Permit History - PermitsPlus

Total Permits: 10

Permit No	COMP TYPE	SUB TYPE	DESCRIPTION	STATUS	APPLIED DATE	APPROVED DATE	EXPIRED/FINALED DATE	VAL TOTAL	PARCEL NO	STREET NO	DIR	STREET NAME
BH000120	BCARD	OTH	LOT 35 BLK 15 SUBDIV BISCAYNE BCH	BHISTORY	3/1/1989	5/1/1950	1/1/0001	$8,300.00	32030111370	1211		STILLWATER DR
BD140355	BDEMOPRJ	ALL	Total Demolition of SFR	FINAL	7/25/2014	9/24/2014	3/23/2015	$14,891.00	32030111370	1211		STILLWATER DR
BMS81774	BMISC	OTH	CONTRACTORS BOND B9803389 (ROOF)	FINAL	8/10/1998	2/9/1999	1/1/0001	$0.00	32030111370	1211		STILLWATER DR
BMS99069	BMISC	OTH	REV FEE-B9801480-RPLC W/DCRTV CNCRT	FINAL	12/16/1998	12/18/1998	1/1/0001	$0.00	32030111370	1211		STILLWATER DR
B0002718	BSBUILD		INSTALL CANVAS CARPORT.	CLOSED	4/19/2000	5/24/2000	11/20/2000	$1,800.00	32030111370	1211		STILLWATER DR
B9803389	BSBUILD	OTH	NEW 350 SQ FT GAF BUILT-UP SEE COND	FINAL	8/10/1998	8/19/1998	5/11/1999	$950.00	32030111370	1211		STILLWATER DR
B9902289	BSBUILD	OTH	ERECT WOODEN FENCE/25 LF	CLOSED	3/31/1999	1/1/0001	1/1/0001	$0.00	32030111370	1211		STILLWATER DR
B0000725	BSBUILD		ERECT WOODEN FENCE/76 LF	CLOSED	11/23/1999	11/29/1999	5/27/2000	$780.00	32030111370	1211		STILLWATER DR
BS911182	BSBUILD	OTH	REPLACE 6 WINDOWS & 1 DOOR	CLOSED	4/5/1991	4/5/1991	10/2/1991	$1,000.00	32030111370	1211		STILLWATER DR
B9801480	BSBUILD	OTH	REPAIR FRNT PORCH ENTRNCE & CBS	FINAL	2/20/1998	3/10/1998	5/11/1999	$10,000.00	32030111370	1211		STILLWATER DR

PERMIT NUMBER

B98-01480

ADDRESS

A

21

Footing uplifting calculations:

Area considered: $\frac{10.5}{2} \times 9.56 \times 45.3^{\#}/SF = 2274^{\#}$

$2274 \times 1.5 = 3411^{\#}$ uplifting.

Dead loads: $3' \times 3' \times 2' \times 150^{\#}/CF$ (Footings) $= 2{,}700^{\#}$

Roof weight: $\frac{10.5}{2} \times 9.56 \times 10^{\#}/SF = 500^{\#}$

10" column weight: $0.785 \times 8 \times 150^{\#}/CF = 942^{\#}$

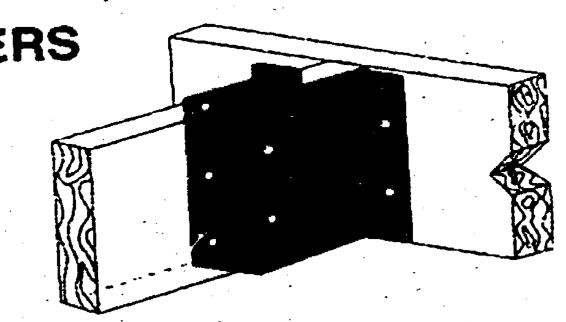
10' - 3½ x 3½ x ¼ steel column $= 100^{\#}$

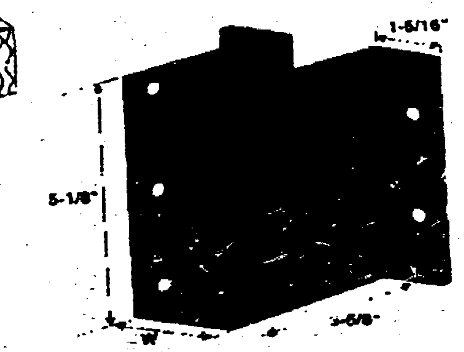
$4242^{\#}$

$4242^{\#} > 3411^{\#}$ OK

3-17-98

DEEP SEATED U-TYPE JOIST HANGERS

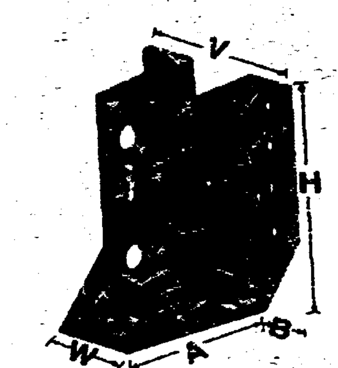


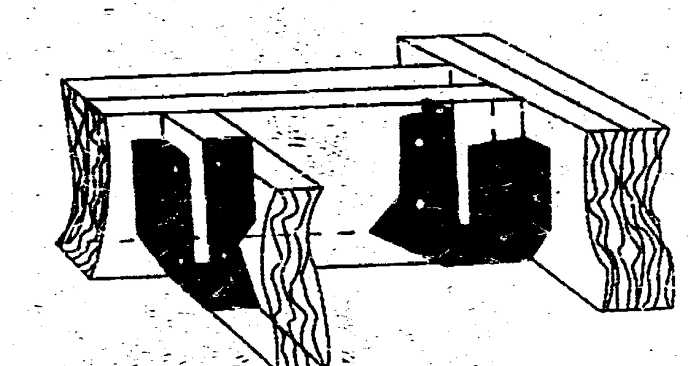


- Easy and fast installation
- Perfect for DIY Deck/Fence application
- 16 gauge galvanized steel
- **SBCCI PST&ESI No. 9408A**
- **Dade County Approval No. 97-0421.05**

Product Code	Description	Qty/ Box	W	Fasteners: Nails in Header	Fasteners: Nails in Joist	Design Loads (lbs): Normal	Design Loads (lbs): Max	Design Loads (lbs): Uplift
U2-4	2XU-Hanger	30	1-5/8"	6-16d	5-10d	870	870	585
U4-4	4XU-Hanger	30	3-5/8"	6-16d	5-10d	870	1085	585

JOIST HANGERS, U-TYPE





- Fast safe framing
- Eliminates toenailing
- Prevents split joists
- Develops full joist capacity
- 18 gauge Galvanized Steel
- May be ordered with reverse flanges
- **SBCCI PST& ESI No. 94139**
- **Dade County Approval No. 97-0205.03**

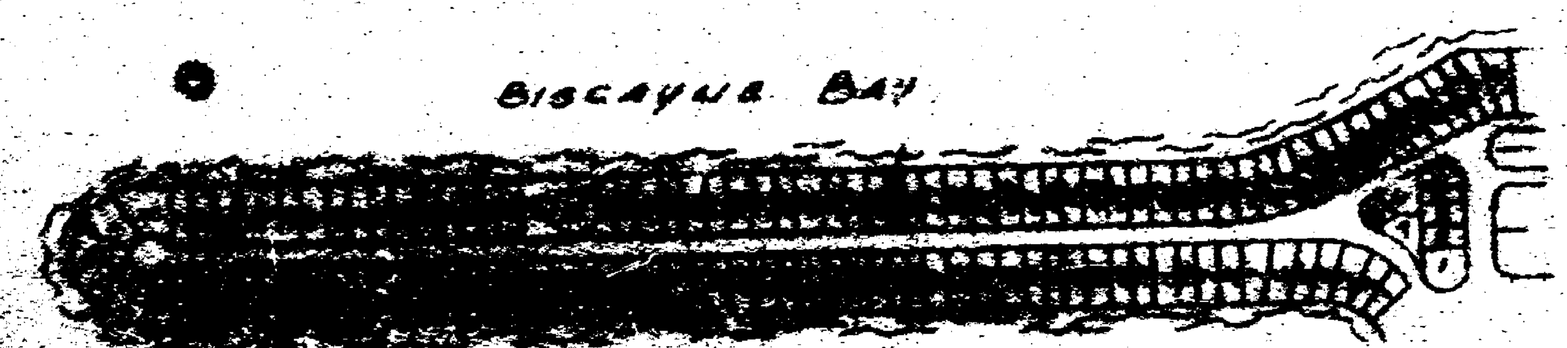
16d Nails

Product Code	Joist Size	Qty/ Box	Wgt/ Box	Dimensions: A	B	H	W	V	Fasteners: Girder/ Header	Truss/ Joist	Design Loads (lbs): Normal 100%	Snow 115%	Roof 125%	Wind 133%	Wind 160%	Uplift 133%	Uplift 160%
UH 24	2X4	100	33	3"	1"	3-1/4"	1-5/8"	3-3/4"	4-16d	2-16d	580	670	725	770	770	360	360
UH 26	2X6	50	33	3"	1"	5-1/4"	1-5/8"	3-3/4"	8-16d	3-16d	1160	1335	1450	1540	1540	720	720
UH 28	2X8	50	32	3"	1"	7-1/4"	1-5/8"	3-3/4"	12-16d	4-16d	1740	1865	1865	1865	1865	735	735
UH 210/212	2X10/12	50	28	3"	1"	9-1/4"	1-5/8"	3-3/4"	16-16d	5-16d	2320	2475	2475	2475	2475	900	900
UH 2-26	(2 Ea) 2X6	50	23	3"	1"	4-1/2"	3-1/8"	5-1/4"	8-16d	3-16d	1140	1310	1425	1520	1540	700	700
UH 2-28	(2 Ea) 2X8	50	31	3"	1"	6-1/2"	3-1/8"	5-1/4"	12-16d	4-16d	1740	1865	1865	1865	1865	930	930
UH 2-210	(2 Ea) 2X10	50	33	3"	1"	8-1/2"	3-1/8"	5-1/4"	16-16d	5-16d	2320	2475	2475	2475	2475	1160	1160
UH 36	3X6	50	23	3"	1"	4-3/4"	2-5/8"	4-3/4"	8-16d	3-16d	1160	1335	1450	1540	1540	700	700
UH 38	3X8	50	28	3"	1"	6-3/4"	2-5/8"	4-3/4"	12-16d	4-16d	1740	1865	1865	1865	1865	930	930
UH 310/312	3X10/12	50	33	3"	1"	8-3/4"	2-5/8"	4-3/4"	16-16d	5-16d	2320	2475	2475	2475	2475	1160	1160
UH 46	4X6	50	23	3"	1"	5-1/4"	3-5/8"	5-3/4"	10-16d	3-16d	1450	1670	1815	1925	1925	700	700
UH 48	4X8	50	33	3"	1"	7-1/4"	3-5/8"	5-3/4"	14-16d	4-16d	2030	2175	2175	2175	2175	930	930
UH 3-210	(3Ea) 2X10	1	1	3"	1"	8-7/16"	4-5/8"	6-5/8"	18-16d	6-16d	2610	2785	2785	2785	2785	1395	1395

20d Nails

Product Code	Joist Size	Qty/ Box	Wgt/ Box	Dimensions: A	B	H	W	V	Fasteners: Girder/ Header	Truss/ Joist	Design Loads (lbs): Normal 100%	Snow 115%	Roof 125%	Wind 133%	Wind 160%	Uplift 133%	Uplift 160%
UH 24	2X4	100	33	3"	1"	3-1/4"	1-5/8"	3-3/4"	4-20d	2-20d	700	805	875	930	990	525	525
UH 26	2X6	50	33	3"	1"	5-1/4"	1-5/8"	3-3/4"	8-20d	3-20d	1400	1610	1750	1860	1980	785	785
UH 28	2X8	50	32	3"	1"	7-1/4"	1-5/8"	3-3/4"	12-20d	4-20d	2100	2415	2625	2650	2650	1110	1110
UH 210/212	2X10/12	50	28	3"	1"	9-1/4"	1-5/8"	3-3/4"	16-20d	5-20d	2800	3220	3270	3270	3270	1440	1440
UH 2-26	(2 Ea) 2X6	50	23	3"	1"	4-1/2"	3-1/8"	5-1/4"	8-20d	3-20d	1140	1310	1425	1520	1825	1200	1200
UH 2-28	(2 Ea) 2X8	50	31	3"	1"	6-1/2"	3-1/8"	5-1/4"	12-20d	4-20d	2100	2415	2625	2650	2650	1600	1600
UH 2-210	(2 Ea) 2X10	50	33	3"	1"	8-1/2"	3-1/8"	5-1/4"	16-20d	5-20d	2800	3220	3270	3270	3270	2000	2000
UH 36	3X6	50	23	3"	1"	4-3/4"	2-5/8"	4-3/4"	8-20d	3-20d	1400	1610	1750	1860	1980	1200	1200
UH 38	3X8	50	28	3"	1"	6-3/4"	2-5/8"	4-3/4"	12-20d	4-20d	2100	2415	2625	2650	2650	1600	1600
UH 310/312	3X10/12	50	33	3"	1"	8-3/4"	2-5/8"	4-3/4"	16-20d	5-20d	2800	3220	3270	3270	3270	2000	2000
UH 46	4X6	50	23	3"	1"	5-1/4"	3-5/8"	5-3/4"	10-20d	3-20d	1750	2015	2190	2330	2475	1200	1200
UH 48	4X8	50	33	3"	1"	7-1/4"	3-5/8"	5-3/4"	14-20d	4-20d	2450	2820	3065	3095	3095	1600	1600
UH 3-210	(3Ea) 2X10	1	1	3"	1"	8-7/16"	4-5/8"	6-5/8"	18-20d	6-20d	3150	3625	3940	4190	5040	2400	2400

BISCAYNE BAY



BISCAYNE BAY

LOCATION

BOUNDARY SURVEY

SKETCH OF SURVEY

OF

LOT 3? - BLOCK 1?

BISCAYNE BEACH
2ND ADDITION
MIAMI-BEACH, MIAMI-DADE COUNTY FLORIDA

AS RECORDED IN PLAT BOOK ?? AT PAGE 39 OF
THE PUBLIC RECORDS OF MIAMI-DADE COUNTY FLORIDA

I hereby certify: That the attache[d] sketch of survey represented herein is true and correct to the best of [my] knowledge and belief, and meet[s] the "Minimum Technical Standards" as set forth by the Florida Bo[ard] of Professional Land Surveyors, pursu[ant] to section 472.027, Florida S[tat]utes.

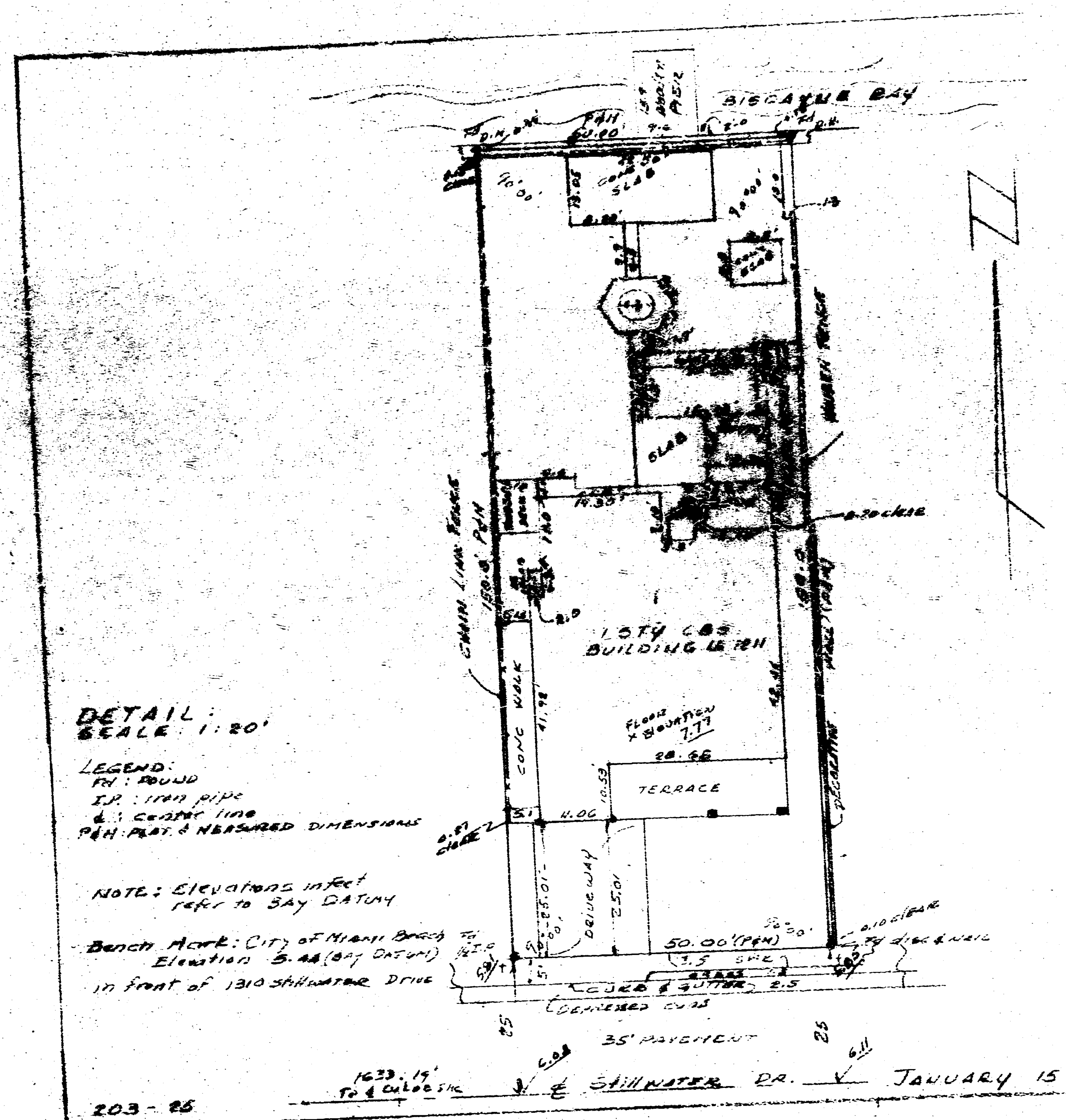
Wenceslao Ortega

WENCESLAO ORTEGA
PROFESSIONAL SURVEYOR AND MAPPER No 2091
STATE OF FLORIDA
8500 SW 8TH ST SUITE 230
MIAMI, FLORIDA 33144

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL
RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

-1998

2 - 98

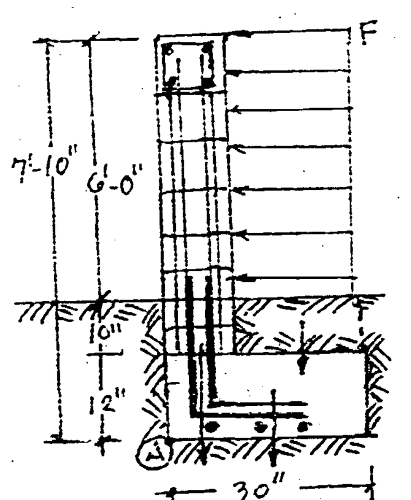

BISCAYNE BAY
1 STY CBS BUILDING
FLOOR ELEVATION 7.77
TERRACE
SLAB
CONC WALK
DRIVE WAY
CHAIN LINK FENCE
CURB & GUTTER
DEPRESSED CURB
35' PAVEMENT
50.00' (P&M)
STILLWATER DR.
JANUARY 15
1633.19' To ℄ CULDESAC
DETAIL:
SCALE 1:20'
LEGEND:
FD: FOUND
I.P.: iron pipe
℄: center line
P&M: PLAT & MEASURED DIMENSIONS
NOTE: Elevations in feet refer to BAY DATUM
Bench Mark: City of Miami Beach
Elevation 5.44 (BAY DATUM)
in front of 1310 Stillwater Drive
203 - 26

21

B9801480

Concrete fence — M.Valdes-Linares, P.E.

Design wind pressure — ASCE - 88 - Chapter 6



DEAD LOADS:

Tie beam: 0.67x1x150#/CF = 100#/LF
Block parapet: 6.83x40x1 = 273#/F
Footing: 2.5x1x150#/CF = 375#/LF
Earth: 1.83x1x100#/CF = 183#/LF
TOTAL: 931#/LF

$q_z = 0.00256\,K_1\,(IV)^2$

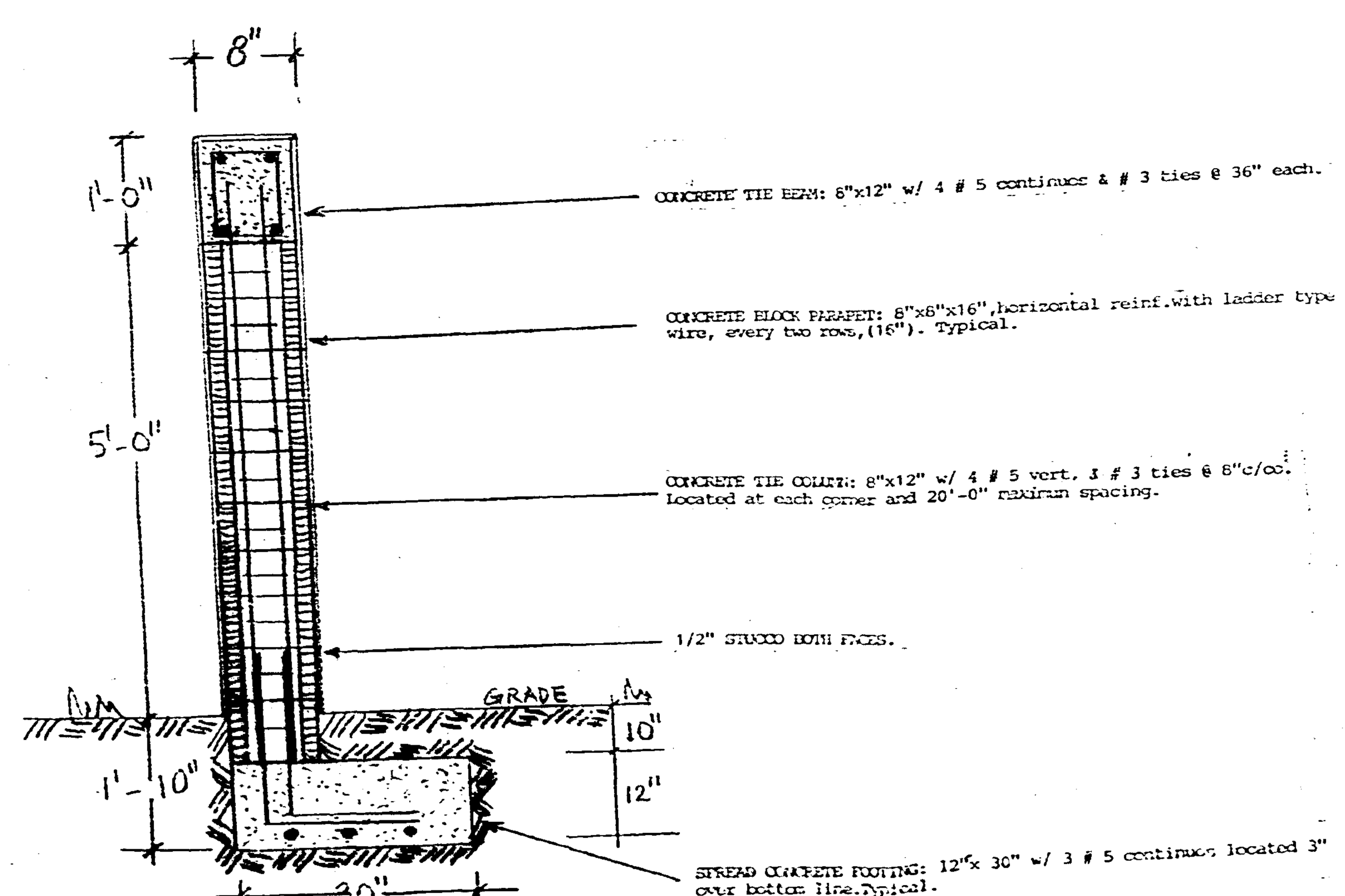
$K_1 = 0.80$ (Table 6) V = 110 MPH
I = 1.05 (Table 5) Exposure: "C"

$q_z = 0.00256 \times 0.80 \times (110 \times 1.05)^2 = 27.32$#/SF

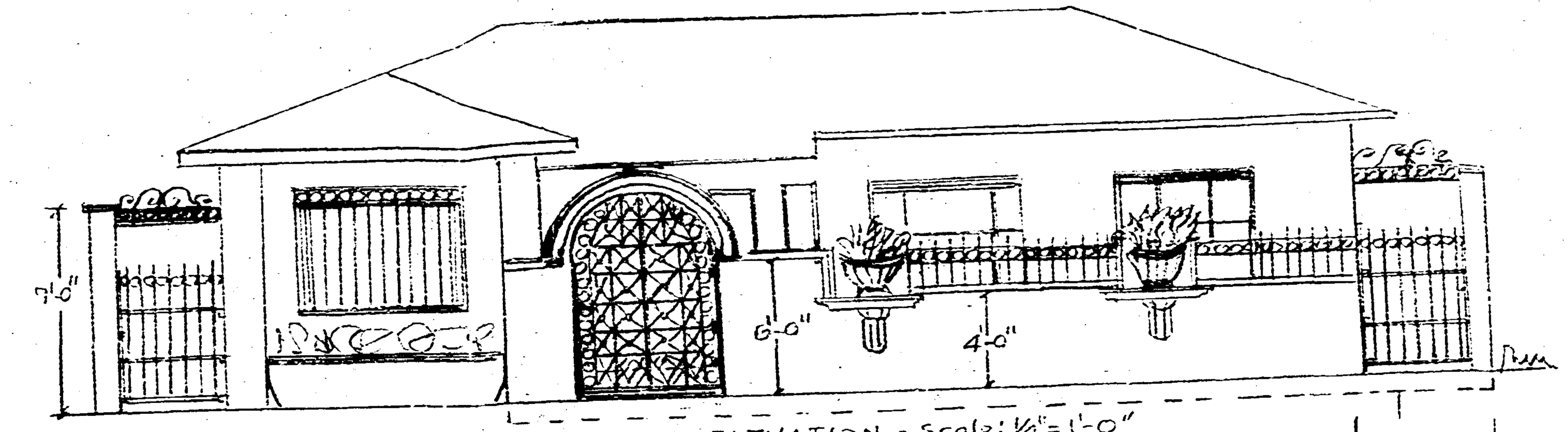
Design wind pressure: As per table 4:

$F = q_z\,G_h\,C_f\,A_f$ — $G_h = 1.32$ (Table 8) $C_f = 1.3$ (Table 12)
$A_f = 1.00$ SF $q_z = 27.32$#/SF

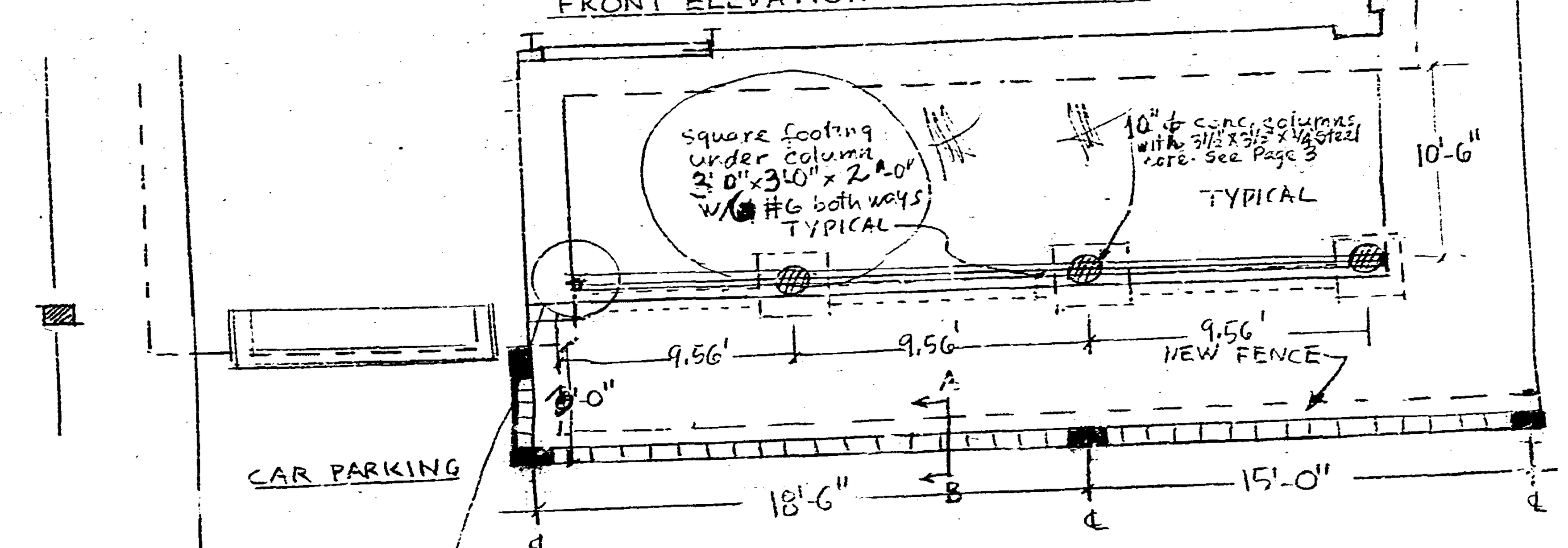
Then:
F = 27.32x1.32x1.3x1 = 46.8 ≈ 47#/SF
Taking moments respect to (A):
F x 7.833 = 47 x 7.833 = 368# — (WIND LOAD)
(100x0.33)+(273x0.33)+(375x1.25)+(183x1.58) = 880# (DEAD LOAD)
368# x 150% = 552# < 880# - OVERTURNING - OK
(2309.4 SFBC)



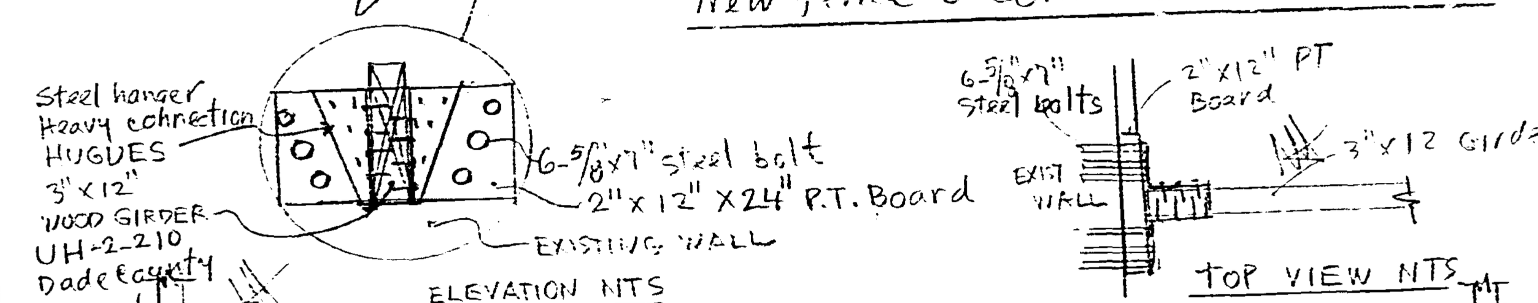
A-B FENCE SECTION - NTS -

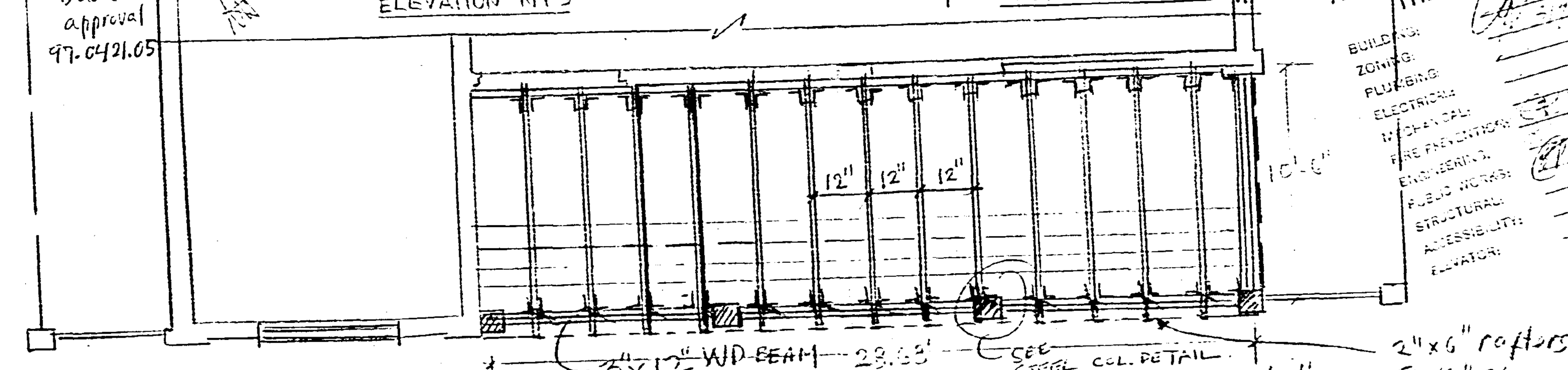


FRONT ELEVATION - Scale: 1/4"=1'-0"



New fence & columns - Scale: 1/4"=1'-0"





Front Plant View - Existing house - Scale: 1/4"=1'-0"

NOTES:
ALL Rotten or broken rafters to be replaced. Existing square columns to be change to rounds 10" diam.

Structural notes:
1.- One # 5 vertical shall be installed at both sides of each openings, like doors and windows.
2.- Two # 5 x 3'-0" long, bent at center 90 degrees, shall be installed in the tie beam at each corner.
3.- Construction works shall be in accordance with the S.F.B.C. and Local Regulations.
4.- General Contractor to coordinate works with other trades.
5.- Measuring at site shall be prior to start construction works and will be mandatory to plans and shop drawings dimensions.
6.- If any demolition will be necessary to do on the existing building, extreme care shall be taken by shoring the roof before starting demolition.
7.- Precast concrete joist manufacturer shall be responsible for plans and shop drawings necessary, also for erection and mounting.
8.- Steel strength shall be 60,000 psi. Concrete strength: Footings: 2,500 psi, Slab on ground: 3,000 psi. Columns and tie beams: 3,000 psi.
9.- Provide a polyethylene film (0.006) under the slab on ground over a well compacted fill.
10.- Firestoping shall be provided to cut off all concealed draft spaces both vertical and horizontal.
11.- P.T. 1"x2" furrings shall be installed spaced 16" c/oc with at least one cut nail in alternate course of blocks.
12.- Soffit ventilation shall be in an intermittent vent with breaks not greater than 4 feet. Shall be clear of obstructions including insulation to allow proper air flow.
13.- Drip strip on eaves and gable shall have a roof flange not less than 2" wide. The metal profile shall be nailed with a minimum 12 gage annular ring shank nail at 4" c.c. with non ferrous nail. The bottom of the drip edge shall have a minimum of 1/2" clearance from the structure.
14.- Drip size will be 3"x3" with not less than 26 galvanized or stainless steel.
15.- Plywood roof sheating shall be 19/32" with a face grain perpendicular to supports. Plywood panels shall be nailed to supports with 8-d common nails. Nails spacing shall be 6"c/c at panel edges, and 4" at gable edge ends.

FIRE STOPPING NOTE:
All penetrations through walls, ceilings, floors and other barriers, resulting from the passage of pipes, conduits, bus ducts, cables, wires, air ducts, pneumatic ducts and penetrations from similar building service equipment installed in connection with this project shall be protected by approved materials or devices meeting the acceptance criteria of the American Society of Testing Materials (ASTM-E814)

CONCRETE REINFORCING NOTES:
The tie beam to have 4 # 3 ties at 12" OC at each side of corners ...

ALL WOOD No 2 SOUTHERN PINE

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING: 3-9-98

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:
PLANS ROUTER:

Revision: 2-29-92

Manuel Valdes Linares
Professional Engineer
Civil-Structural
State of Florida
Lic. 14863
11980 S.W. 43 St.
Miami, FL 33175
Phone (305) 553-6622

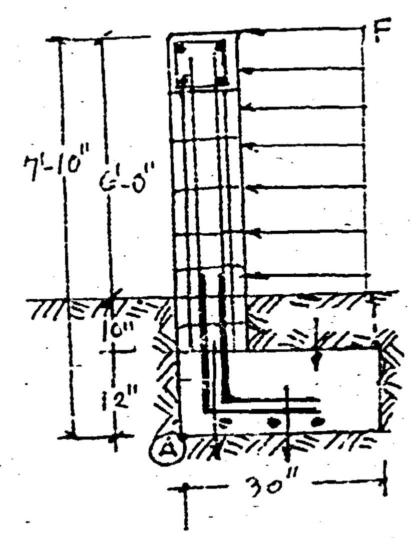
Porch Rafter repair - & New conc. fence --		
SCALE: As shown	APPROVED BY: M.VALDES-LINARES, P.E.	DRAWN BY
DATE: 2-6-98		REVISED MVL
1211 Stillwater Drive		Miami Beach
owner: Arturo Diaz		DRAWING NUMBER 1

21

concrete fence

M. Valdes-Linares, P.E.

Design wind pressure ASCE - 88 - Chapter 6



DEAD LOADS:

Tie beam: $0.67 \times 1 \times 150$ #/CF = 100 #/LF
Block parapet: $6.83 \times 40 \times 1$ = 273 #/LF
Footing: $2.5 \times 1 \times 150$ #/CF = 375 #/LF
Earth: $1.83 \times 1 \times 100$ #/CF = 183 #/LF
TOTAL: 931 #/LF

$q_z = 0.00256\, K_1 (IV)^2$

$K_1 = 0.80$ (Table 6) V = 110 MPH
$I = 1.05$ (Table 5) Exposure: "C"

$q_z = 0.00256 \times 0.80 \times (110 \times 1.05)^2 = 27.32$ #/SF

Design wind pressure: As per table 4:

$F = q_z G_h C_f A_f$ $G_h = 1.32$ (Table 8) $C_f = 1.3$ (Table 12)
$A_f = 1.00$ SF $q_z = 27.32$ #/SF

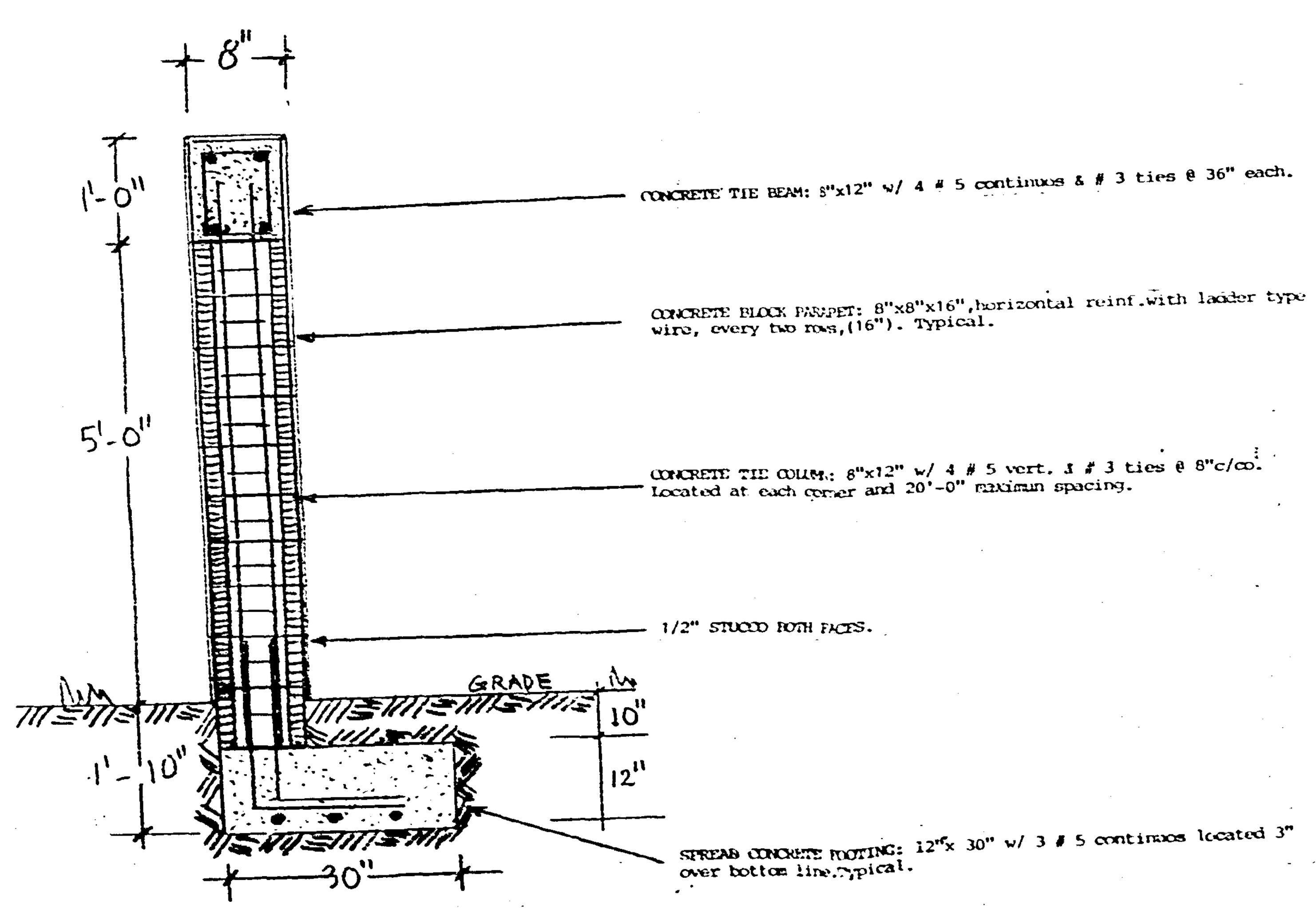
Then:
$F = 27.32 \times 1.32 \times 1.3 \times 1 = 46.8 \approx 47$ #/SF

Taking moments respect to (A):

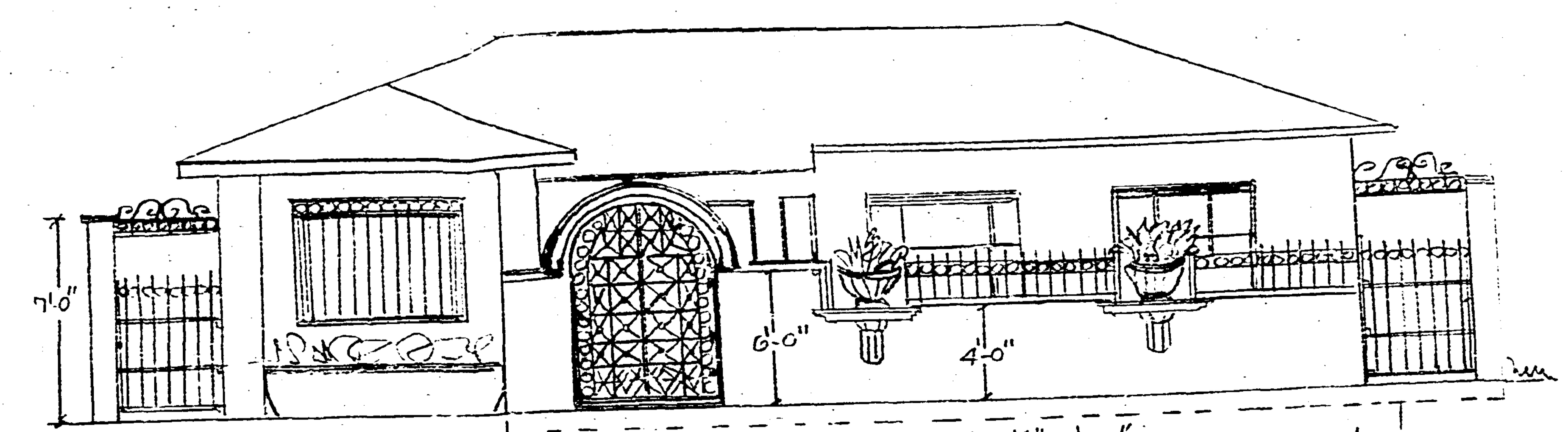
$F \times 7.833 = 47 \times 7.833 = 368$ # — (WIND LOAD)

$(100 \times 0.33) + (273 \times 0.33) + (375 \times 1.25) + (183 \times 1.58) = 880$ # (DEAD LOAD)

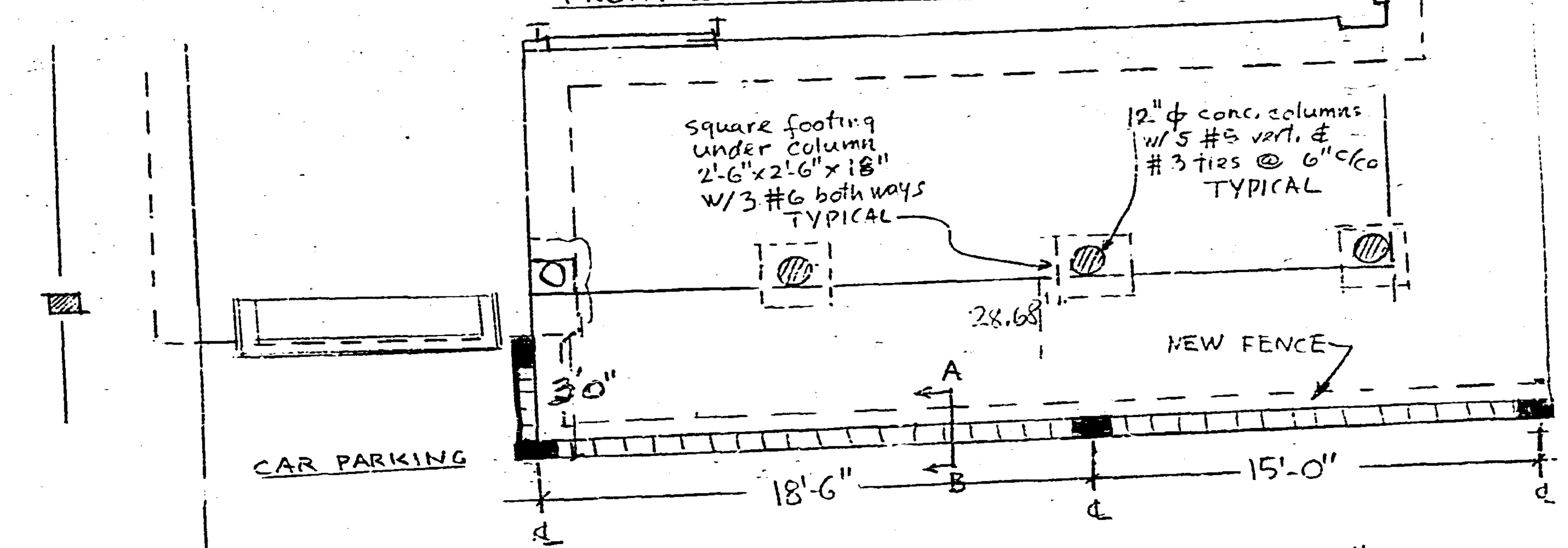
368 # × 150% = 552 # < 880 # - OVERTURNING - OK
(2309.4 SFBC)



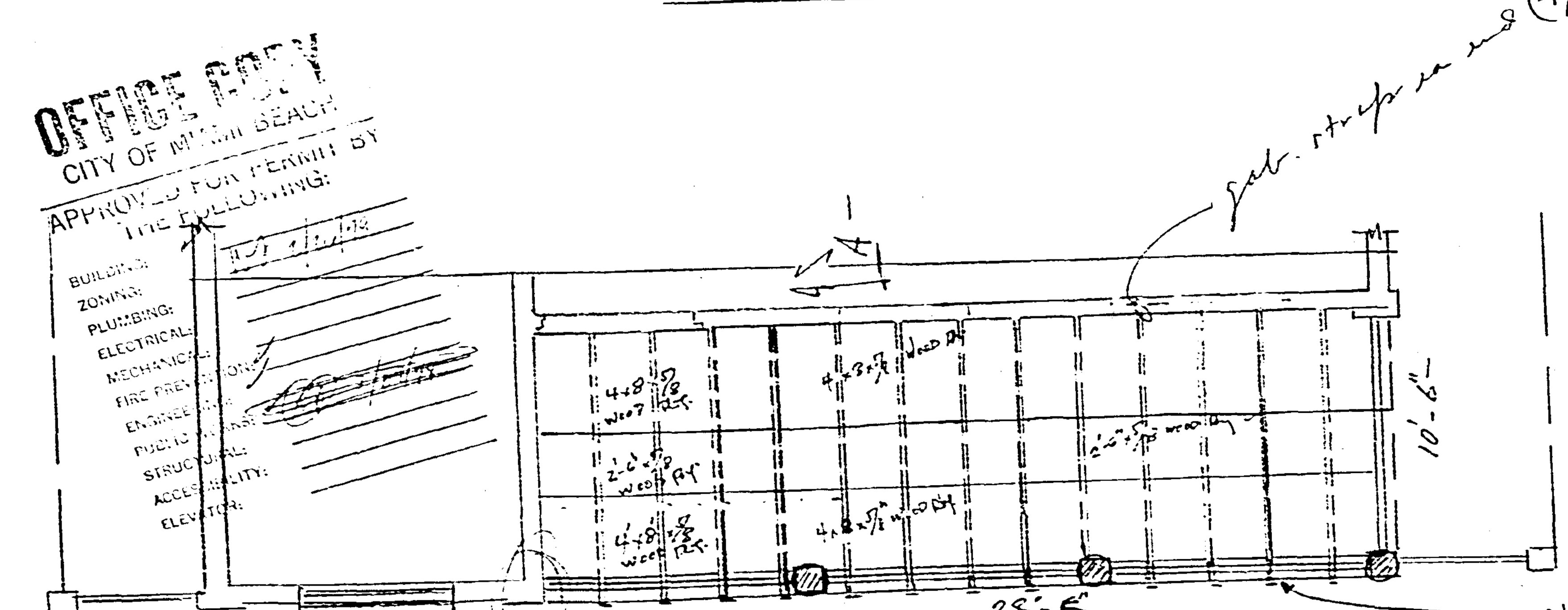
A-B FENCE SECTION - NTS -



FRONT ELEVATION - Scale: 1/4" = 1'-0"



New fence & columns - Scale: 1/4" = 1'-0"

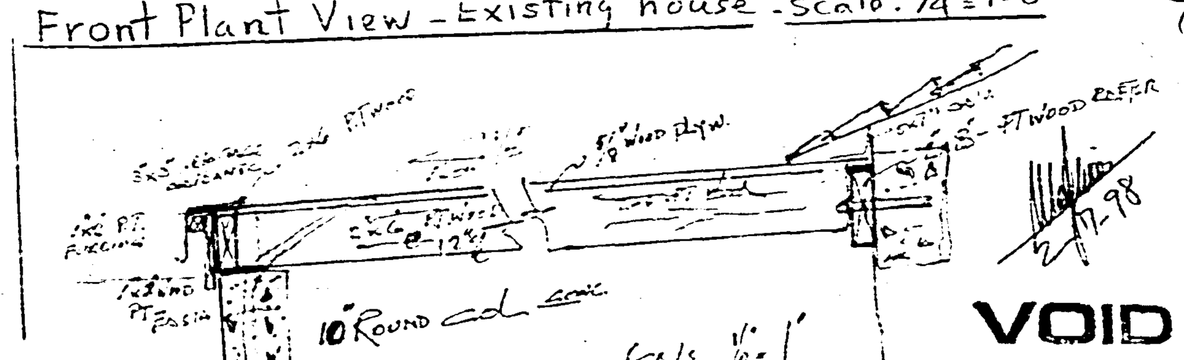


Front Plant View - Existing house - Scale: 1/4" = 1'-0"

2"x6" rafters @ 24" c/co (EXISTING) To be REPLACED the ROTTEN or BROKEN.

NOTES:

ALL Rotten or broken rafters to be replaced.
Existing square columns to be change to rounds 12" diam.



Soil statement:
The soil is sand and rocks with a minimum bearing capacity of 2,000 p.s.f., by visual inspection, however if at the excavation time different soil characteristic are encountered, the works shall be stopped and the Engineer shall be call.

Soil compaction:
Fill and backfill to be compacted to a minimum of 90% of the maximum density at optimum moisture as determined by tests.

Roof framing notes:
Drop tie beams over window openings with 2 # 5 at bottom, additional extend lintels 8" each end of opening. Provide # 3 ties at 12" for openings larger than 6'-0".
Provide 1 # 5 vertical at each side of openings.

Slab on fill:
Shall be placed on clean non organic soil, throughly moistened before concrete is pouring. Shall be pouring in a checkerboard sequence each fragment of which is not exceed a maximum area of 625 s.f. or 30 l.f. in any direction.

Structural notes:
1.- One # 5 vertical shall be installed at both sides of each openings, like doors and windows.
2.- Two # 5 x 3'-0" long, bent at center 90 degrees, shall be installed in the tie beam at each corner.
3.- Construction works shall be in accordance with the S.F.B.C. and Local Regulations.
4.- General Contractor to coordinate works with other trades.
5.- Measuring at site shall be prior to start construction works and will be mandatory to plans and shop drawings dimensions.
6.- If any demolition will be necessary to do on the existing building, extreme care shall be taken by shoring the roof before starting demolition.
7.- Precast concrete joist manufacturer shall be responsible for plans and shop drawings necessary, also for erection and mounting.
8.- Steel strengh shall be 60,000 psi
Concrete strengh: Footings: 2,500 psi. Slab on ground: 3,000 psi. Columns and tie beams: 3,000 psi.
9.- Provide a polyethylene film (0.006) under the slab on ground over a well compacted fill.
10.- Firestoping shall be provided to cut off all concealed draft spaces both vertical and horizontal.
11.- P.T. 1"x2" furrings shall be installed spaced 16" c/co with at least one cut nail in alternate course of blocks.
12.- Soffit ventilation shall be in an intermitent vent with breaks not greater than 4 feet. Shall be clear of obstructions including insulation to allow proper air flow.
13.- Drip strip on eaves and gable shall have 3 roof flange not less than 2" wide. The metal profile shall be nailed with a minimum 12 gage annular ring shank nail at 4" o.c. with non ferrous nail. The bottom of the drip edge shall have a minimum of 1/2" clearance from the structure.
14.- Drip size will be 3"x3" with not less than 26 galvanized or stainless steel.
15.- Plywood roof sheating shall be 19/32" with a face grain perpendicular to supports. Plywood panels shall be nailed to supports with 8-d common nails. Nails spacing shall be 6"o/c at panel edges, and 4" at gable edge ends.

FIRE STOPPING NOTE:
All penetrations through walls, ceilings, floors and other barriers, resulting from the passage of pipes, conduits, bus ducts, cables, wires, air - ducts, pneumatic ducts and penetrations from similar building service equipment installed in connection with this project shall be protected by approved materials or devices meeting the acceptance criteria of the American Society of Testing Materials (ASTM-E814)

CONCRETE REINFORCING NOTES:
The tie beam to have 4 # 3 ties at 12" OC at each side of corners, and # 3 ties at 48" OC. elsewhere.
Horizontal reinforcing on tie beam shall run straight and lapping splices not less than 20".
Provide 2 # 5 bent 90 degrees and extended 30" each way from corner, typical at each corner.
Provide horizontal joint reinforcing every second course of conc. block wall (16" vertical).
Standard trusses shall be extended 4" into tie column or tie to structural column by approved methods.

STRUCTURAL NOTES:
Compressive strength shall be: 2,500 psi for footings, 3,000 psi for slab on fill, columns, tie beams for concrete poured.
Reinforcing steel shall be detailed and fabricated according with the Manual of Standard practice for detailing reinforced concrete structures and shall be A-615 grade 60. Overlap continuos reinforcing 30 bar diameters in beams, 36 diameters in slabs. Lap bottom steel over supports and top steel at midspan. Hook all discontinuos top reinforcing, turn all corners with 2 # 5x4'- bars. Clear cover for reinforcing shall be: Formed faces in contact with earth....2", unformed faces..3". Slabs....3/4".
Slab on fill shall be placed on clean, non organic soil throughly moistened immediately before concrete is poured. Provide polyethilene membrane for all slab on grade. (0.006 in visquen).
Structural steel shall conform to ASTM, A-36, bolts to be A-107. Weldings to be E-70XX.
Soil compaction: Fill and backfill to be compacted to a minimum of 95% of the maximum density at optimum moisture as determined by the Standard Proctor Test.

Concrete notes:
All concrete work is designed on the basis of strength design, in accordance with ACI 318, Building Code Requirements for Reinforced Concrete. All concrete work shall be constructed in strict accordance with ACI 318.
Concrete work shall be proportioned in accordance with ACI 301, Specifications for Structural Concrete and ACI 211.1, Recommended Practice for Selection Proportions for Normal Weight Concrete, to produce a 28 days compressive strenght in accordance with design plans.
Concrete slump limitation: 4" minimum and 6" maximum.
No water shall be added to the concrete mix at the job site without approval of the Architect or the Engineer.
All concrete shall be consolidated through use of mechanical vibrators.
All splices in reinforcing bars shall be lapped 40 diam. Reinforcing bars shall be continuos at corners.
Pour floor slabs on grade in alternate bays, with 1/4" premoulded fillers between sections or cut all control joints in floor slab within 24 hours of pouring concrete.
When concrete pumped a minimum inside diameter pump and pipe shall be used for 3" diameter.
All concrete pumped shall be designed accordingly and shall comply with slump limitations stated herein above and with ACI 301.

VOID

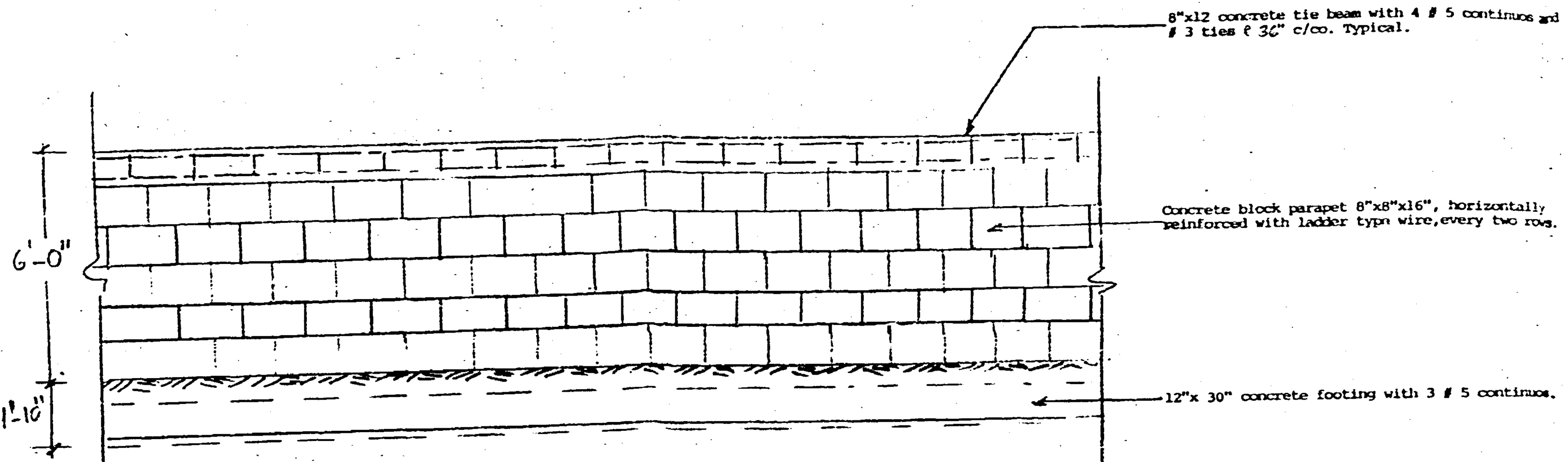
VOID VOID

VOID

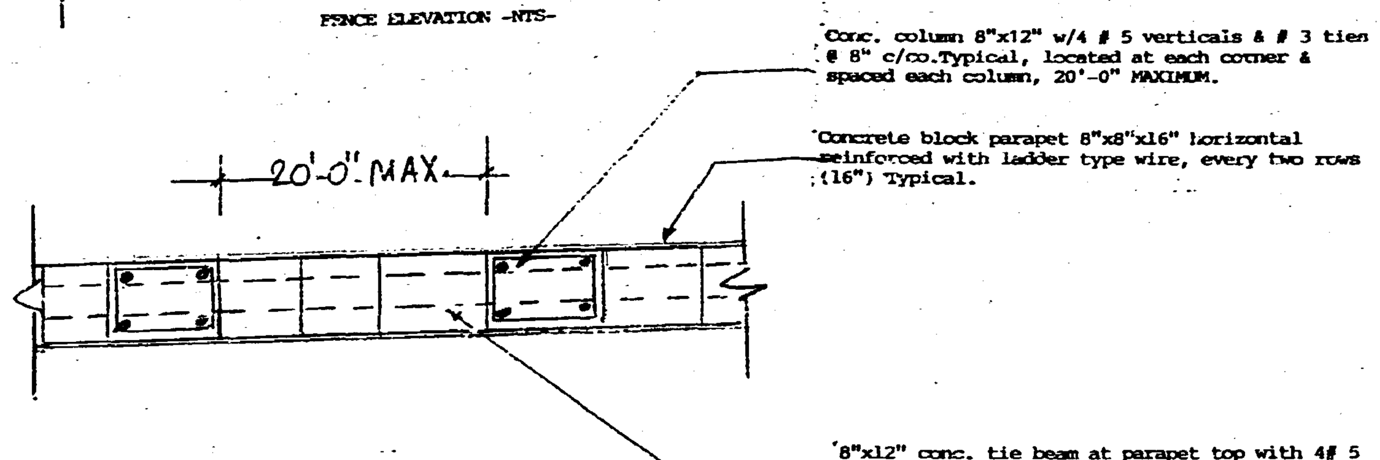
2-17-98

Manuel Valdes Linares
Professional Engineer
Civil-Structural
State of Florida
Lic. 14393
11980 S.W. 43 St.
Miami, FL 33175
Phone: (305) 553-0022

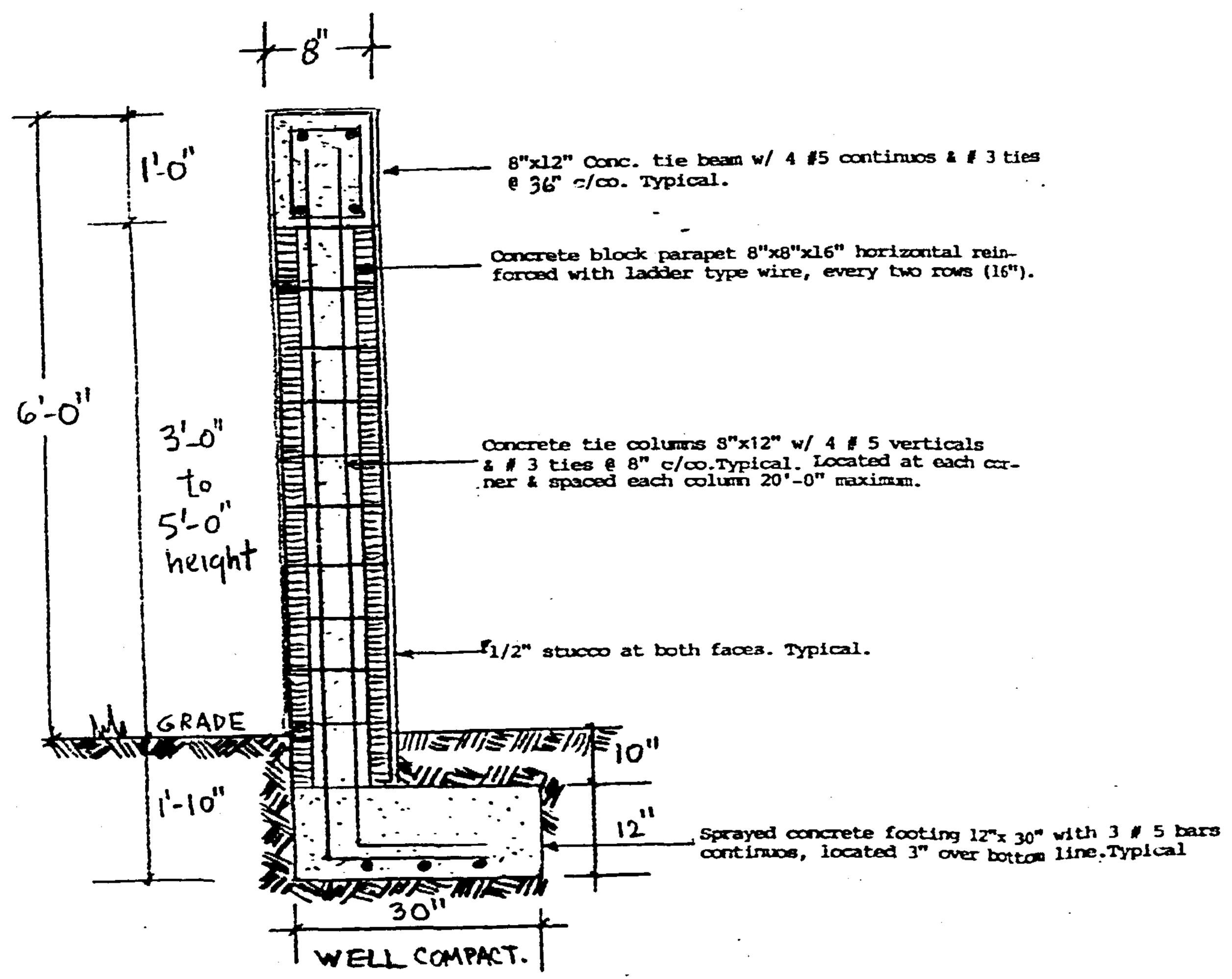
Porch Rafter repair - & New conc. fence --		
SCALE: As shown	APPROVED BY: M.VALDES-LINARES, P.E.	DRAWN BY
DATE: 2-6-98		REVISED MVL
1211 Stillwater Drive		Miami Beach
owner: Arturo Diaz		DRAWING NUMBER 1



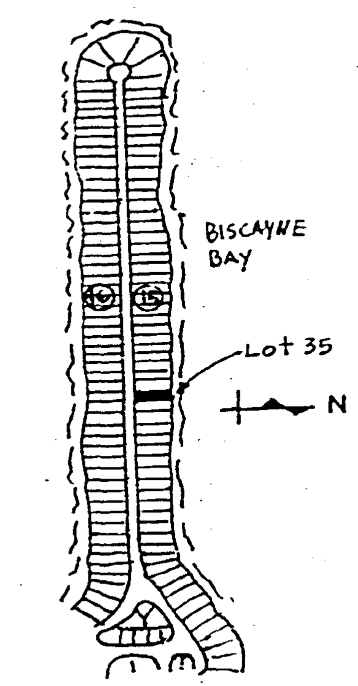
FENCE ELEVATION -NTS-



FENCE PLANT VIEW -NTS-



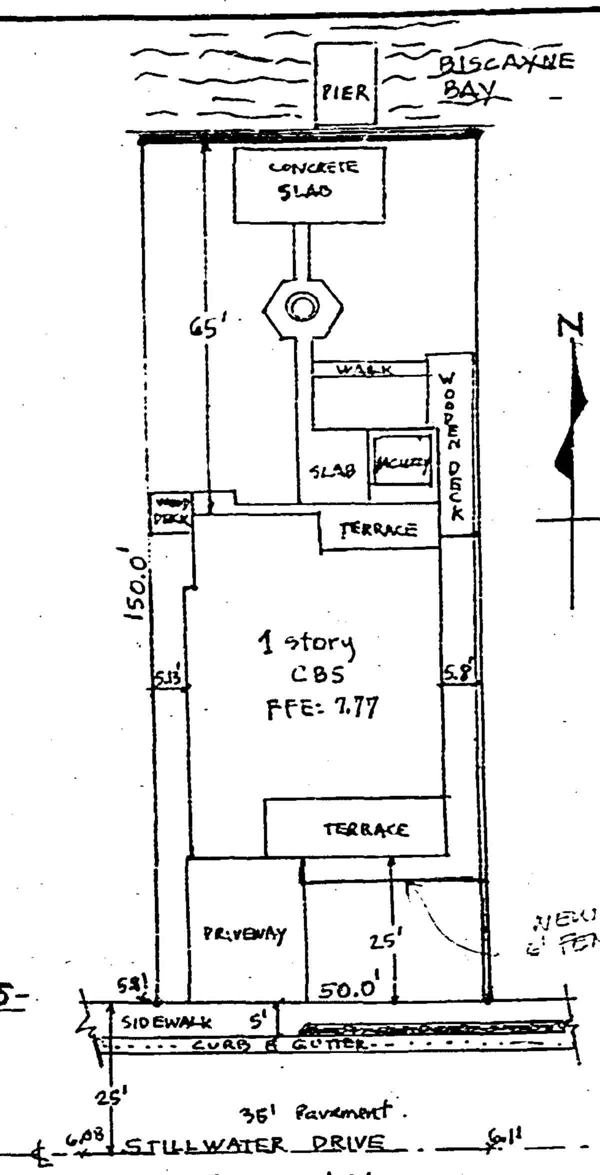
FENCE SECTION -NTS-



LOCATION PLAN-NTS-

LEGAL DESCRIPTION:

Lot 35, Block 15, Biscayne Beach 2 addition, as recorded in Plat Book 46, Page 39 of the Public Records of Miami-Dade County,Fl.



PLOT PLAN-Scale.1:20

JOB COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:
3-9-98
BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS: 3/09/98
STRUCTURAL:
ACCESSIBILITY:
ELEVATOR:

21

2-9-98

Manuel Valdes Linares
Professional Engineer
Civil-Structural
State of Florida
Lic. 14393
11980 S.W. 43 St.
Miami, FL 33175
Phone: (305) 553-0022

Porch Rafter repair & NEW conc. fence		
SCALE: As shown / DATE: 1-30-98	APPROVED BY: M.Valdes-Linares,P.E.	DRAWN BY / REVISED MVL
1211 Stillwater Drive M.B.		
OWNER: Arturo Diaz Artiles.		DRAWING NUMBER 2

WIND LOADS CALCULATIONS: M. VALDES-LINARES, P.E.

1211 Stillwater Dr - M. Beach — 2-29-98

WIND VELOCITY PRESSURE: $q_1 = 0.00256\,K_1\,(IV)^2$

Where: $K_1 = 0.80$ (Table 6) $I = 1.05$ (Table 5) Exposure: "C"

V = 110 MPH - Height average: 8'-3".

Then: $q_1 = 0.00256 \times 0.80 \times (1.05 \times 110)^2 = \underline{27.32\ PSF}$

Roof overhang: $C_p = 0.80$ (Fig 2)

windward: $C_p = -0.70$ Leeward: C_p: -0.70 (Main)

Flat roof coef. $C_p = -0.7 + (0.8) = -1.5$

$P = q\,G_h\,C_p - q_h\,(GC_{pi})$ (Table 4)

NORMAL:

Wall { Windward: $p = 27.32 \times 1.35 \times 0.8 - 27.32(\pm 0.25)$ < +36.3 / +22.6
Wall { Leeward: $p = 27.32 \times 1.35 \times (-0.5) - 27.32(\pm 0.25)$ < -11.6 / -25.3

Roof { windward: $p = 27.32 \times 1.35 \times (-0.32) - 27.32(\pm 0.25)$ < -4.9 / -18.6
Roof { Leeward: $p = 27.32 \times 1.35 \times (-0.7) - 27.32(\pm 0.25)$ < -18.9 / -32.6

PARALLEL:

Wall: $p = 27.32(1.35)(-0.7) - 27.32(\pm 0.25)$ < -18.9 / -32.6
Roof: $p = 27.32(1.35)(-0.7) - 27.32(\pm 0.25)$ < -18.9 / -32.6

Roof overhang: $p = 27.32(1.35)(-0.8) = -29.5$

Flat roof: $p = 27.32(1.35)(-1.5) = -55.3$

Dead loads: 10#/SF Live loads: 30#/SF

WIND NET UPLIFTING LOAD: 55.3 - 10 = 45.3 PSF

STRAP UPLIFTING: $45.3 \times \frac{10.5}{2} \times 1'\text{-}0" = 238$# each strap

238# < 1365# DTE Hughes — OK.

Analysis & design rafter calculations.

M. Valdes-Linares P.E
2-28-98 -

Rafter 2"x6" @ 12" c/co.
Southern Pine #2 Dense.
(Grading SPIB) (NDS-1997 Edit) SPAN = 10'-6"

$F_B = 1150$ $F_V = 90$ $E = 1.700{,}000$ $F_c = 1600$

2"x6" { S = 7.563, I = 20.797, A = 8.250

DL: 10#/SF
LL = 30#/SF
(30 + 10) 1.2 = 48#/SF

$M = \frac{wL^2}{8} = \frac{48 \times (10.5)^2}{8} = 662$ FT-LB = $\underline{7{,}944\ \text{in-lb}}$

$R = \frac{wL}{2} = \frac{48 \times 10.5}{2} = 252$#

Allowable deflection: $\frac{L}{360} = \frac{10.5' \times 12}{360} = 0.35"$

$f_b = \frac{M}{S} = \frac{7944}{7.563} = 1050 < 1150$ - OK

$f_v = \frac{1.5R}{A} = \frac{1.5 \times 252}{8.250} = 46 < 90$ - OK

Actual deflection load:

$A_{TL} = \frac{5 \times 48 \times (10.5)^4 \times 1728}{384 \times 1.700{,}000 \times 20.797} = 0.37" < 0.67$ - OK

Use: Rafter: 2"x6" Southern Pine #2 Dense - OK @ 12" c/co

Footing design M. Valdes-Linares, P.E. 2-29-98

Dead loads:

$\frac{10.5}{2} \times 9.56' \times 10\#/SF = 500\#$

Wood Girder weight = 9.56' x 9.6#/LF = 92#

12" Column weight = 0.7854 x 8 x 150#/CF = 942#

= 1534#

LIVE loads: $30\#/SF \times \frac{10.5}{2} \times 9.56' = 1505\#$

= 3040#

Footing design load: 3040#
Column round 12" diam.
Soil pressure strength: 2,000#/'²
Concrete strength: 2,500#/"²
Allowable tension steel: 20,000#/"²

Try: h = 18" footing weight: 1.5' x 150#/CF = 225#/SF

Usable soil pressure: 2000 - 225 = 1775#/SF

Require width:

$\frac{3040}{1775} = 1.71' = 1'\text{-}9"$ say: 2'-0" = 24"

Design soil pressure: $\frac{3040 \times 12}{24} = 1520$#

$d = h - 3 - 0.75\,(\#6) = 18 - 3 - 0.75 = 14.25"$

$F = 1520\left(\frac{18}{12}\right) = 2280$#
$M = 1520\left(\frac{18}{2}\right) = 13680$ lb-Foot
$S = \frac{12 \times (18)^2}{6} = 648\ in^3$

v_c (allowable) $= 1.1\sqrt{f'_c} = 55$ PSI
f_c = (allowable) $= 1.6\sqrt{f'_c} = 80$ PSI

$V_c = \frac{1520}{(12)(18)} = 7.03$ PSI < 55 PSI OK

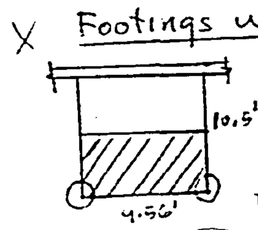
$f_c = \frac{M}{S} = \frac{13680}{648} = 21$ PSI < 80 PSI OK

$A_s = \frac{M}{f_s\,j\,d} = \frac{13680 \times 12}{20{,}000 \times 0.9 \times 14.25} = 0.64 \times 2 = 1.28\ in^2$

$4\#6 = 1.76\ in^2 > 1.28\ in^2$ — OK

Use 4 #6 both ways in conc. footing.
3'-0" x 3'-0" x 2'-0" - TYPICAL.

Footings uplifting calculations:



Area considered x wind net uplifting =

$\frac{10.5}{2} \times 9.56 \times 45.3$ PSF = 2274# x 1.5 3411

Dead load + Live load + footing weigth =

1534 + 1505 + (2x2x1.5x150) = 3,940# 2435#

$\frac{3940\#}{2274\#} = 1.73$ (Factor safety require: 1.5)

Therefore Footings uplifting - OK

Analysis & design girder calculations.

Span: 9.56' Area roof: $9.56 \times \frac{10.5}{2} = 50$#

Gravity load + wind load x 1.2 = 48#/SF

TOTAL Load: 48# x 50 = 2,400#

overhang load: 1.00 x 9.56 x 48# = 458

2,858#

$\frac{2858}{9.56} = 299\#/LF$

$M = \frac{wL^2}{8} = \frac{299 \times (9.56)^2}{8} = 341$ FT-LB = $\underline{40{,}990\ \text{in-lb}}$

$R = \frac{wL}{2} = \frac{299 \times 9.56}{2} = 1429$#

Deflection criteria: $\frac{L}{360} = \frac{9.56 \times 12}{360} = 0.31"$

Use Southern Pine #2 (dense):
FB = 1150 Fv = 90 E = 1.700,000 Fc = 1600

Try. 3"x12" girder.

3"x12" { A = 28.125, I = 296.631, S = 52.734

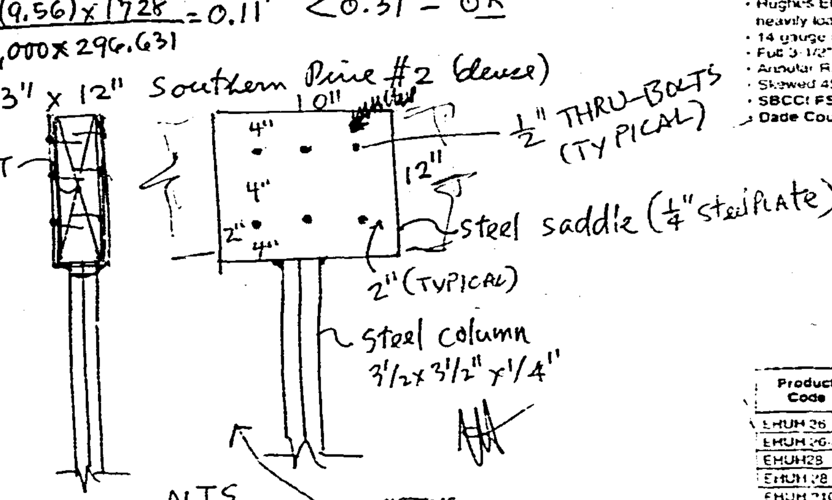
$f_b = \frac{M}{S} = \frac{40{,}990}{52.734} = 777 < 1150$ OK

$f_v = \frac{1.5R}{A} = \frac{1.5 \times 1429}{28.125} = 76 < 90$ OK

Actual deflection load:

$A_{TL} = \frac{5 \times 299 \times (9.56)^4 \times 1728}{384 \times 1.700{,}000 \times 296.631} = 0.11" < 0.31"$ - OK

Girder: Use. 3" x 12" Southern Pine #2 (dense)



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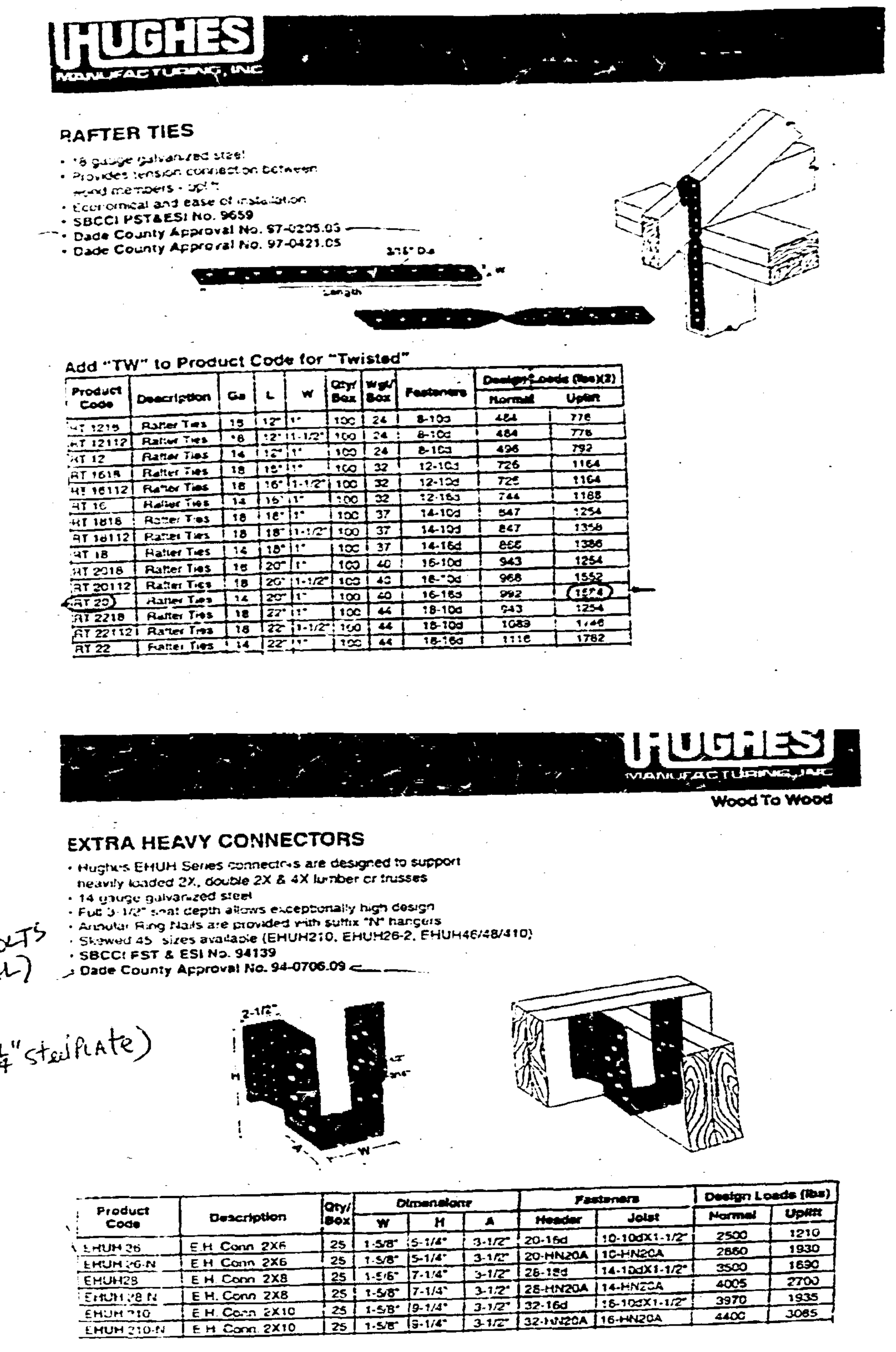
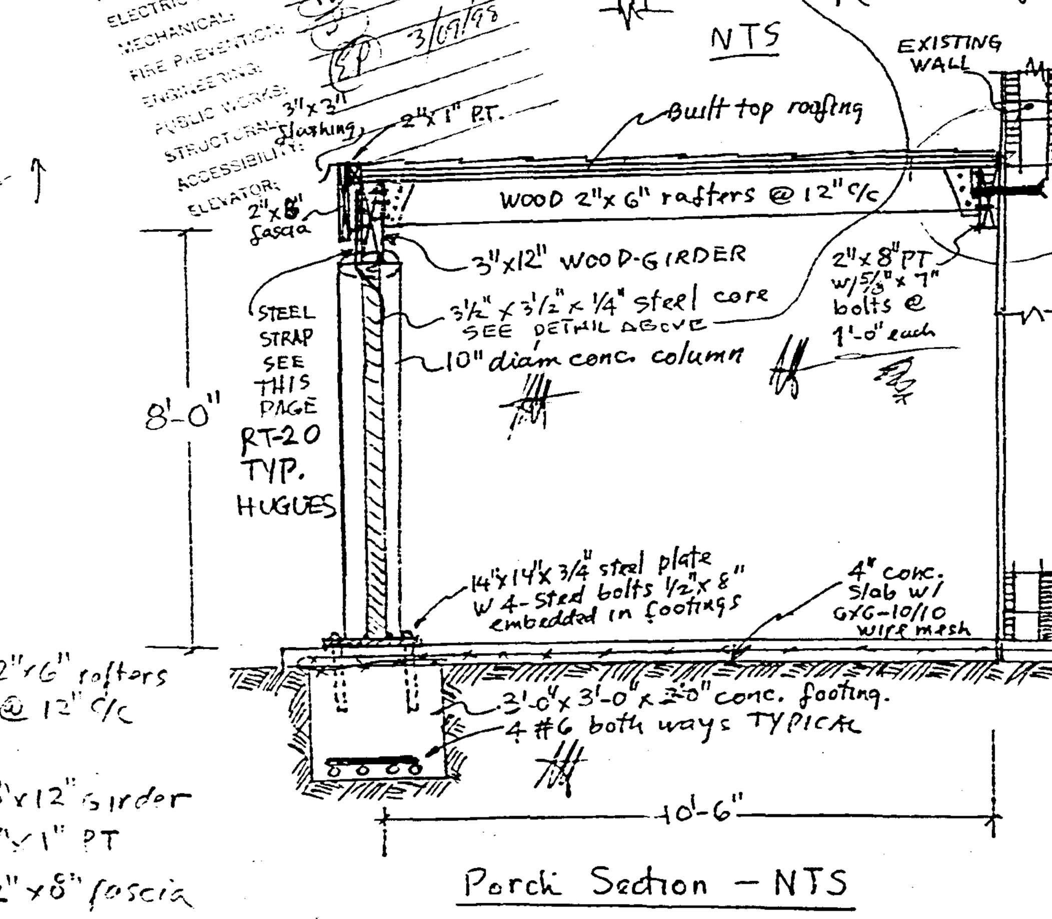
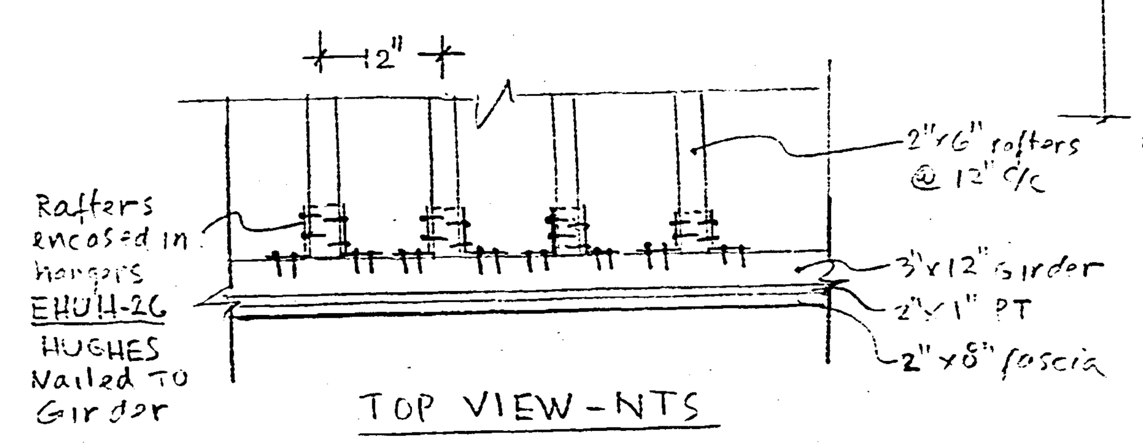
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:
BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:
ELEVATOR:

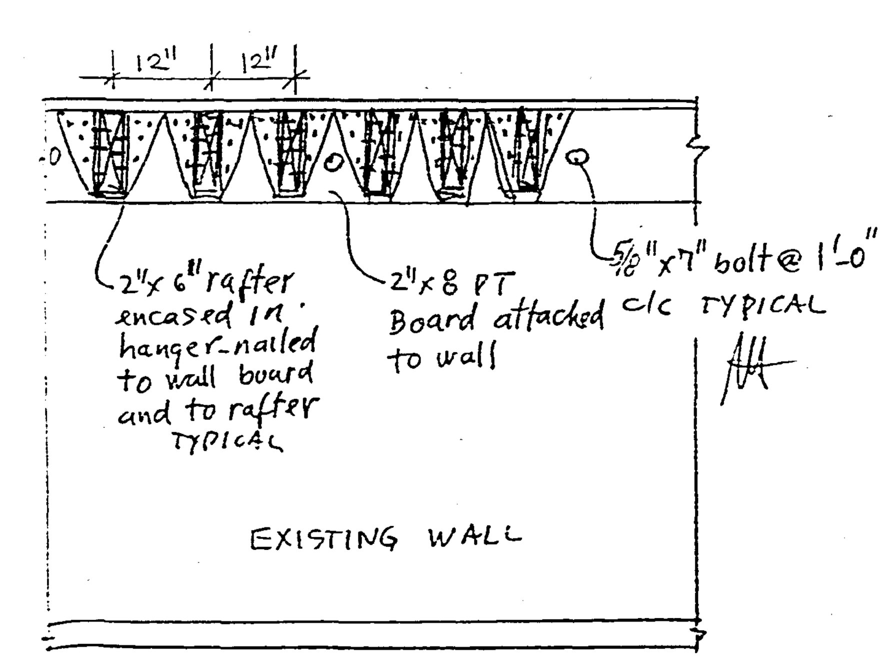




Porch Section — NTS



TOP VIEW - NTS



Rafters connection to wall - NTS -

REVISION: 3-1-98.

3-1-98

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Lic 14393
11980 S W 43 St
Miami, FL 33175
Phone (305) 553-0022

Porch Rafter repair & NEW Conc. fence		
SCALE: AS SHOWN / DATE: 3-2-98	APPROVED BY: M. VALDES-LINARES, P.E	DRAWN BY / REVISED MVL
1211 Stillwater Drive M.B.		
owner: Arturo Diaz Artiles		DRAWING NUMBER 3

21

B9801480

21

PERMIT NUMBER

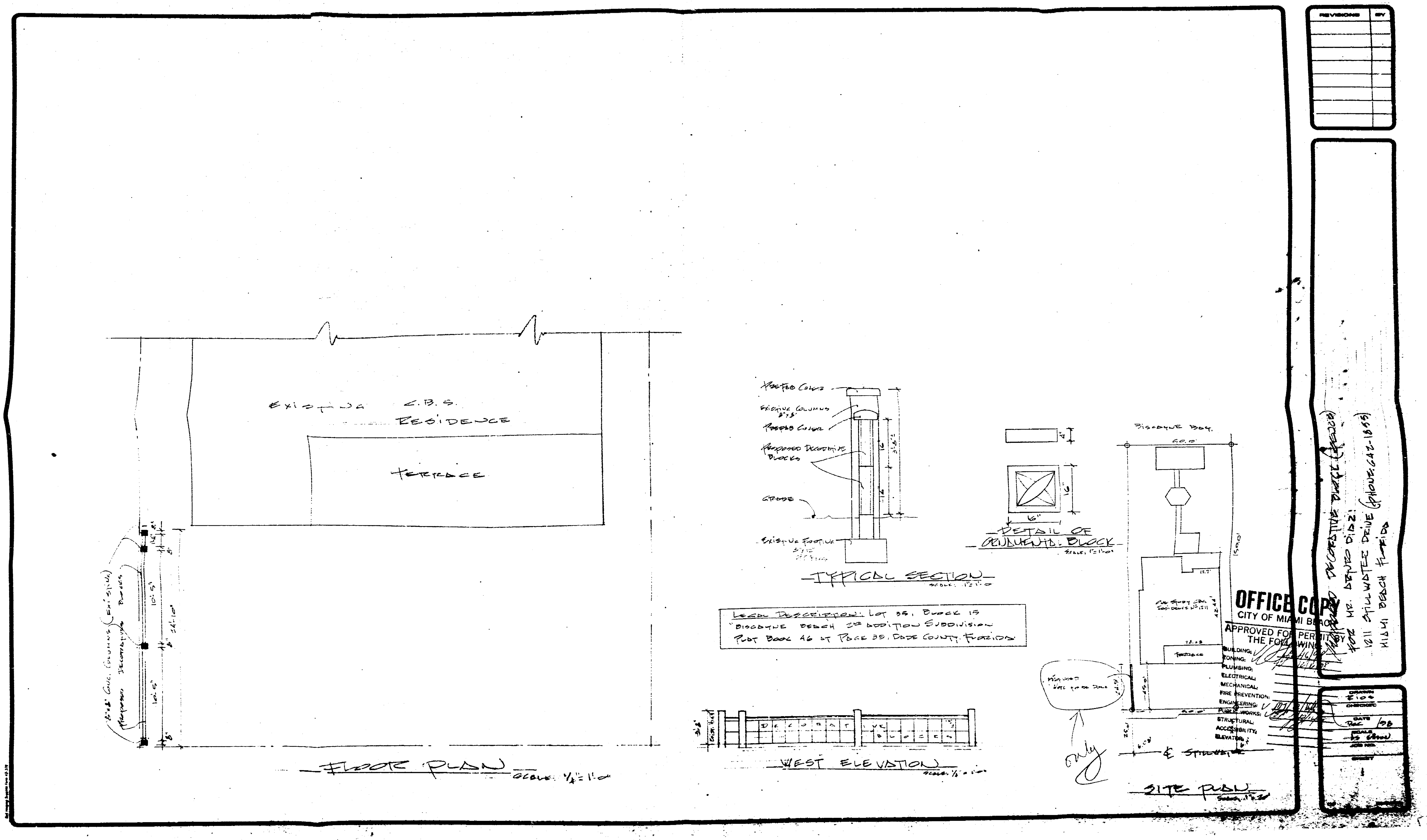
B98-01480

BMS-99069

ADDRESS

25

B98 01480
Bms 99069


EXISTING C.B.S. RESIDENCE
TERRACE
FLOOR PLAN
SCALE: 1/4" = 1'-0"
TYPICAL SECTION
SCALE: 1" = 1'-0"
DETAIL OF ORNAMENTAL BLOCK
LEGAL DESCRIPTION: LOT 35, BLOCK 15 "BISCAYNE BEACH 2ND ADDITION SUBDIVISION PLAT BOOK 46 AT PAGE 39, DADE COUNTY, FLORIDA
WEST ELEVATION
BISCAYNE BAY
50.0'
150.0'
SITE PLAN
ONLY
OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING
BUILDING:
ZONING:
PLUMBING:
ELECTRICAL:
MECHANICAL:
FIRE PREVENTION:
ENGINEERING:
PUBLIC WORKS:
STRUCTURAL:
ACCESSIBILITY:
ELEVATOR:
PROPOSED DECORATIVE BLOCK (FENCE)
FOR MR. ARTURO DIAZ
1211 STILLWATER DRIVE (PHONE: 642-1855)
MIAMI BEACH FLORIDA
REVISIONS
BY
DRAWN
CHECKED
DATE
DEC /98
SCALE
AS SHOWN
JOB NO.
SHEET

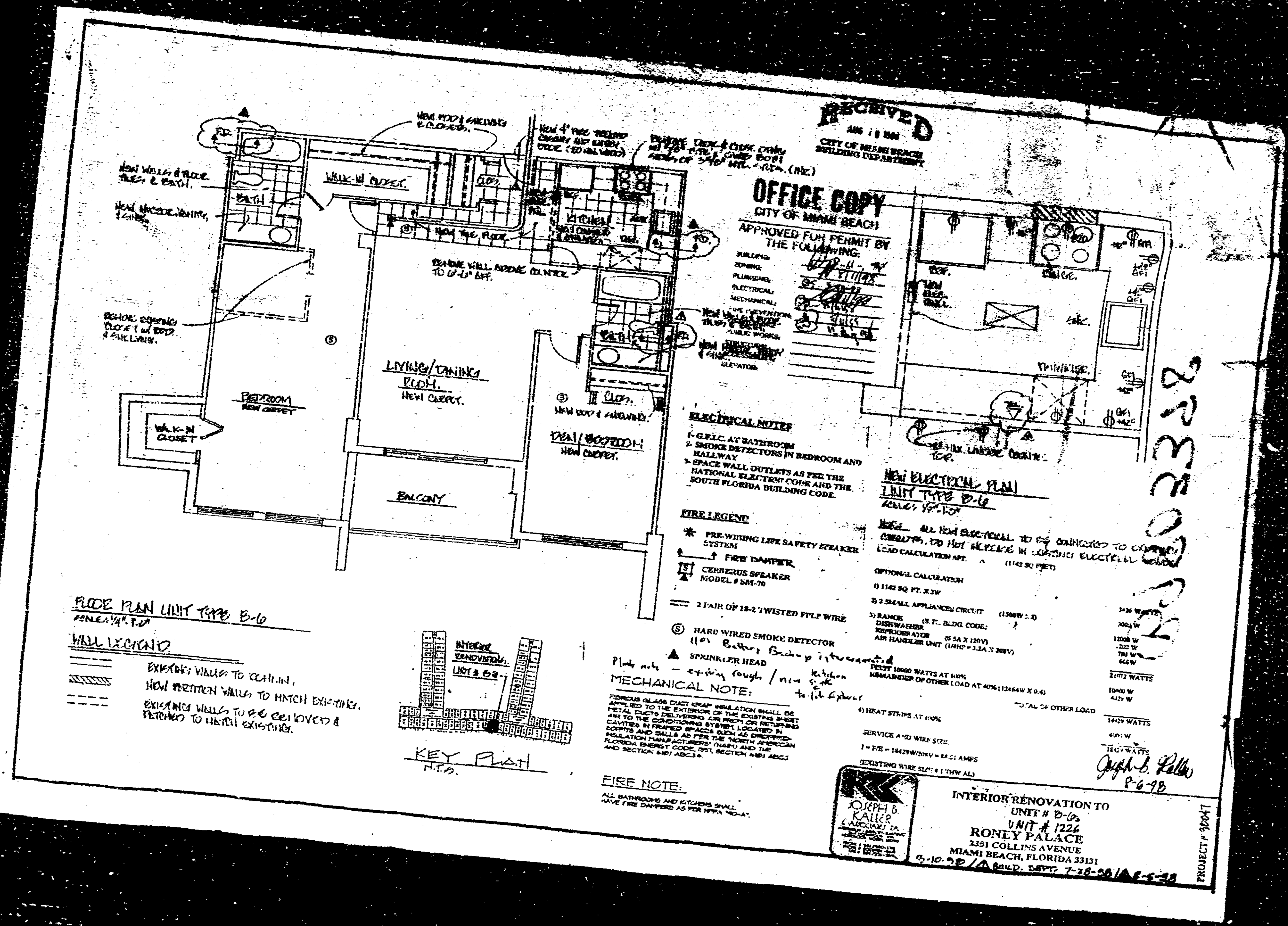
PERMIT #

B9?0 3398

ADDRESS

2301 Collins Ave

000019



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CITY OF MIAMI BEACH BUILDING DEPARTMENT
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CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:
BATH
WALK-IN CLOSET
KITCHEN
LIVING/DINING ROOM
NEW CARPET
BEDROOM
WALK-IN CLOSET
DEN/BEDROOM
BALCONY
FLOOR PLAN UNIT TYPE B-6
WALL LEGEND
KEY PLAN
ELECTRICAL NOTES
1- G.F.I.C. AT BATHROOM
2- SMOKE DETECTORS IN BEDROOM AND HALLWAY
3- SPACE WALL OUTLETS AS PER THE NATIONAL ELECTRIC CODE AND THE SOUTH FLORIDA BUILDING CODE.
FIRE LEGEND
PRE-WIRING LIFE SAFETY SPEAKER SYSTEM
FIRE DAMPER
CERBERUS SPEAKER MODEL # SM-70
2 PAIR OF 18-2 TWISTED FPLP WIRE
HARD WIRED SMOKE DETECTOR
SPRINKLER HEAD
MECHANICAL NOTE:
FIRE NOTE:
ALL BATHROOMS AND KITCHENS SHALL HAVE FIRE DAMPERS AS PER NFPA 90-A.
NEW ELECTRICAL PLAN
UNIT TYPE B-6
LOAD CALCULATION APT. (1142 SQ FEET)
OPTIONAL CALCULATION
1) 1142 SQ. FT. X 3W
2) 2 SMALL APPLIANCES CIRCUIT (1500W X 2)
3) RANGE
DISHWASHER
REFRIGERATOR
AIR HANDLER UNIT
FIRST 10000 WATTS AT 100%
REMAINDER OF OTHER LOAD AT 40% (12464W X 0.4)
TOTAL OF OTHER LOAD
4) HEAT STRIPS AT 100%
SERVICE AND WIRE SIZE
21072 WATTS
10000 W
14429 WATTS
Joseph B. Kaller
8-6-98
JOSEPH B. KALLER & ASSOCIATES P.A.
INTERIOR RENOVATION TO
UNIT # B-6
UNIT # 1226
RONEY PALACE
2351 COLLINS AVENUE
MIAMI BEACH, FLORIDA 33131
PROJECT # 98047
B98 3328