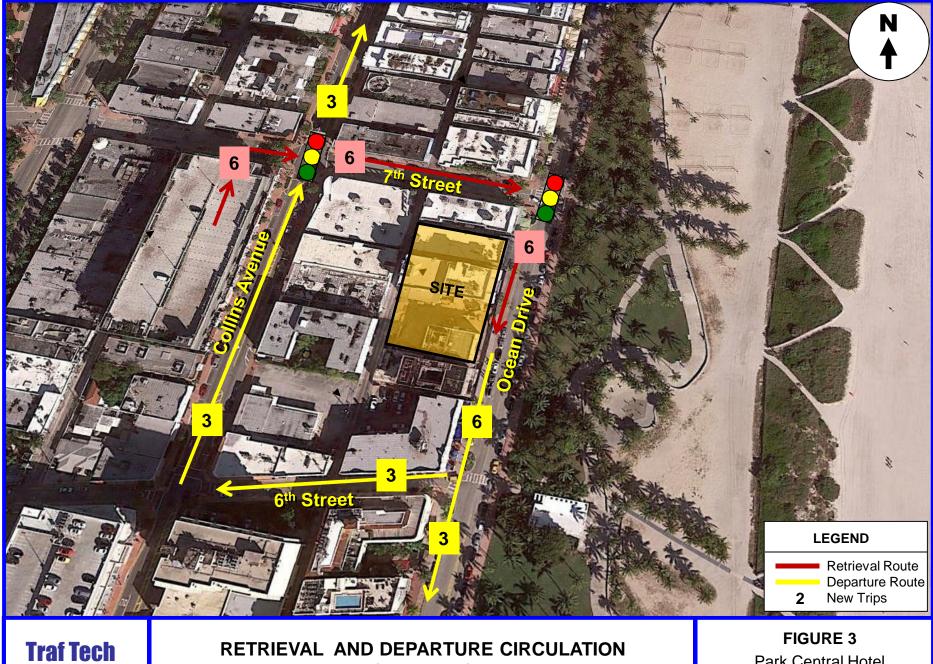
Analyst: Vargas



Net External Trips for Multi-Use Development									
	LAND USE A	LAND USE B	LAND USE C	TOTAL					
Enter	0	18	36	55	]				
Exit	0	9	34	43					
Total	0	27	70	98	INTERNAL CAP				
Single-Use Trip Gen. Est.	0	29	72	101	3%				



ARRIVAL AND PARKING CIRCULATION (New Trips) FIGURE 2 Park Central Hotel Miami Beach, Florida



**RETRIEVAL AND DEPARTURE CIRCULATION** (New Trips)

ENGINEERING, INC.

Park Central Hotel Miami Beach, Florida

## **APPENDIX A**

## **Site Plan – Park Central Hotel**

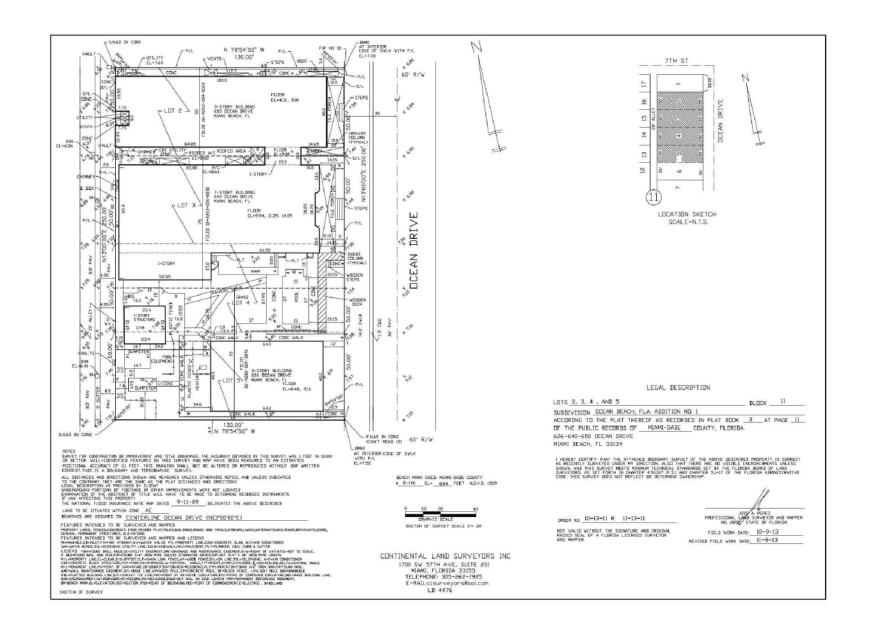
## THE PARK CENTRAL HOTEL

626-650 OCEAN DRIVE

### MIAMI BEACH FLORIDA

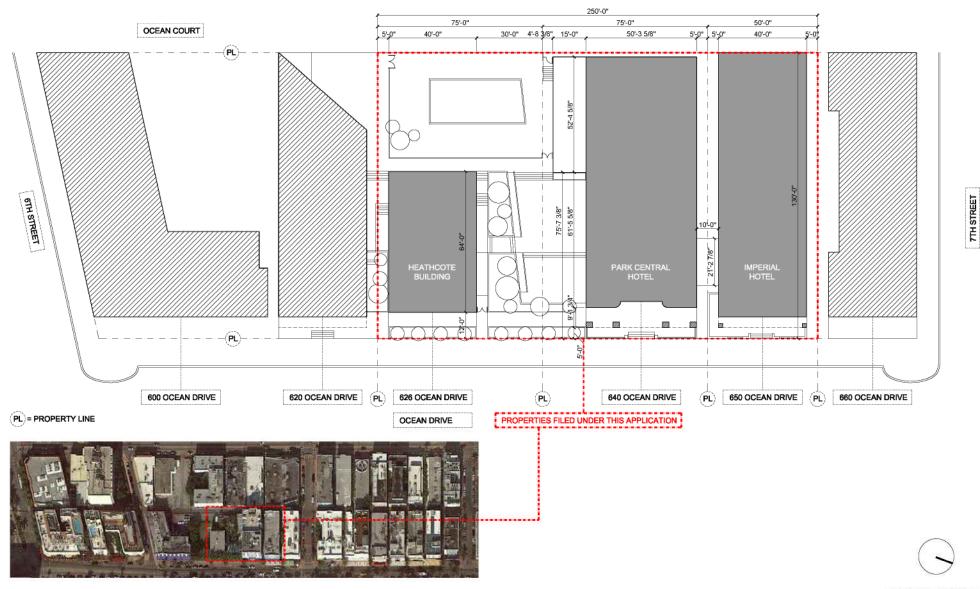
#### **space4** architecture

HISTORIC PRESERVATION BOARD SUBMITTAL (JANUARY 21, 2014)



space4architecture

THE PARK CENTRAL PARTNERS LLC 620-650 Ocean Drive, Mami Beach, FL 33139 PROJECT NAME THE PARK CENTRAL HOTEL 626/640/650 Ocean Drive, Miami Beach, FL 33139 LARDSCAPE ARCHITECT URBAN ROBOT ASSOCIATES 420 Lincoln Road Ste. #406, Mami Beach, FL 33139 T: 786-246-4857 Executive wechnest 1200 Brickell Avenue Suite 1525 Miami, FL 33131 1: 305-374-9216 DESION ARCHITECT SPACE 4 ARCHITECTURE 22 E 21st St # 9F New York, NY 10010 T: 212-253-7095 THE PARK CENTRAL HOTEL / MIAMIFLORIDA CERTIFIED SURVEY 626/640/650 OCEAN DRIVE SCAL: 128: DBATE 120 S-100 S-100



LOCATIONAL SITE PLAN

space4architecture

THE PARK CENTRAL PARTNERS LLC 620-650 Ocean Drive, Mami Beach, FL 33139

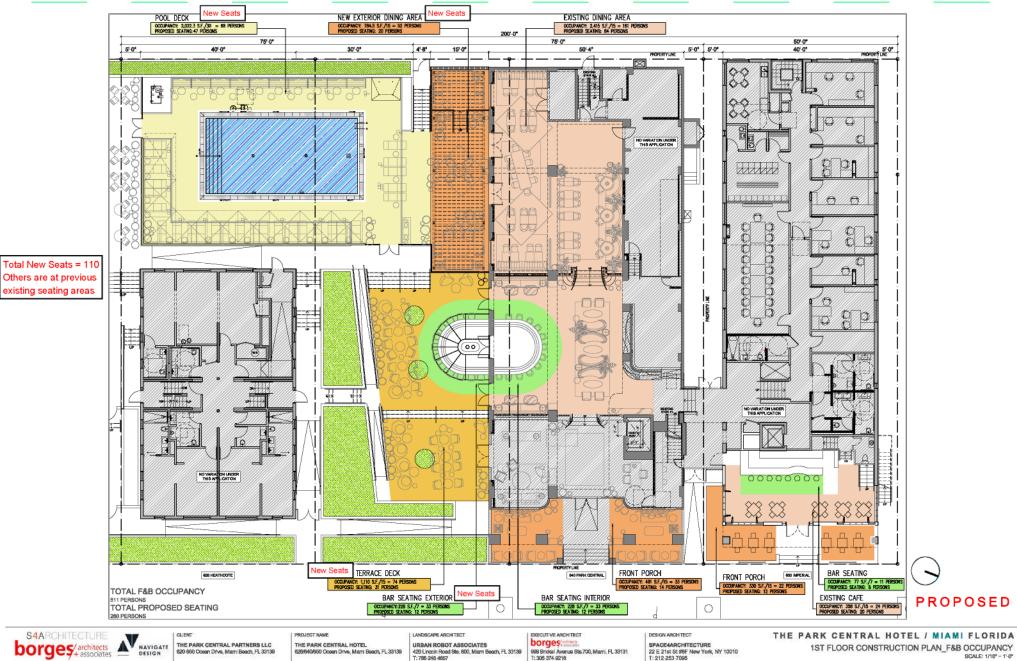
CLIENT

PROJECT NAME THE PARK CENTRAL HOTEL 626/640/650 Ocean Drive, Miami Beach, FL 33139

LANDSCARE ARCHITECT URBAN ROBOT ASSOCIATES 420 Lincoln Road Ste. #406, Miami Beach, FL 33139 T: 786-246-4857

borges+ EXECUTIVE ARCHITECT 1200 Brickell Avenue Suite 1525 Miami, FL 33131 T: 305-374-9216 DESIGN ARCHITECT SPACE 4 ARCHITECTURE 22 E 21st St # 8F New York, NY 10010 T: 212-253-7005

THE PARK CENTRAL HOTEL / MIAMI FLORIDA LOCATIONAL SITE PLAN NOT TO SUALE / DATE 12.92.12 Z-100

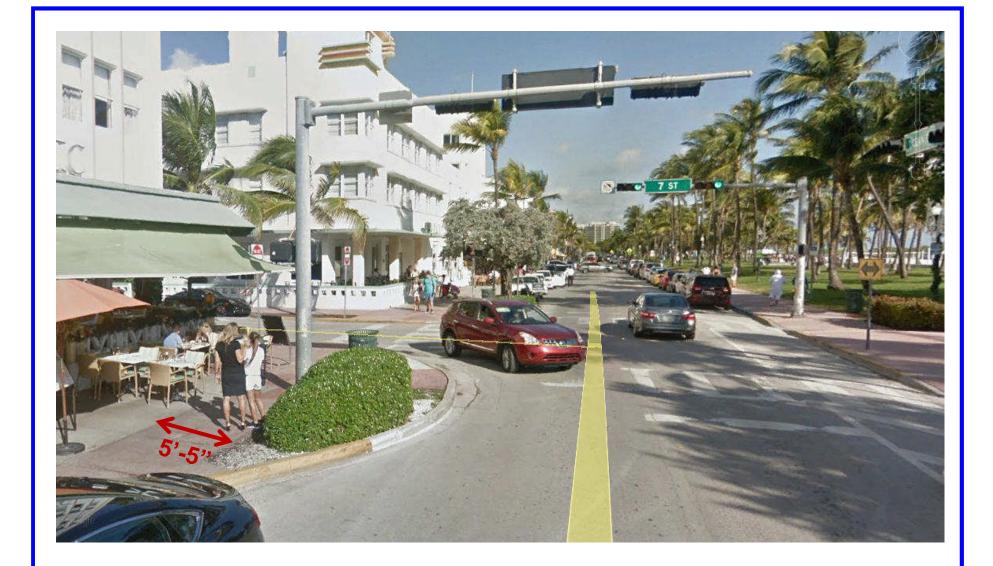


SCALE: 1/18\* - 1'-0\*

# **APPENDIX B Pedestrian Counts**

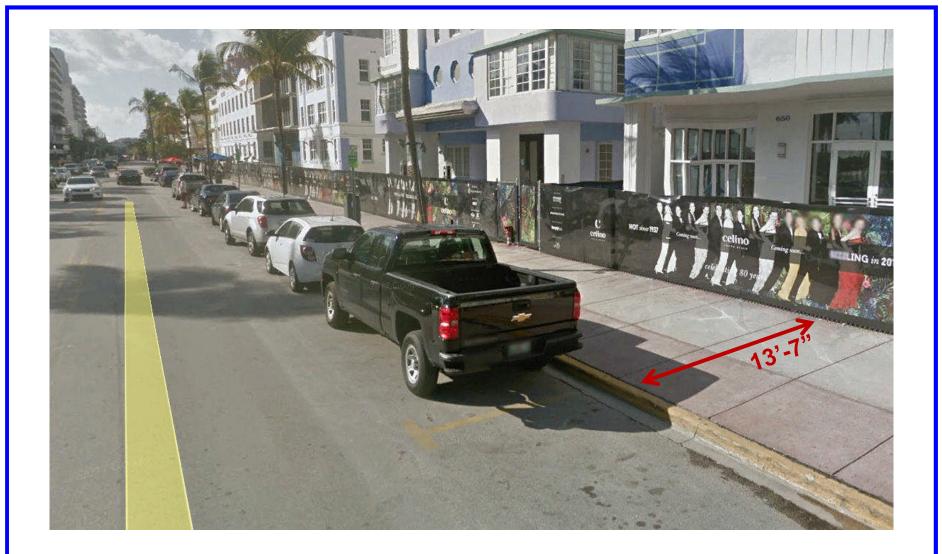
# 7<sup>TH</sup> STREET AND OCEAN DR., MIAMI BEACH DATE: MAY 19, 2017 PEDESTRIAN COUNT (W SIDEWALK & CROSSWALKS)

TIME	OCEAN DRIVE	OCEAN DRIVE	N CRC	<b>SSWALK</b>	·	S CROS	SWALK
	SOUTHBOUND	NORTHBOUND	 EB	WB		EB	WB
16:30-16:45	25	13	5	13		12	22
16:45-17:00	53	51	5	3		17	18
17:00-17:15	46	55	0	4		14	27
17:15-17:30	38	29	4	8		8	29
17:30-17:45	59	35	5	31		8	29
17:45-18:00	57	58	1	23		8	28
18:00-18:15	78	52	3	12		7	17
18:15-18:30	67	62	9	2		9	9
18:30-18:45	80	68	2	0		1	15
18:45-19:00	90	79	8	1		2	28
19:00-19:15	97	76	2	0		2	9
19:15-19:30	92	93	0	0		2	4



### SIDEWALK MEASUREMENT

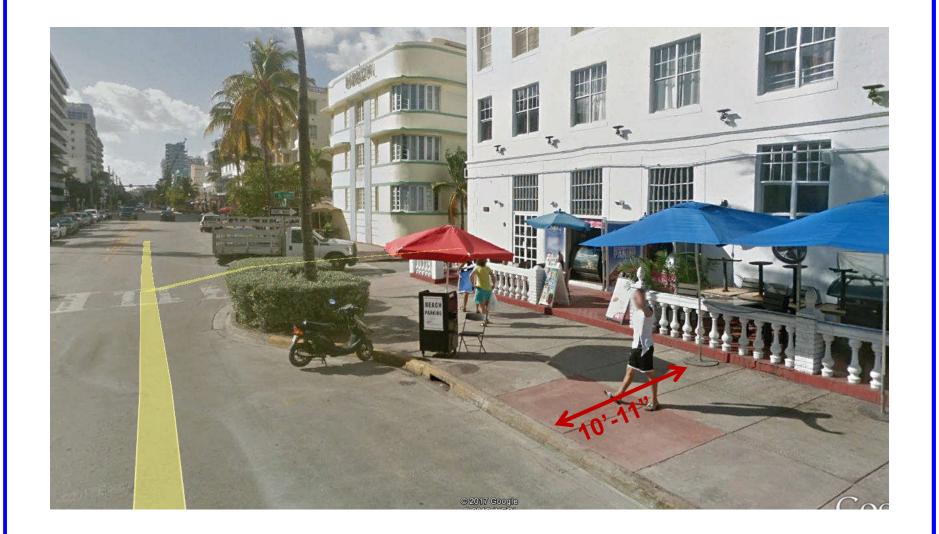
**FIGURE B-1** 1787 Purdy Avenue Miami Beach, Florida





### SIDEWALK MEASUREMENT

**FIGURE B-2** 1787 Purdy Avenue Miami Beach, Florida

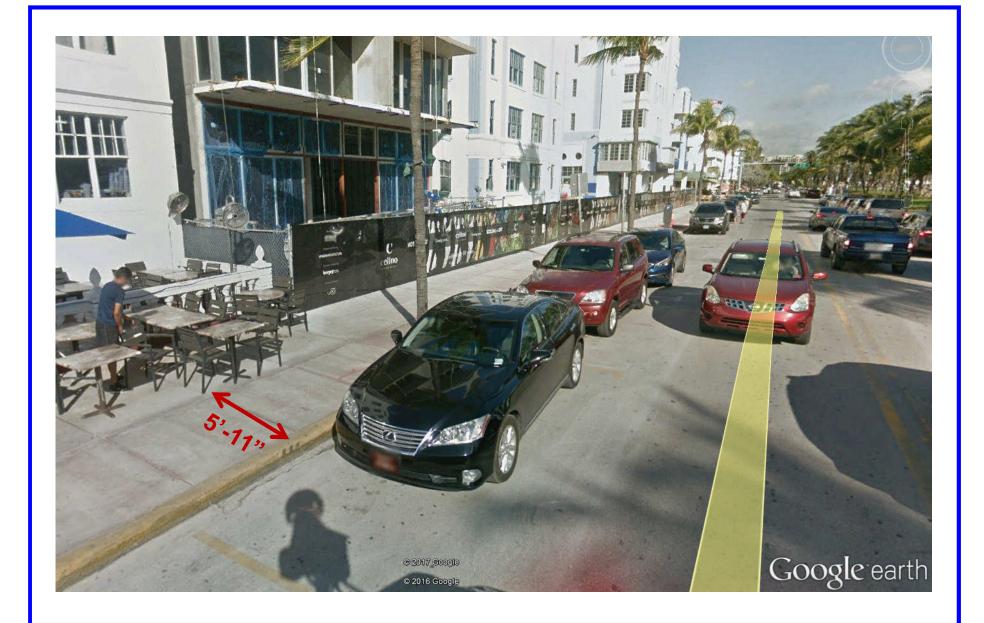




### SIDEWALK MEASUREMENT

1787 Purdy Avenue Miami Beach, Florida

FIGURE B-3



### SIDEWALK MEASUREMENT

**FIGURE B-4** 1787 Purdy Avenue Miami Beach, Florida

## **APPENDIX C**

Signal Timing (Ocean Drive and 7<sup>th</sup> Street)

Print Date: /25/2016					for 6345: Oc	ean Dr&7 St					Print Time 4:09 PM
<u>Asset</u> 6345		<u>section</u> Dr&7 St		<u>OD</u> edule <u>Op N</u> /-2	<u>Aode Pla</u>	<u>n #</u> N/A	<u>Cycle</u> 0	<u>Offset</u> 0	<u>TOD</u> <u>Setting</u> N/A	<u>Active</u> <u>PhaseBank</u> 0	<u>Active</u> <u>Maximum</u> Max 0
			<u>Splits</u>								
<u>PH 1</u> -	PH 2 PH SBT	<u>13 PH 4</u>	<u>РН 5</u>		<u>H7 PH8</u> - EBT						
0	0	0 0	0	0	0 0						
ctive Phase Phase	e Bank: Pha <u>Walk</u> Phase Bank 1 2 3	ase Bank 1 Don't Walk	<u>Min Initial</u>	<u>Veh Ext</u>	<u>Max Limit</u>	<u>Max 2</u> <u>Y</u>	<u>′ellow Red</u>	·	Service Date:	unknown	
- SBT -	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	0 - 0 - 0 12 - 12 - 12 0 - 0 - 0	0 - 0 - 0	0 - 0 -	0 0 - 0 - 0 1 25 - 25 - 25	0 - 0 - 0	0 0 4 2 0 0	Perm Defa	itted Phases	<u>12345678</u> -26-8	
-	0 - 0 - 0	0 - 0 - 0	0 - 0 - 0	0 - 0 -	0 0 - 0 - 0	0 - 0 - 0	0 0 0 0	Exter	nal Permit 0 nal Permit 1		
NBT - EBT	7     -     7     -     7       0     -     0     -     0       7     -     7     -     7	12 - 12 - 12 0 - 0 - 0 10 - 10 - 10	7 - 7 - 7 0 - 0 - 0	1 - 1 - 0 - 0 -	1 25 - 25 - 25	0 - 0 - 0	4 2 0 0 4 2	Exter	nal Permit 2		
	1 - 1 - 1	10 - 10 - 10		2.5 - 2.5 - 2			7 2	4 1			
<u>Current</u> TOD Sched		1 Cycle	2 3 SBT -	4 5	6 7 NBT -	8 EBT <u>Ring Offse</u>	t <u>Offset</u>	Local 1 <u>Time</u>	OD Schedule <u>Plan</u>	DOW	

## TOD Schedule Report

Page 1 of 2

#### TOD Schedule Report for 6345: Ocean Dr&7 St

Print Time:

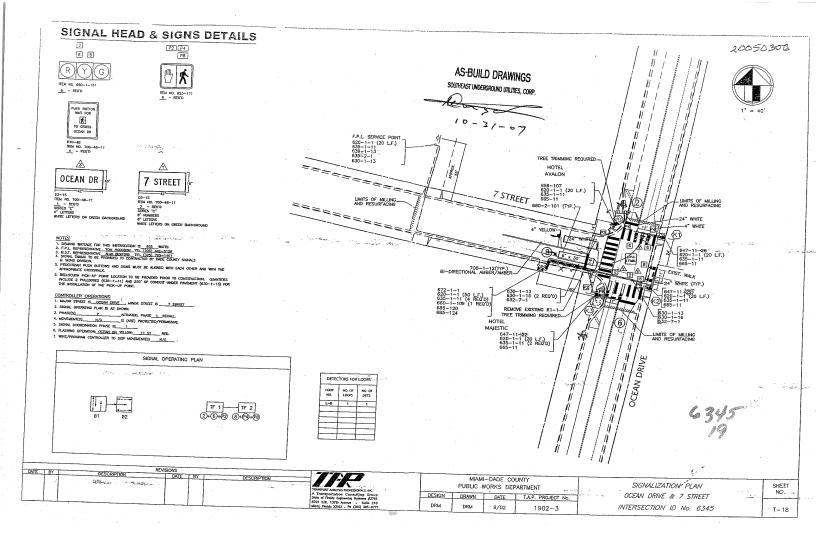
4:09 PM

Print Date: 1/25/2016

Currer	t Time of Day Function			Local	Time of Day Function			* Settings
<u>Time</u> 0000	Function TOD OUTPUTS	<u>Settings *</u> 	<u>Day of Week</u> SuM T W ThF S	<u>Time</u> 0000	Function TOD OUTPUTS	<u>Settings *</u> 	<u>Day of Week</u> SuM T W ThF S	Blank - FREE - Phase Bank 1, Max 1 Blank - Plan - Phase Bank 1, Max 2
								1 - Phase Bank 2, Max 1 2 - Phase Bank 2, Max 2 3 - Phase Bank 3, Max 1 4 - Phase Bank 3, Max 2 5 - EXTERNAL PERMIT 1 6 - EXTERNAL PERMIT 2
								6 - EXTERNAL PERMIT 2 7 - X-PED OMIT 8 - TBA

No Calendar Defined/Enabled

Page 2 of 2



## **APPENDIX D**

## **Pedestrian Flow and LOS for Sidewalks**

#### Highway Capacity Manual 2010

parts of the walkway. In cross-flow locations, the LOS E–F threshold is 13 ft<sup>2</sup>/p, as indicated in the notes for Exhibit 23-1 and Exhibit 23-2.

	Average	Re	lated Measure	S	
LOS	Space (ft <sup>2</sup> /p)	Flow Rate (p/min/ft) <sup>a</sup>	Average Speed (ft/s)	v/c Ratio <sup>b</sup>	Comments
А	>60	≤5	>4.25	≤0.21	Ability to move in desired path, no need to alter movements
В	>40-60	>5-7	>4.17-4.25	>0.21-0.31	Occasional need to adjust path to avoid conflicts
С	>24-40	>7–10	>4.00-4.17	>0.31-0.44	Frequent need to adjust path to avoid conflicts
D	>15-24	>10-15	>3.75-4.00	>0.44-0.65	Speed and ability to pass slower pedestrians restricted
Е	>8–15 <sup>c</sup>	>15-23	>2.50-3.75	>0.65-1.00	Speed restricted, very limited ability to pass slower pedestrians
F	≤8 <sup>c</sup>	Variable	≤2.50	Variable	Speeds severely restricted, frequent contact with other users

**Notes:** Exhibit 23-1 does not apply to walkways with steep grades (>5%). See the Special Cases section for further discussion.

<sup>a</sup> Pedestrians per minute per foot of walkway width.

 $^{b}$  v/c ratio = flow rate/23. LOS is based on average space per pedestrian.

<sup>c</sup> In cross-flow situations, the LOS E–F threshold is 13 ft<sup>2</sup>/p.

LOS	Average Space (ft <sup>2</sup> /p)	Related <u>Measure</u> Flow Rate <sup>a</sup> (p/min/ft) <sup>b</sup>	Comments
A	>530	≤0.5	Ability to move in desired path, no need to alter movements
В	>90-530	>0.5-3	Occasional need to adjust path to avoid conflicts
C	>40-90	>3-6	Frequent need to adjust path to avoid conflicts
D	>23-40	>6-11	Speed and ability to pass slower pedestrians restricted
Е	>11-23 <sup>c</sup>	>11-18	Speed restricted, very limited ability to pass slower pedestrians
F	≤11 <sup>c</sup>	>18	Speeds severely restricted, frequent contact with other users

**Notes:** <sup>a</sup> Rates in the table represent average flow rates over a 5-min period. Flow rate is directly related to space; however, LOS is based on average space per pedestrian.

<sup>b</sup> Pedestrians per minute per foot of walkway width. <sup>c</sup> In cross-flow situations, the LOS E–F threshold is 13 ft<sup>2</sup>/p.

#### **Stairways**

Exhibit 23-3 provides the LOS criteria for stairways.

LOS	Average Space (ft <sup>2</sup> /p)	Related N Flow Rate (p/min/ft) <sup>a</sup>	<u>Measures</u> v/c Ratio <sup>b</sup>	Comments
А	>20	≤5	≤ 0.33	No need to alter movements
В	>17-20	>5-6	>0.33-0.41	Occasional need to adjust path to avoid conflicts
С	>12-17	>6-8	>0.41-0.53	Frequent need to adjust path to avoid conflicts
D	>8-12	>8-11	>0.53-0.73	Limited ability to pass slower pedestrians
E	>5-8	>11-15	>0.73-1.00	Very limited ability to pass slower pedestrians
F	≤5	Variable	Variable	Speeds severely restricted, frequent contact with other users

Notes: <sup>a</sup> Pedestrians per minute per foot of walkway width.

<sup>b</sup> v/c ratio = flow rate/15. LOS is based on average space per pedestrian.

Chapter 23/Off-Street Pedestrian and Bicycle Facilities December 2010 Exhibit 23-1

**Exhibit 23-2** Platoon-Adjusted LOS Criteria for Walkways

Average Flow LOS Criteria for Walkways

n de State - Lochter La del State - Lochter La del State - Lochter

Exhibit 23-3 LOS Criteria for Stairways

Introduction