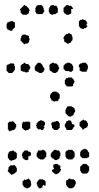
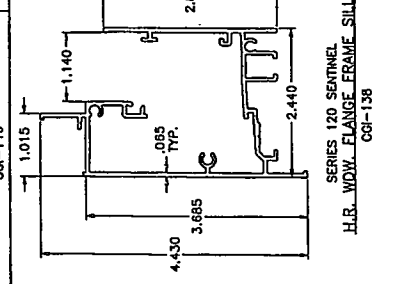
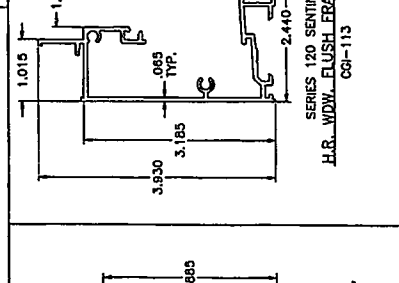
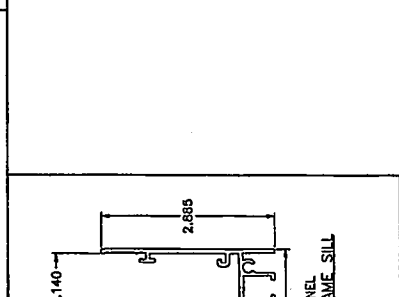
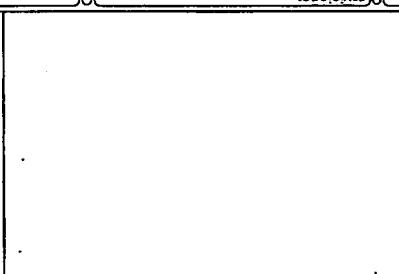


FIXED WINDOW ANCHOR INSTALLATION DETAILS SHOWN.
ALSO APPLICABLE TO SINGLE HUNG AND
HORIZONTAL SLIDING WINDOW FRAMES IN FLANGE,
EQUAL LEG OR FIN FRAME CONFIGURATIONS.

PRODUCT: VISED
complying with the Florida
Building Code
License No. 11-124107
Expiration Date 11-12-2015
Signature: [Signature]
Title: Project Engineer
Date: 11-18-2012



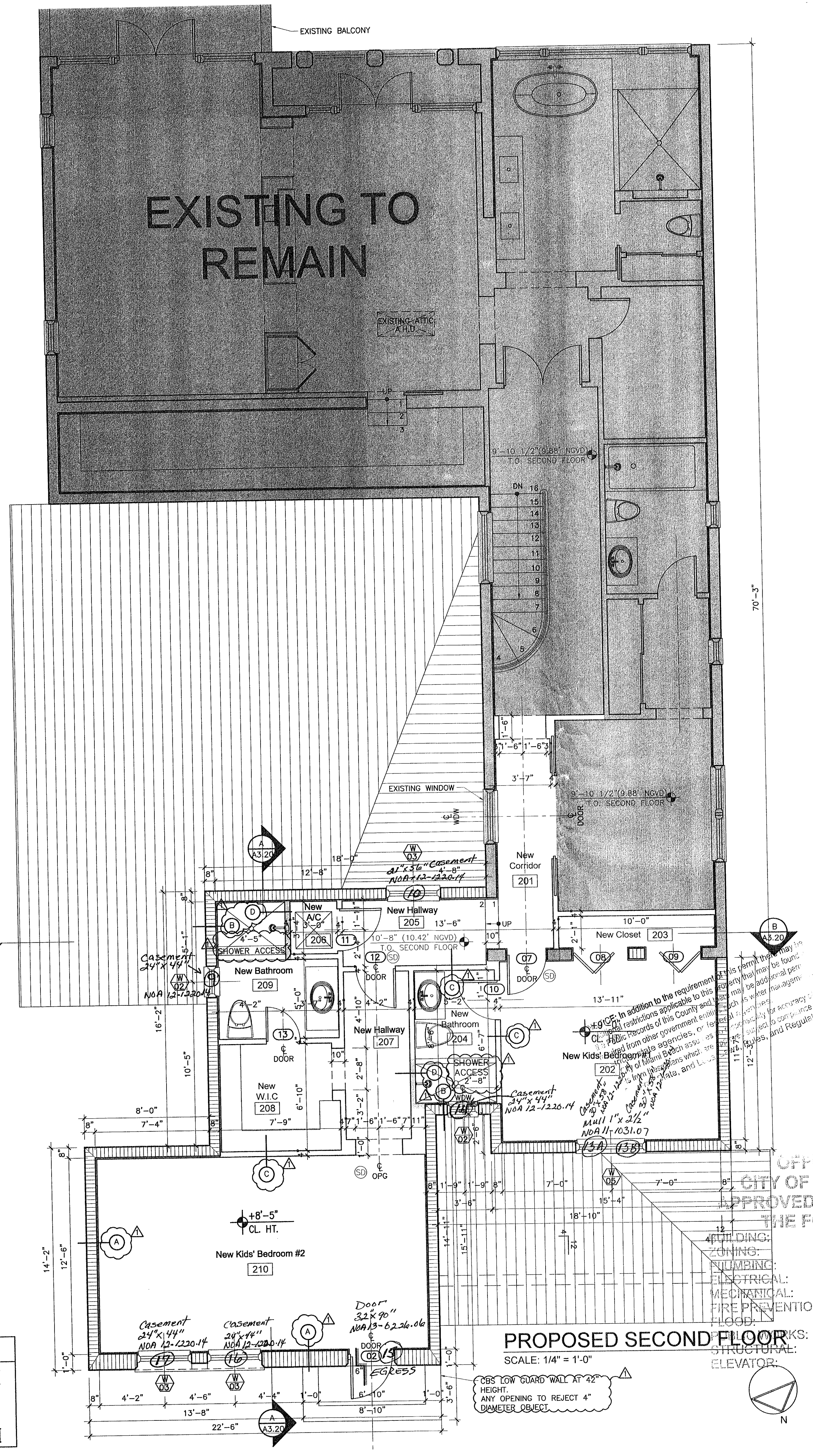
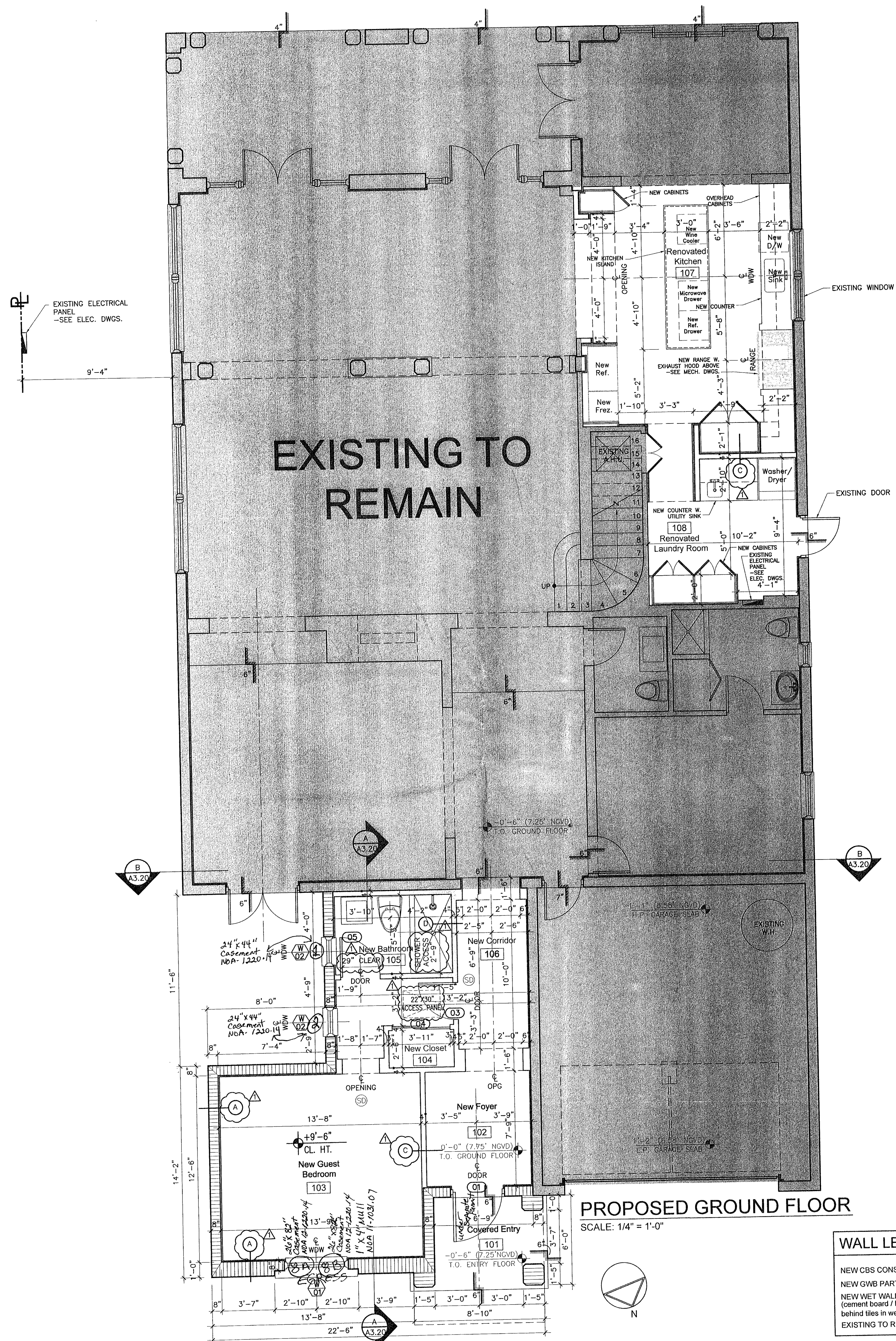


EXPERT JAWAD AHMAD
 CIVIL
 FUA RE # 78442
 DAIN # 3338
 JAN 13 2012
 PRODUCT REVISED
 2: complying with the Florida
 Building Code
 Acceptance No. 11-103607
 Registration Date 2-27-2015
 By Manuel Ruiz
 Millen Date Product Condon

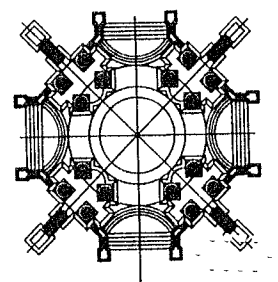


NEWDOC

207 8L



Blue 2636



Z.W. JAROSZ ARCHITECT, P.A.
3326 MARY STREET SUITE 500
COCONUT GROVE, FLORIDA 33133
305.446.0888 WWW.JAROSZARCH.COM

PROJECT / SHEET TITLE

SNYDER RESIDENCE
190 S Hibiscus Drive, Miami Beach, FL 33139

PROPOSED FLOORPLANS

LIC. AR8223

REVISIONS		
NO.	DATE	DESCRIPTION
1	11.20.13	Building Comments

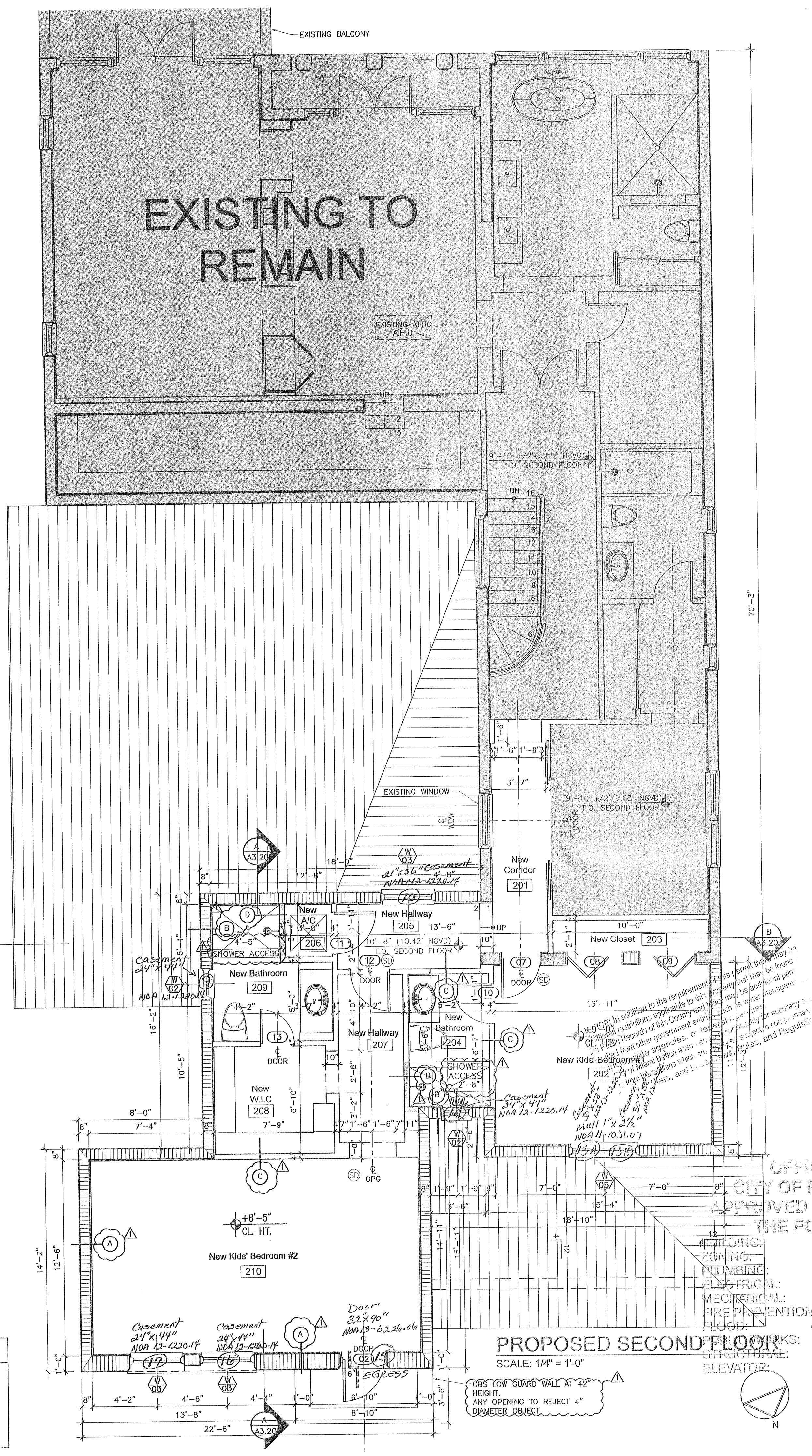
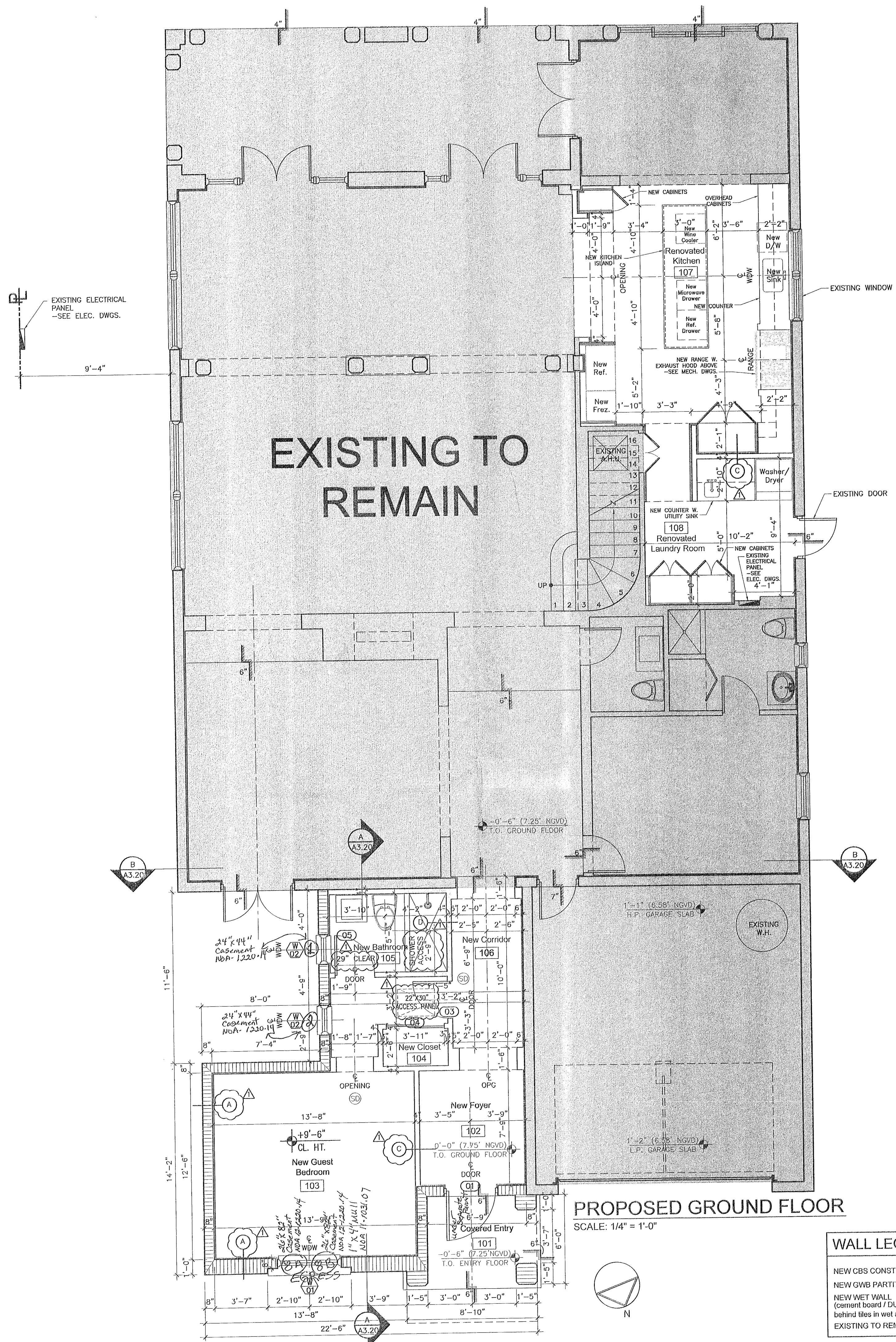
OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:

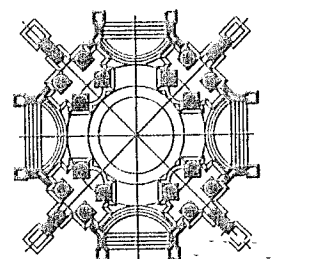
Mr. 4/3/14
Mr. 4/3/14

1301-SNYD

DATE	NOV 20, 2013
DRAWN BY	AB
CHECKED BY	CHECKED BY: ZI
SCALE	AS SHOWN
SHEET	

A2.01





Z.W. JAROSZ ARCHITECT, P.A.
3326 MARY STREET SUITE 500
COCONUT GROVE, FLORIDA 33133
305.446.0888 WWW.JAROSZARCH.COM

PROJECT / SHEET TITLE

SNYDER RESIDENCE
190 S Hibiscus Drive, Miami Beach, FL 33139

PROPOSED FLOORPLANS

LIC. AR8223

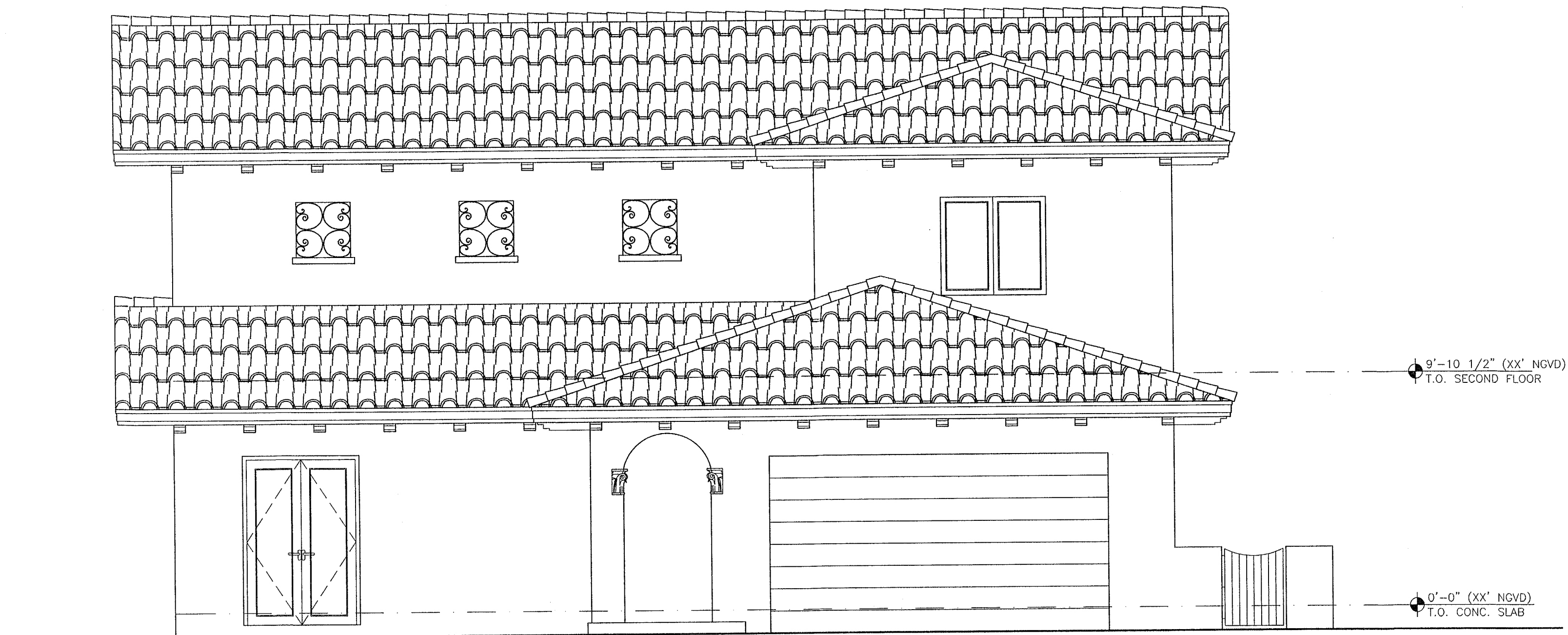
REVISIONS		
NO.	DATE	DESCRIPTION
1	11.20.13	Building Comments

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY THE FOLLOWING:

1301-SNYD

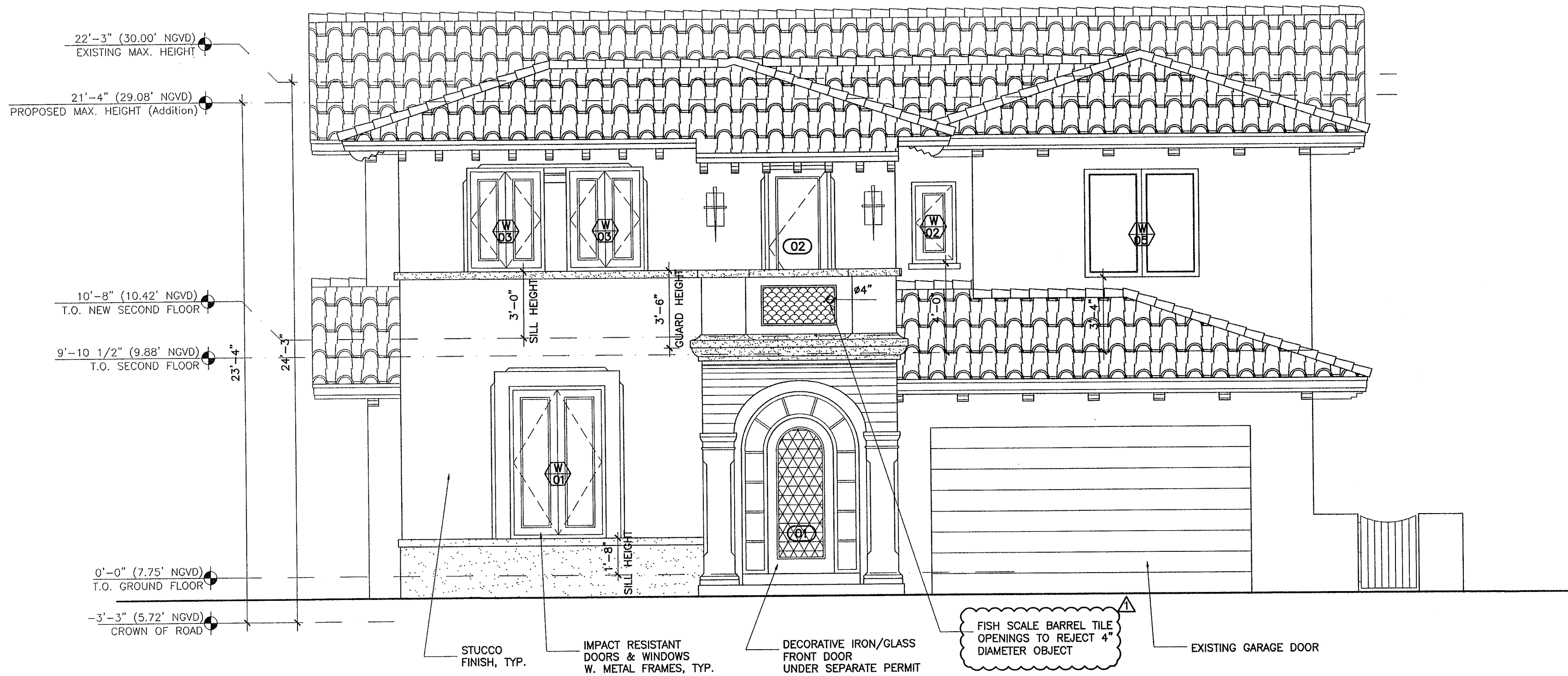
DATE: Nov 20, 2013
DRAWN BY: AB
CHECKED BY: ZJ
SCALE: AS SHOWN
SHEET

A2.01



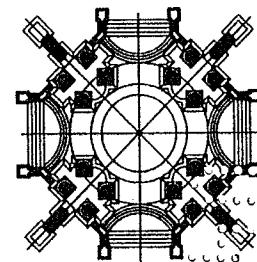
EXISTING FRONT ELEVATION

SCALE: 1/4" = 1'-0"



PROPOSED FRONT ELEVATION

SCALE: 1/4" = 1'-0"



Z.W. JAROSZ ARCHITECT, P.A.

3326 MARY STREET, SUITE 500
COCONUT GROVE, FLORIDA 33133
305.446.0888 WWW.JAROSZARCH.COM

PROJECT /
SHEET TITLE

SNYDER RESIDENCE
190 S Hibiscus Drive, Miami Beach, FL 33139

FRONT ELEVATION
EXISTING VS. PROPOSED

LIC. AR8223

REVISIONS

NO.	DATE	DESCRIPTION
1	11.20.13	Building Comments

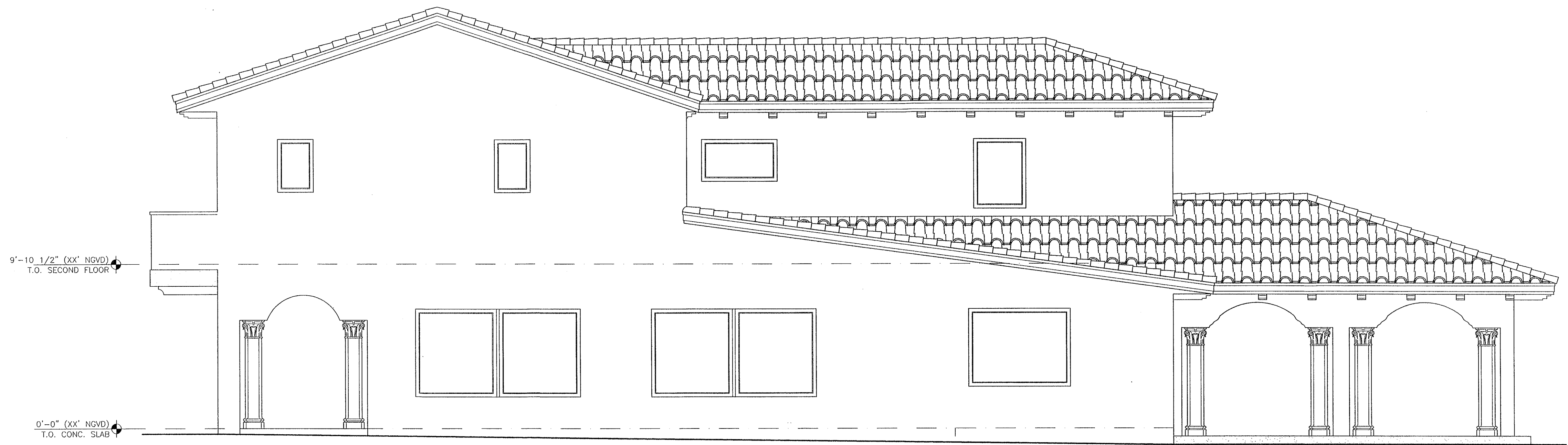
COPYRIGHT 2013 ALL RIGHTS RESERVED. THESE DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY OF Z.W. JAROSZ ARCHITECT, P.A. AND SHALL REMAIN THE PROPERTY OF Z.W. JAROSZ ARCHITECT, P.A. UNLESS THE PROJECT FOR WHICH THEY WERE PREPARED IS LOCATED ON A LOT THAT IS NOT TO BE USED IN ANY MANNER FOR OTHER PROJECTS OR OTHERWISE TO THE PROJECT EXCEPT BY AGREEMENT IN WRITING AND WITH THE WRITTEN CONSENT OF Z.W. JAROSZ ARCHITECT, P.A. REPRODUCTIONS OF DRAWINGS AND SPECIFICATIONS WITHOUT THE WRITTEN CONSENT OF Z.W. JAROSZ ARCHITECT, P.A. ARE PROHIBITED. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SITE CONDITIONS PRIOR TO PROCEEDING WITH WORK.

1301-SNYD

DATE:	Nov 20, 2013
DRAWN BY:	AS
CHECKED BY:	CHECKED BY: ZJ
SCALE:	AS SHOWN

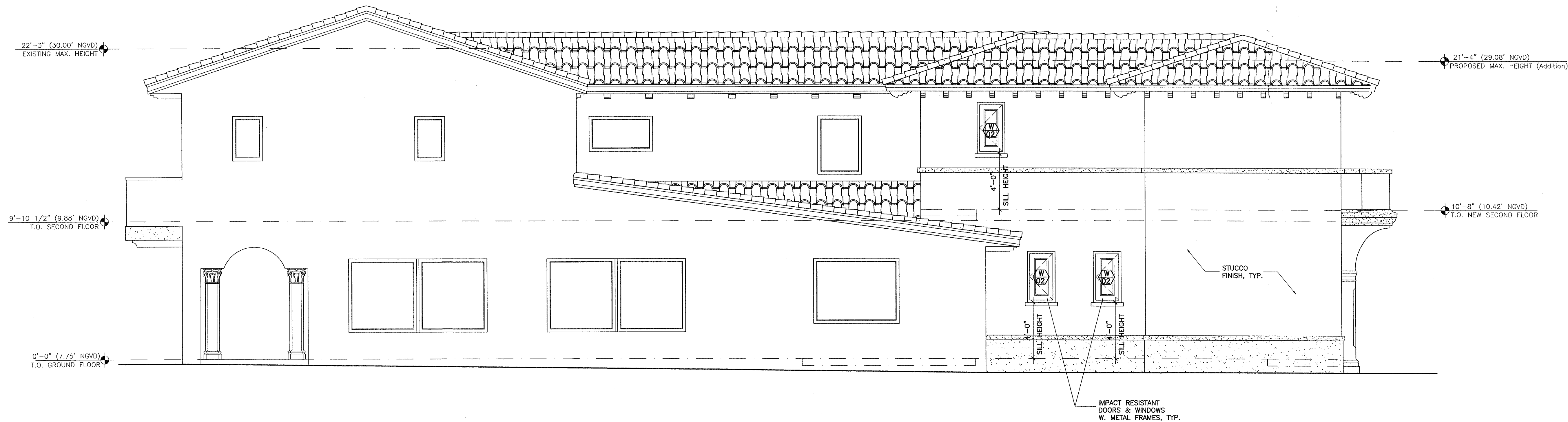
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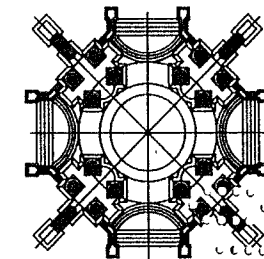
EXISTING EAST ELEVATION

SCALE: 1/4" = 1'-0"



PROPOSED EAST ELEVATION

SCALE: 1/4" = 1'-0"



Z.W. JAROSZ ARCHITECT, P.A.

3326 MARY STREET SUITE 500
COCONUT GROVE, FLORIDA 33133
305.446.0888 WWW.JAROSZARCH.COM

PROJECT /
SHEET TITLE

SNYDER RESIDENCE
190 S Hibiscus Drive, Miami Beach, FL 33139

WEST ELEVATION
EXISTING VS. PROPOSED

LIC. AR8223

REVISIONS

NO.	DATE	DESCRIPTION
1	11.20.13	Building Comments

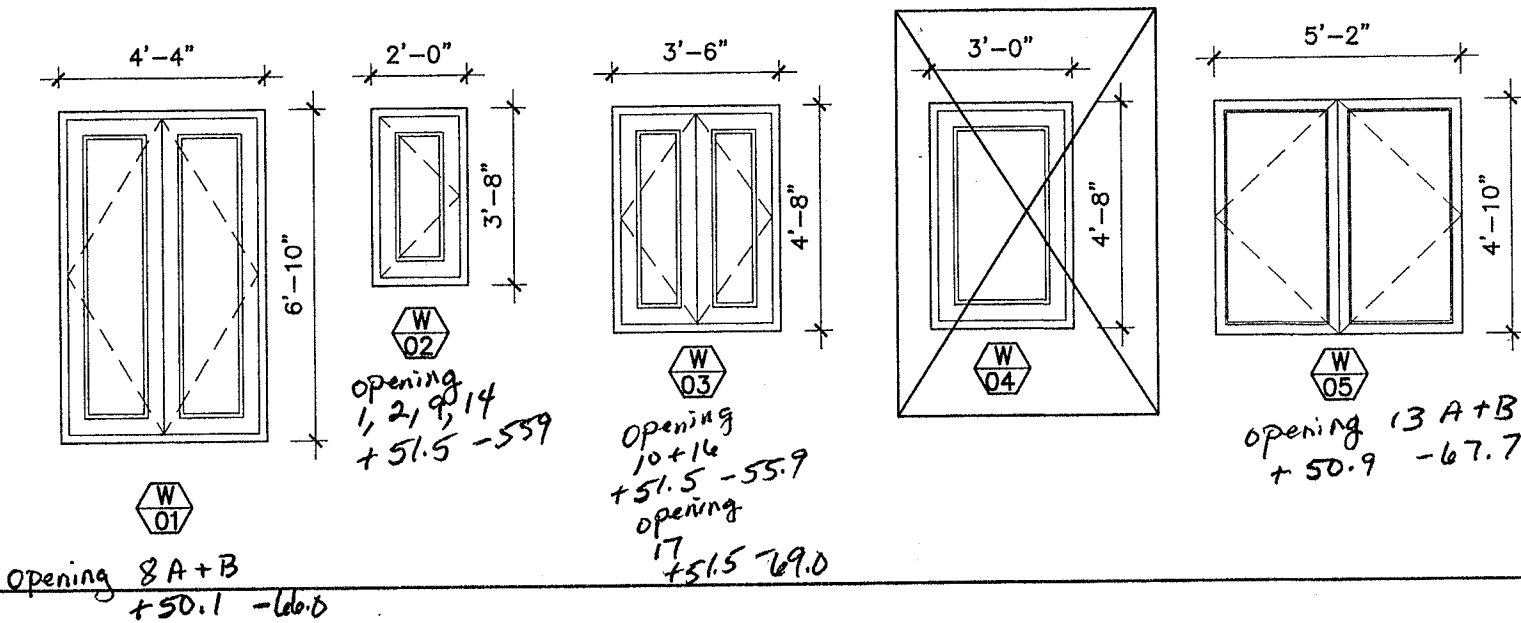
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1301-SNYD

DATE:	Nov 20, 2013
DRAWN BY:	AB
CHECKED BY:	CHECKED BY: ZJ
SCALE:	AS SHOWN

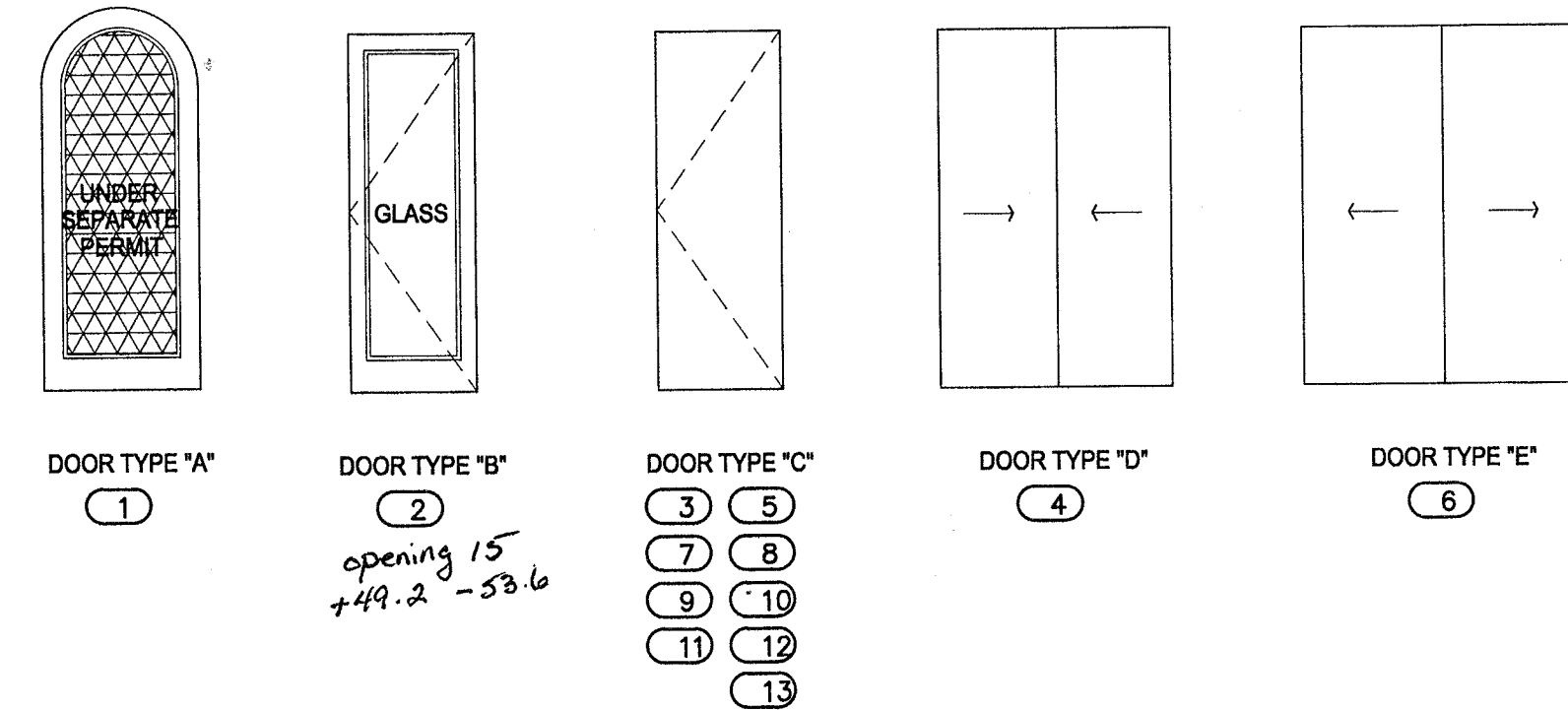
SHEET

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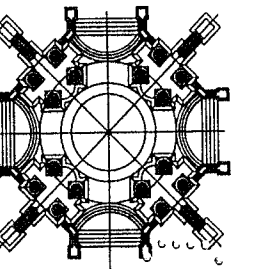
SCALE: 1/4" = 1'-0"

DOOR #	DR TYPE	N.O.A.#	DOOR SIZE	REMARKS
1	A		3'-4" X 8'-0" X 1 3/4"	WROUGHT IRON
2	B		2'-8" X 7'-6" X 1 3/4"	METAL FRAME W. GLASS
3	C		2'-6" X 8'-0" X 1 3/4"	
4	D		(2)2'-0" X 8'-0" X 1 3/4"	DOUBLE SLIDING WOOD DOORS
5	C		2'-8" X 8'-0" X 1 3/4"	WOOD
6	E		(2)2'-6" X 7'-0" X 1 3/4"	DOUBLE POCKET WOOD DOORS
7,10,12	C		2'-6" X 7'-0" X 1 3/4"	WOOD
8,9	C		3'-6" X 7'-0" X 1 3/4"	WOOD
11	C		2'-8" X 7'-0" X 1 3/4"	WOOD
13	C		2'-2" X 7'-0" X 1 3/4"	WOOD



SCALE: 1/4" = 1'-0"

NOTE:
Windows and doors will be under separate permit.



Z.W. JAROSZ ARCHITECT, P.A.

3326 MARY STREET, SUITE 300
COCONUT GROVE, FLORIDA 33133
305.446.0888 WWW.JAROSZARCH.COM

PROJECT /
SHEET TITLE

SNYDER RESIDENCE
190 S Hibiscus Drive, Miami Beach, FL 33139

DOOR & WINDOW SCHEDULE

IC. AR8223

REVISIONS

[illegible]

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1301-SNYD

DATE:	Nov 20, 2013
DRAWN BY:	AB
CHECKED BY:	CHECKED BY: ZJ
SCALE:	AS SHOWN
QUEST	

A5.01

B402636

B400193

190 S Hibiscus Dr.

OFFICE COPY



MIAMI BEACH

Building Department
1700 Convention Center Drive, 2nd Flr
Miami Beach, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING
DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR
UNDER THE FLORIDA BUILDING CODE

I have been retained by: Forever Pools to perform special inspector services under the
Florida Building Code at the 190 S Hibiscus Dr. project on the below listed structures as of
6/5/14 (date). I am a professional engineer licensed in the State of Florida.

Process Number: B1404308 Master Permit (IF APPLICABLE): _____

- ☒ Special Inspector for Pilings, FBC 1822.1.20
☐ Special Inspector for Lightweight Insulating Concrete, FBC 1917.2
☐ Special Inspector for Soil Compaction, FBC 1820.3.1
☐ Special Inspector for Precast Units and Attachments, FBC 1927.12.2 (By P.E. or R.A.)
☐ Special Inspector for Reinforced Masonry, FBC 2122.4 (By P.E. or R.A.)
☐ Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A.)
☐ Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R.A.)
☐ Special Inspector for _____

NOTE: Only the marked boxes apply.

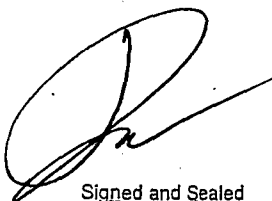
The following individual's employed by this firm or me are authorized representatives to perform inspections

- | | |
|-------------------------|-------------------------|
| 1. <u>Paul Pena, PE</u> | 2. <u>Raviery Cotto</u> |
| 3. <u>Jose Presti</u> | 4. _____ |

* Special inspectors utilizing authorized representatives shall insure the authorized representative is qualified by education or licensure to perform the duties assigned by the Special Inspector. The qualifications shall include: licensure as a professional engineer or architect; graduation from an engineering education program in civil or structural engineering; graduation from an architectural education program; successful completion of the NCEES Fundamentals Examination; or registration as a building inspector or general contractor.

I will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, understand that all mandatory inspections, as required by the Florida Building Code, shall be requested by the permit holder and approved by the Building Department Inspectors. Inspections performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. A Special Inspection Log for each building must be displayed in a convenient location on the site for inspection by the Building Department Inspectors. Further, upon completion of the work under each building permit, I will submit to the Building Department at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.


Signed and Sealed
37334

License Number

Date: 6/5/14

Architect/Engineer Signature:

Architect/Engineer

Name Printed:

Address:

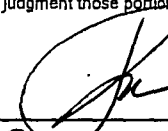
Phone Number:

Owner/Agent Signature:

Owner/Agent Name Printed:

Building Department

Accepted By:

 6/5/14
Paul Pena, PE
7450 Griffin Rd #140, Davie, FL
954-584-6115
x DAVID CANUT
x [Signature]
7/1/14



MIAMI BEACH

City of Miami Beach, 1700 Convention Center Drive, Miami Beach, Florida 33139, www.miamibeachfl.gov

Residential Swimming Pool, Spa or Hot Tub Safety Act Notice of Requirements

I (we) acknowledge that a new swimming pool, spa, or hot tub will be constructed or installed at 190 S Hibiscus DR and hereby affirm that one of the following methods will be used to meet the requirements of Florida Statute Chapter 515, and Florida Building Code Section 242.2.

Please initial the method(s) to be used for your pool or spa.

- ☒ The pool will be equipped with an approved safety pool cover that complies with ASTM F1346-91. (Submit Manufacturer specifications)
- ☐ A removable child barrier (with one end that shall be removable without the aide of tools) in compliance with FBC424.17 will protect the pool perimeter. (Submit Manufacturer Specifications).
- ☐ A combination of "non-dwelling" walls (fences, screen enclosures, etc.) will protect the perimeter. The plans must specify the type and location of all non-dwelling walls.
- ☐ A combination of protection which incorporates dwelling walls with openings into the pool perimeter and complying with FBC Section 424.2.17.1.9 (2): All doors and windows providing direct access to the pool must be equipped with self close and self latch-locking mechanical devices installed a minimum of 54" above the threshold. (Submit specifications for approval).
- ☐ A combination of protection which incorporates dwelling walls with openings into the pool perimeter and complying with FBC Section 424.2.17.1.9 (1): All doors and windows providing direct access to the pool shall be equipped with an exit alarm complying with UL 2017. (Submit Manufacturers specifications).

In accordance with the Florida Building Code, a final inspection of the pool project will not be approved without compliance with

Private Swimming Pool Safety Requirements, and upon expiration of the permit, the pool shall be presumed to be unsafe.

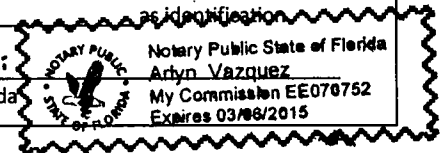
I understand that not having one of the above systems installed will constitute a violation of Chapter 515, F.S., and will be considered as committing a misdemeanor of the second degree, punishable as provided in Section 775.082 or Section 775.083 F.S. This form must be signed by the owner/agent and the prime contractor.

Owner/Agent Printed Name, Signature and Date

State of Florida
County of Miami-Dade
Sworn and Subscribed before me this 1 day of JULY, 2014
By _____ who

☒ is personally known, or
☐ produced _____ as identification

Notary Public, State of Florida

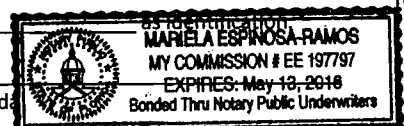


Prime Contractor Printed Name, Signature and Date

State of Florida
County of Miami-Dade
Sworn and Subscribed before me this 1 day of July, 2014
By Jose M. Arzuina who

☐ is personally known, or
☐ produced _____

Notary Public, State of Florida





MIAMI BEACH

Building Department
1700 Convention Center Drive, 2nd Flr
Miami Beach, FL 33139

NOTICE TO THE CITY OF MIAMI BEACH BUILDING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I have been retained by: Forever Pools to perform special inspector services under the Florida Building Code at the 190 S Hibiscus Dr. project on the below listed structures as of 6/5/14 (date). I am a professional engineer licensed in the State of Florida.

Process Number: _____ Master Permit (IF APPLICABLE): _____

- ☒ Special Inspector for Pilings, FBC 1822.1.20
- ☐ Special Inspector for Lightweight Insulating Concrete, FBC 1917.2
- ☐ Special Inspector for Soil Compaction, FBC 1820.3.1
- ☐ Special Inspector for Precast Units and Attachments, FBC 1927.12.2 (By P.E. or R.A..)
- ☐ Special Inspector for Reinforced Masonry, FBC 2122.4 (By P.E. or R.A.)
- ☐ Special Inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A..)
- ☐ Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R. A..)
- ☐ Special Inspector for _____

NOTE: Only the marked boxes apply.

The following individual's employed by this firm or me are authorized representatives to perform inspections

- | | |
|--------------------------|------------------------|
| 1. <u>Paul Peana, PE</u> | 2. <u>Raviery Cole</u> |
| 3. <u>Jose Presti</u> | 4. _____ |

* Special inspectors utilizing authorized representatives shall insure the authorized representative is qualified by education or licensure to perform the duties assigned by the Special Inspector. The qualifications shall include: licensure as a professional engineer or architect; graduation from an engineering education program in civil or structural engineering; graduation from an architectural education program; successful completion of the NCEES Fundamentals Examination; or registration as a building inspector or general contractor.

I will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I, understand that all mandatory inspections, as required by the Florida Building Code, shall be requested by the permit holder and approved by the Building Department inspectors. Inspections performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. A Special Inspection Log for each building must be displayed in a convenient location on the site for inspection by the Building Department inspectors. Further, upon completion of the work under each building permit, I will submit to the Building Department at the time of final inspection the completed Inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.



Architect/Engineer Signature:
Architect/Engineer
Name Printed:

Address:

Phone Number:

Owner/Agent Signature:

Owner/Agent Name Printed:

Building Department

Accepted By:

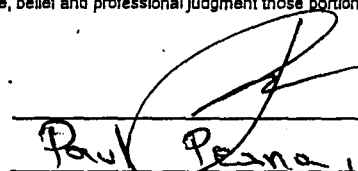
Signed and Sealed

37334

License Number

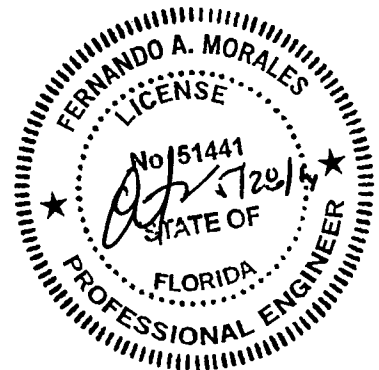
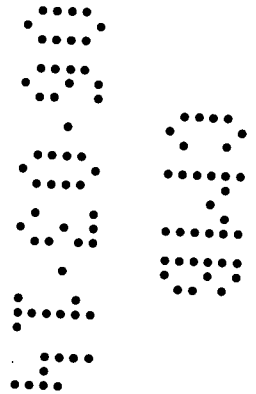
Date:

6/5/14

 6/5/14
Paul Peana, PE
7450 Griffin Rd #140, Davie, FL
954-584-6115
X DALMO CANUT
X [Signature]
Ar 7/1/14

SNYDER RESIDENCE
STRUCTURAL CALCULATIONS

190 S HIBISCUS DR,
MIAMI BEACH, FL 33140



FERNANDO MORALES, PE
5201 SW 162 PL
MIAMI, FL 33185
(786) 380- 9739
P.E. # 51441

5/28/14

Snyder Residence

190 S. Hibiscus Dr

Miami Beach, FL

SPA Size = 10.5' x 10.5'

SPA Area = 110.25 SF

SPA Perimeter = 42 LF

Foundation = 5 Ton Pin Piles

Check for Compression

Wall = depth x Perimeter x thickness x 150 psi

Wall = 4' x 42 LF x 1' x 150 psi = 25,200 lbs = 12.60 ton

Slab = Area x thickness x 150 psi

Slab = 110.25' x 0.833 x 150 psi = 13,775.74 lbs = 6.89 ton

Water = depth x 62.4 x Area

Water = 3.5' x 62.4 x 110.25 SF = 24,078.60 = 12.04 ton

Total weight = 12.60 ton + 6.89 ton + 12.04 ton = 31.53 ton

Piles Required = $\frac{TW}{EPN} = \frac{31.53 \text{ ton}}{5 \text{ ton}} = 6.31 \approx 7 \text{ piles}$

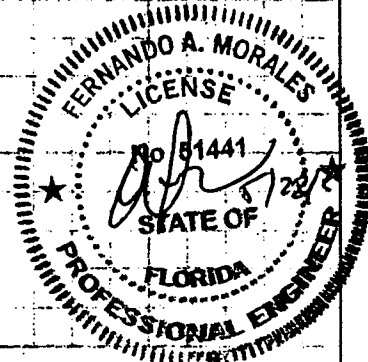
Piles Required 7, Piles Provided 10 OK

Check for tension

UP - Uplift Force = $\left(\frac{\text{depth} \times 62.4 \times \text{Area}}{2000} - \text{shell weight} \right)$

UP = $\left(\frac{2.833' \times 62.4 \times 110.25}{2000} - 9.75 \text{ ton} \right) = 9.74 \approx 10$

Piles Required 9.74, Piles Provided = 10 ✓



5/28/14

Snyder Residence
190 S. Hibiscus Dr
Miami Beach, FL

Design for Reinforcement

$$Use: M^+ = \frac{1}{8} q L^2$$

$$L_{max} = 4 \text{ ft}$$

$$\text{Column Str. } p = \left(\frac{4}{2} + 1.25 \right) = 2.25$$

$$\text{Design Str. } p = \left(\frac{4}{2} + 1.25 \right) = 3.25$$

$$\text{Slab} = 0.833 \times 150 = 124.95 \times 1.4 = 174.93 \text{ pcf}$$

$$\text{Water} = 4 \times 62.4 = 249.60 \text{ pcf} \quad \rightarrow 599.25$$

$$\text{Wall} = 4 \times \frac{8}{12} \times 150 = 400 \times 1.4 = 560 \text{ pcf}$$

$$M = \frac{1}{8} \times 599.25 \times 3.25 \times 4^2 \times \frac{0.75}{2.25} + \left(\frac{1}{8} \times \frac{560}{2.25} \times 4^2 \right)$$

$$M = 1,298.38 + 497.78 = 1,796.16 \text{ ft-kip}$$

$$d = 10'' - \frac{1}{2} \times \frac{0.5}{2} = 8.75 \quad f'_c = 5000 \quad \phi = 0.9$$

$$K = \frac{1,796.16 \times 12}{0.9 \times 12 (8.75)^2} = \frac{21.55}{826.88} = 0.0261 \quad \therefore \rho = 0.0010$$

$$A_s = 0.0010 \times 12 \times 8.75 = 0.099 < 0.16$$

Use #3 @ 8 BW TB

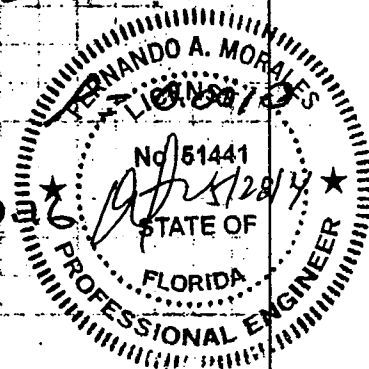
$$M^+ = 1,298.38 \times \frac{0.6}{0.75} + 497.78 = 1,536.48 \text{ ft-kip}$$

$$d = 10'' - 3'' - \frac{0.5}{2} = 6.75 \quad f'_c = 5000 \quad \phi = 0.9$$

$$K = \frac{1,536.48 \times 12}{0.9 \times 12 (6.75)^2} = \frac{18.44}{492.075} = 0.0375$$

$$A_s = 0.0010 \times 12 \times 6.75 = 0.0810 < 0.16$$

Use #3 @ 8 BW TB



RESIDENCE

EXISTING
EQUIPMENT
PAD

2-1/2" POOL OVERFLOW

3" SPA DRAINS
2-1/2" JET BLOWER
3" SPA JETS

NEW SPA ON
PIN PILES

NEW STEPS

EXISTING POOL
TO REMAIN

NEW BENCH

PUBLIC WORKS
PLAN REVIEW NOTICE
Phone 305-673-7080 Fax 305-673-7028

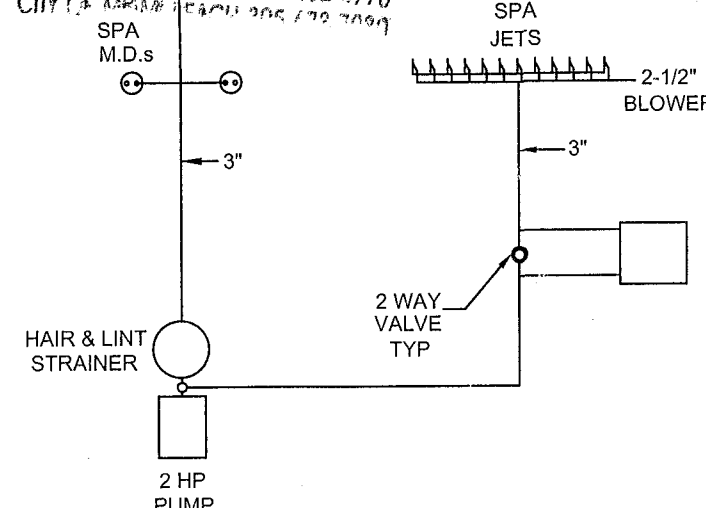
THIS PLAN REVIEW CONSTITUTES APPROVAL FOR
OBTAINING BUILDING PERMITS ONLY.

All construction and/or use of equipment in the right-of-way and/or
easements, requires a separate Public Works Department permit prior
to start of construction.

Permit Requirements: Proof of existing sidewalk/swale area conditions
(pictures) and/or posting of sidewalk/roadway bonds
(Public Works Inspection of the right-of-way will be required prior to
final sign-off on the C.C. / C.O., or the release of bonds.)

Approved/Reviewed By: AD Date: 6/4/14

48 HOURS PRIOR TO EXCAVATING
CONTRACTOR SHALL CALL FOR LOCATION
OF UNDERGROUND UTILITIES
SUNSHINE ONE CALL 1-800-432-4770
CITY OF MIAMI 305-375-7000



PLUMBING SCHEMATIC
ALL PIPING SHALL CONFORM TO
FBC-RESIDENTIAL R4101.6

SCALE:
1/8" = 1'



THE POOL IS LOCATED IN A MANOR THAT
COMPLIES WITH N.E.C. 680.8 FOR
OVERHEAD CONDUCTORS AND N.E.C.
680.10 FOR ALL UNDERGROUND
CONDUCTORS.

THE POOL EQUIPMENT PAD IS SET AT
THE SAME ELEVATION AS THE FINISHED
FLOOR OF THE HOUSE.

EXISTING POOL TO REMAIN

NEW POOL STEPS
NEW BENCH IN DEEP END

NEW SPA TO BE ADDED

SIZE: 8'6" x 8'6"
DEPTH: 36"
PERIMETER: 34'
SURFACE AREA: 72 sq.ft.
VOLUME:
TURNOVER RATE:

SPA RAISED +14" ABOVE DECK
(12) SPA JETS
(1) LED SPA LIGHT
DUAL MAIN DRAINS
4' SPILLWAY

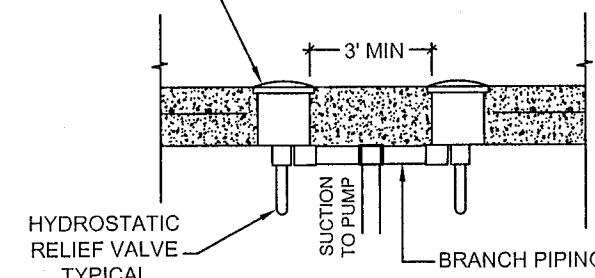
EQUIPMENT
NEW 2hp PUMP FOR SPA JETS
2hp JET BLOWER
ELECTRICAL HEATER
EXISTING POOL PUMP AND FILTER TO REMAIN

FINISHES
6" GLASS WATERLINE TILE
GEM FINISH
TRAVERTINE COPING

NOTICE: In addition to the requirement of this permit there may be
additional restrictions applicable to this property that may be found in
the Public Records of this County and there may be additional permits
required from other government entities such as water management's
districts, state agencies, or federal agencies.
The City of Miami Beach assumes no responsibility for accuracy of or
results from these plans which are approved subject to compliance with
all Federal, State, and Local Laws, Rules, and Regulations

HEATER

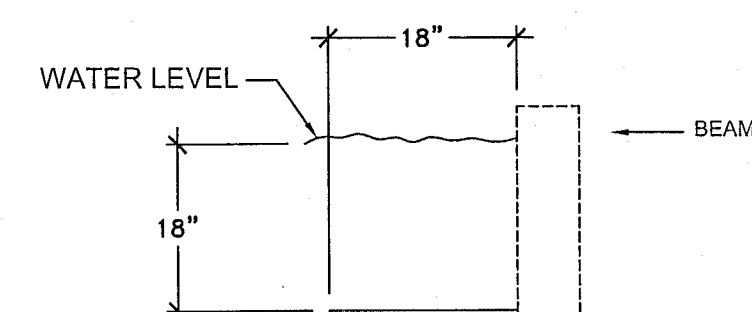
ANTI-VORTEX MAIN DRAIN COVER TESTED TO
ASME A112.19.8M 2007 BY UL 8172 LISTED WITH
ANTI-ENTRAPMENT COVER (TYPICAL OF 2)



DOUBLE MAIN DRAIN (DETAIL)
ACCORDING TO FBC-RESIDENTIAL 2010 CHAPTER 41

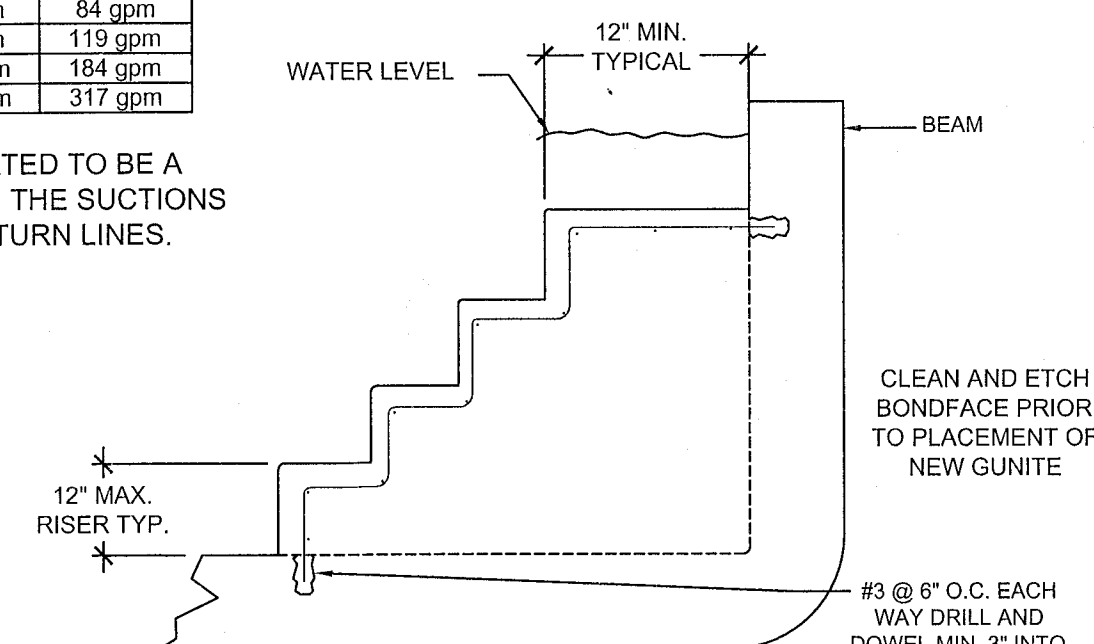
PIPE SIZE INCHES	MAX. FLOW	
	6.0 ft/sec	8.0 ft/sec
1-1/2"	38 gpm	51 gpm
2"	63 gpm	84 gpm
2-1/2"	90 gpm	119 gpm
3"	138 gpm	184 gpm
4"	238 gpm	317 gpm

ALL PIPING CALCULATED TO BE A
MAXIMUM OF 6fps ON THE SUCTIONS
AND 8fps ON THE RETURN LINES.

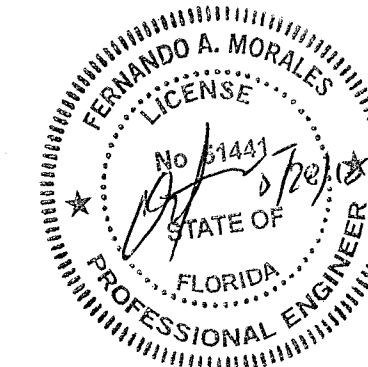


BENCH SECTION
N.T.S.

UNDERWATER BENCHES, SWIMOUTS
AND SUNSHELVES TO BE VISUALLY
SET APART FROM POOL FINISH WITH A
TILE EDGE OF A CONTRASTING COLOR.



ADDITION OF STEPS TO EXISTING POOL
N.T.S.



Fernando Morales, P.E.
5201 S.W. 162 Place
Miami, Florida 33185
786-380-9739
License #51441

SHEET 1: SPECIFICATIONS & PLUMBING

SNYDER RESIDENCE
190 S. HIBISCUS DRIVE
MIAMI BEACH

LEGAL: LOT 28, BLOCK 3
HIBISCUS ISLAND

JOB NO. 35055

8067041308

PROPERTY LINE

RESIDENCE

EXISTING
EQUIPMENT
PAD

3" SPA DRAINS
2" SPA INLETS
2-1/2" JET BLOWER
3" SPA JETS

NEW SPA ON
PIN PILES

EXISTING POOL
TO REMAIN

NEW BENCH

SCALE:
1/8" = 1'



THE POOL IS LOCATED IN A MANOR THAT
COMPLIES WITH N.E.C. 680.8 FOR
OVERHEAD CONDUCTORS AND N.E.C.
680.10 FOR ALL UNDERGROUND
CONDUCTORS.

THE POOL EQUIPMENT PAD IS SET AT
THE SAME ELEVATION AS THE FINISHED
FLOOR OF THE HOUSE.

EXISTING POOL TO REMAIN

NEW POOL STEPS
NEW BENCH IN DEEP END

NEW SPA TO BE ADDED

SIZE: 8'6" x 8'6"
DEPTH: 36"
PERIMETER: 34'
SURFACE AREA: 72 sq.ft.
VOLUME: 1,215 GALLONS

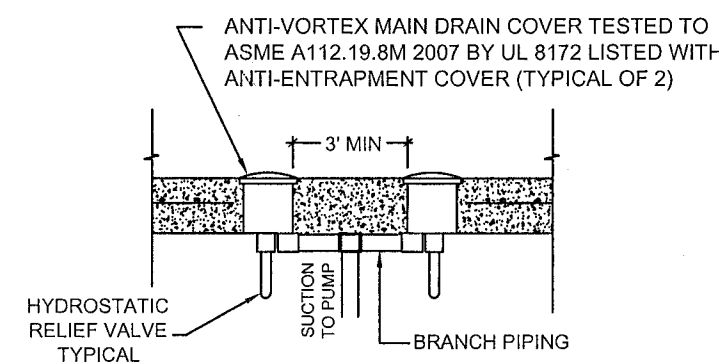
SPA RAISED +14" ABOVE DECK
(12) SPA JETS
(1) LED SPA LIGHT
DUAL MAIN DRAINS
4' SPILLWAY

EQUIPMENT

NEW 2hp PUMP FOR SPA JETS
2hp JET BLOWER
ELECTRICAL HEATER
EXISTING POOL PUMP AND FILTER TO REMAIN

FINISHES

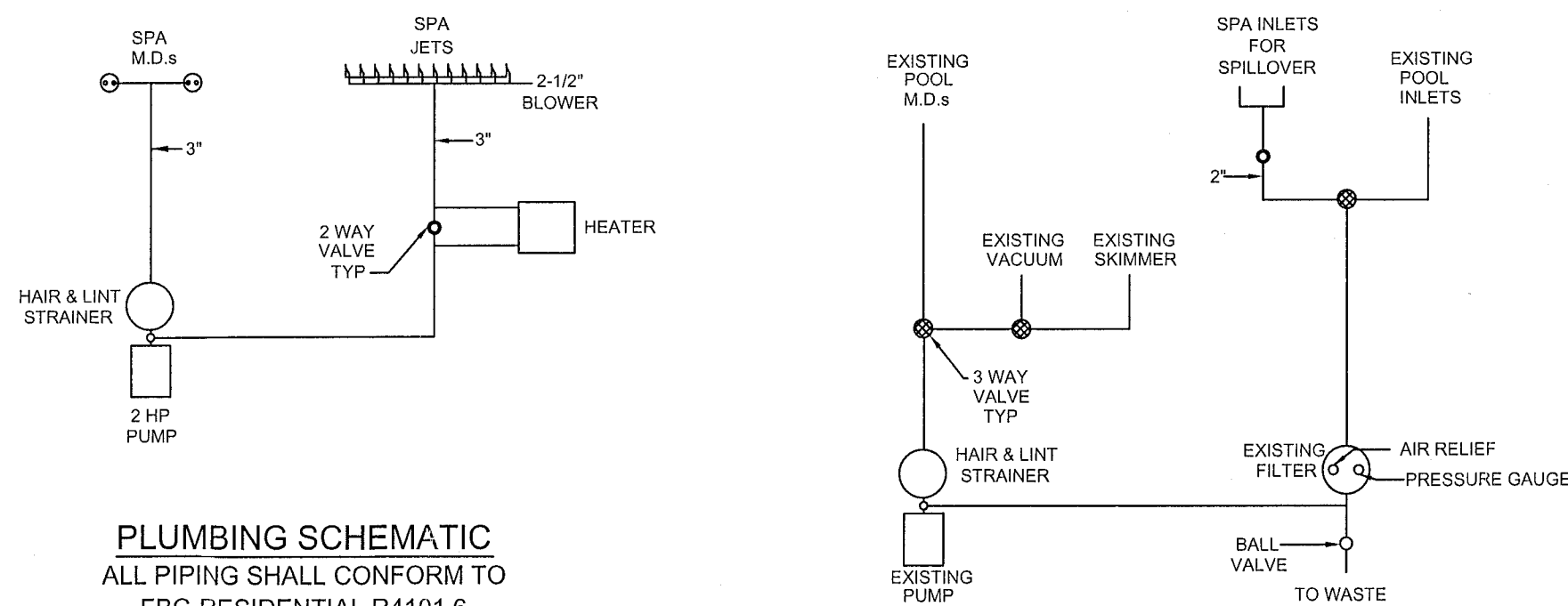
6" GLASS WATERLINE TILE
GEM FINISH
TRAVERTINE COPING



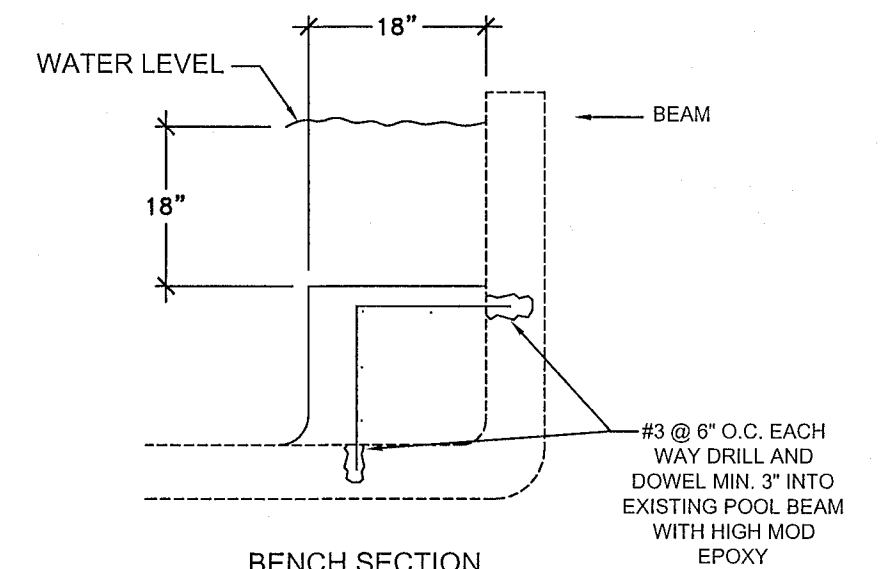
DOUBLE MAIN DRAIN (DETAIL)
ACCORDING TO FBC-RESIDENTIAL 2010 CHAPTER 41

PIPE SIZE INCHES	MAX. FLOW	
	6.0 ft/sec	8.0 ft/sec
1-1/2"	38 gpm	51 gpm
2"	63 gpm	84 gpm
2-1/2"	90 gpm	119 gpm
3"	138 gpm	184 gpm
4"	238 gpm	317 gpm

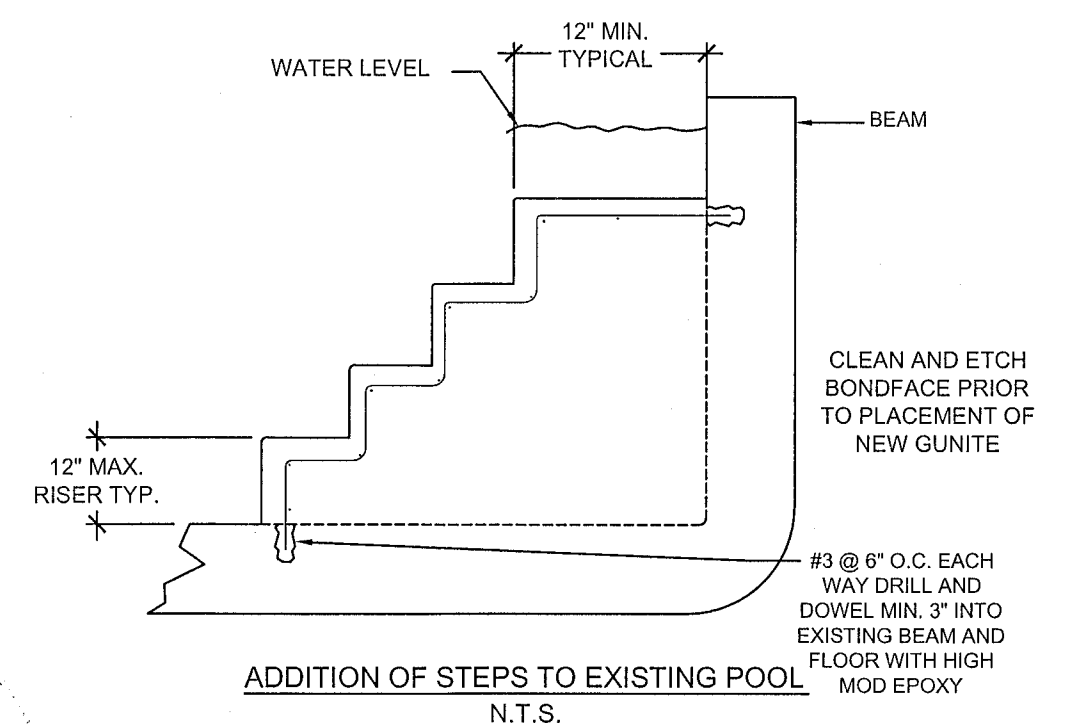
ALL PIPING CALCULATED TO BE A
MAXIMUM OF 6fps ON THE SUCTIONS
AND 8fps ON THE RETURN LINES.



PLUMBING SCHEMATIC
ALL PIPING SHALL CONFORM TO
FBC-RESIDENTIAL R4101.6



UNDERWATER BENCHES, SWIMOUTS
AND SUNSHELVES TO BE VISUALLY
SET APART FROM POOL FINISH WITH A
TILE EDGE OF A CONTRASTING COLOR.



Fernando Morales, P.E.
5201 S.W. 162 Place
Miami, Florida 33185
786-380-9739
License #51441

SHEET 1: SPECIFICATIONS & PLUMBING

SNYDER RESIDENCE
190 S. HIBISCUS DRIVE
MIAMI BEACH

LEGAL: LOT 28, BLOCK 3
HIBISCUS ISLAND

JOB NO. 35055

1'-3"

4'

4'

1'-3"

4'

4'

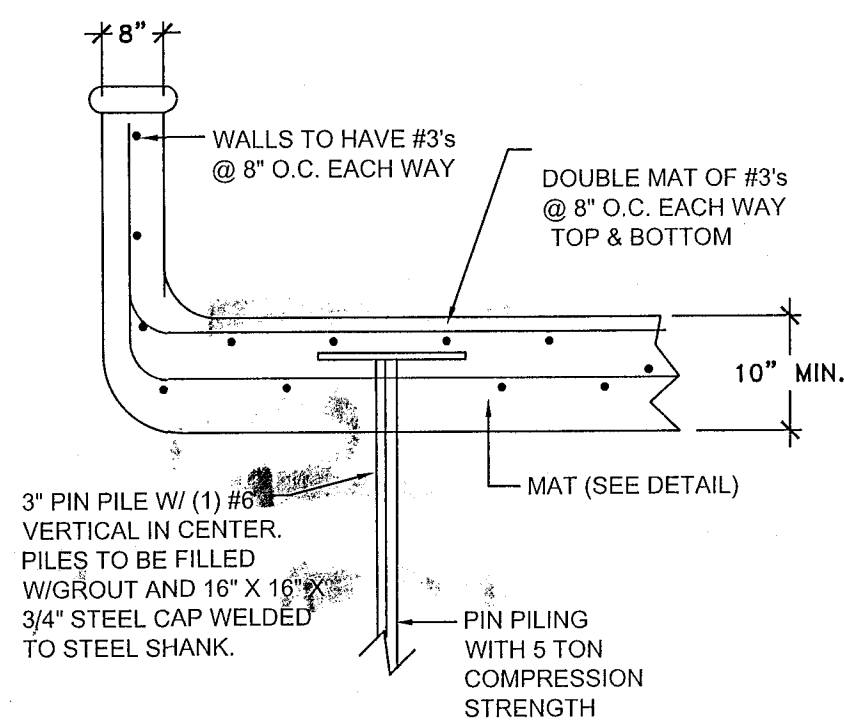
EQUAL

EQUAL

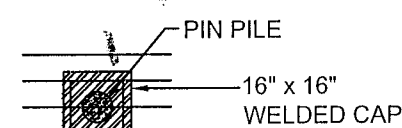
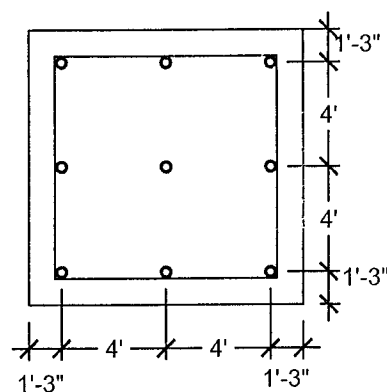
LAYOUT POINTS TAKEN FROM WATERLINE OF EXISTING POOL

EXISTING POOL TO REMAIN

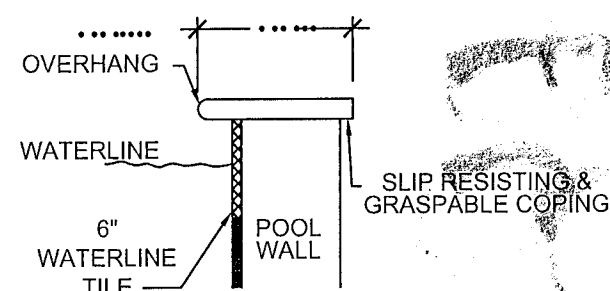
1. PILING SHALL BE INSTALLED PER SOIL REPORT AND MANUFACTURER'S RECOMMENDATIONS.
2. REINFORCING STEEL SHALL COMPLY WITH ASTM A615 GRADE 60.
3. CONCRETE SHALL BE PNEUMATICALLY APPLIED WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.



PIN PILE DETAIL
N.T.S.



MAT OVER EACH PILING
TOP VIEW
N.T.S.



COPING DETAIL
N.T.S.

1. ALL CONSTRUCTION AND WORKMANSHIP SHALL BE IN CONFORMITY WITH FBC-RESIDENTIAL 2010 CHAPTER 41.
2. REFER TO SOIL REPORT FOR SOIL ANALYSIS.
3. SWIMMING POOL TO HAVE PNEUMATICALLY PLACED CONCRETE FLOOR, WALL AND BOND BEAM. CONCRETE TO HAVE 28 DAY COMPRESSIVE STRENGTH OF 2,800 P.S.I.
4. ALL REINFORCED STEEL TO BE INTERIM GRADE DEFORMED BARS OF NEW BILLET STEEL: CONFORMING TO ASTM A-615. STEEL TO BE BENT, LAPPED AND PLACED IN ACCORDANCE WITH A.C.I. STANDARDS AND SPECS.
5. IN AREA OF SKIMMER, 2-#3 BARS IN BOND BEAM MAY BE PLACED EITHER BELOW OR BEHIND SKIMMER.
6. ALL PIPING SHALL BE N.S.F. APPROVED AND SHALL BE SCHEDULE 40 PVC.
7. MAIN DRAIN TO HAVE A FREE AREA OF 4 TIMES THE AREA OF THE SUCTION LINE.
8. WATER SUPPLY AND DISPOSAL TO BE ARRANGED SO THAT THERE IS NO CROSS-CONNECTION WITH A DOMESTIC WATER SUPPLY.
9. IF REQUIRED, UNSCREENED POOLS SHALL HAVE A MINIMUM 4 FT. FENCE WITH SELF-CLOSING AND LATCHING GATE.
10. ALL METALLIC POOL FITTINGS WITHIN 5 FEET OF THE INSIDE WALL AND THE DECK REINFORCING SHALL BE BONDED TO THE POOL REINFORCING STEEL WITH A NO. 8 AWG COPPER WIRE. THE POOL REINFORCING STEEL SHALL BE BONDED TO THE POOL LIGHT NICHE WITH NO. 8 AWG COPPER WIRE. TWO NO. 8 AWG COPPER GROUND WIRES SHALL BE RUN WITH N.E.C. APPROVED CONDUIT, ONE INTERNALLY, FROM THE LIGHT NICHE TO THE JUNCTION BOX. COMPLETION OF THE POOL GROUNDING SYSTEM TO THE PANEL BOARD BY ELECTRICIAN.
11. POOL CONSTRUCTION SHALL BEAR ON CLEAN SANDS OR ROCK WITH A BEARING CAPACITY 2,000 P.S.F.
12. FBC-RESIDENTIAL 2010 CHAPTER 41 SECTION R4101.6.1 CONFORMANCE STANDARD: DESIGN, CONSTRUCTION AND WORKMANSHIP SHALL BE IN CONFORMITY WITH THE REQUIREMENTS OF ANSI/NSPI 3, ANSI/NSPI 4, ANSI/NSPI 5, ANSI/NSPI 6 AND ANSI/NSPI 7.

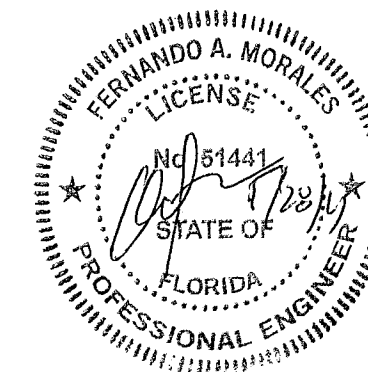
[illegible]

ALL ELECTRICAL WORK SHALL
CONFORM TO N.E.C. ARTICLE #680

PER F.B.C. AMENDMENTS SECTION 104.23 ALTERNATIVE MATERIALS AND METHODS FOR SWIMMING POOL AND SPAS WITH THE INSTALLATION OF 15-VOLT OR LESS SWIMMING POOL LIGHTS OR NO LIGHT, PERIMETER SURFACES AND EQUIPOTENTIAL BONDING GRID (SINGLE WIRE).

PERIMETER SURFACES AND EQUIPOTENTIAL BONDING GRID (SINGLE WIRE) INCLUDES UNPAVED SURFACES AS WELL AS POURED CONCRETE AND OTHER TYPES OF PAVING. BONDING FOR PERIMETER SURFACES SHALL BE INSTALLED AS FOLLOWS:

1. ATTACHED TO THE POOL REINFORCING STEEL OR COPPER CONDUCTOR GRID (SINGLE WIRE) AND
2. BURIED TO A MINIMUM DEPTH OF (4) FOUR INCHES TO (6) SIX INCHES BELOW GRADE AND
3. MINIMUM OF (4) FOUR POINTS UNIFORMLY SPACED AROUND THE PERIMETER OF THE POOL (EXCEPT FOR NONCONDUCTIVE POOL SHELLS, BONDING AT FOUR POINTS SHALL NOT BE REQUIRED) AND
4. CONNECTED TO AN EQUIPOTENTIAL BONDING GRID (SINGLE WIRE) WITH A SOLID COPPER BARE CONDUCTOR NOT SMALLER THAN #8 AWG AND
5. CONNECTION SHALL BE MADE EXOTHERMIC WELDING OR BY LISTED PRESSURE CONNECTORS OR CLAMPS THAT ARE LABELED AS BEING SUITABLE FOR THE PURPOSE AND ARE OF STAINLESS STEEL, BRASS, COPPER OR COPPER ALLOY AND
6. THE EQUIPOTENTIAL BONDING GRID (SINGLE WIRE) SHALL EXTEND UNDER PAVED WALKING SURFACES FOR MINIMUM 18" AND MAXIMUM 24" HORIZONTALLY BEYOND THE INSIDE WALLS OF THE POOL AS ALLOWED UNDER EXISTING CONDITIONS.



SHEET 2: PILING DETAILS & ELECTRICAL

SNYDER RESIDENCE
190 S. HIBISCUS DRIVE
MIAMI BEACH

LEGAL: LOT 28, BLOCK 3
HIBISCUS ISLAND

JOB NO. 35055

214-04200

in
housen
de

2015

JB

11-29-18

ENG LTR.

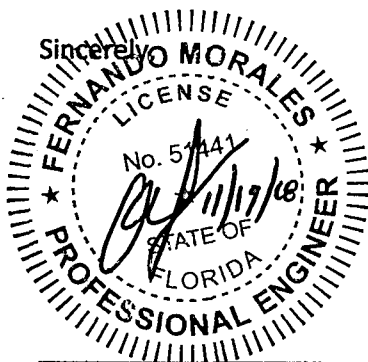
on do accept
11/24/18

November 19th, 2018

To: Building Department
City of Miami Beach

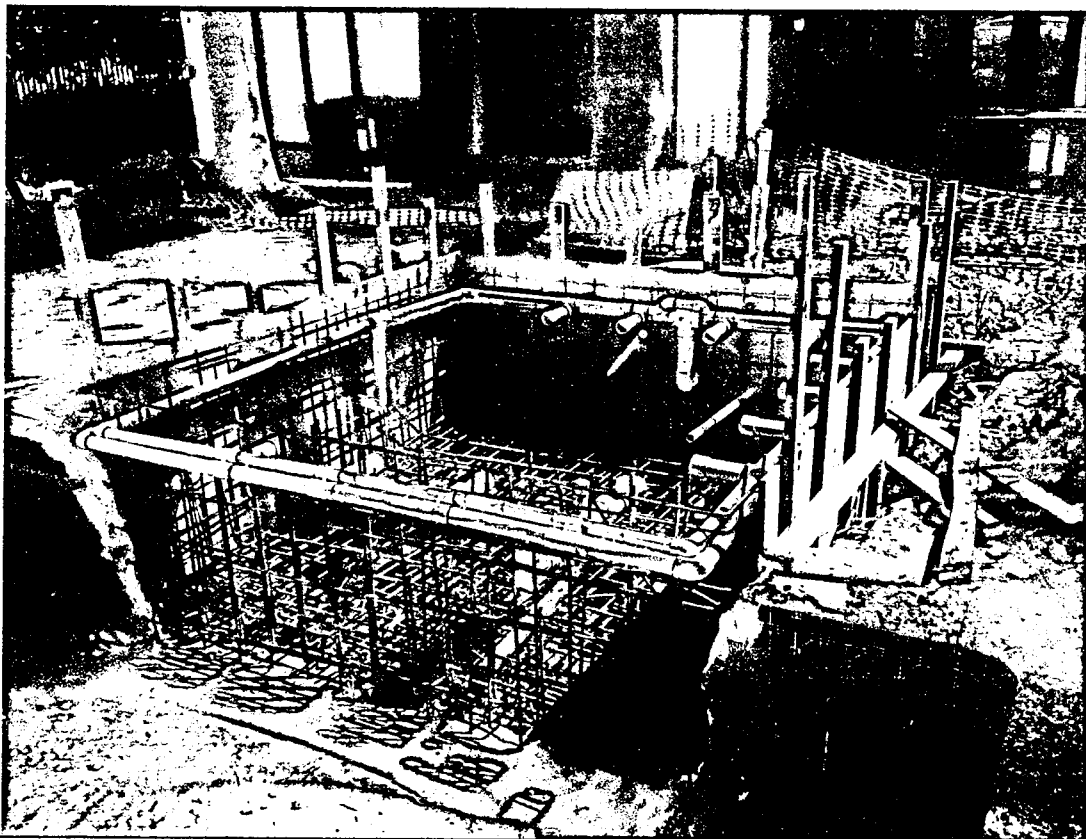
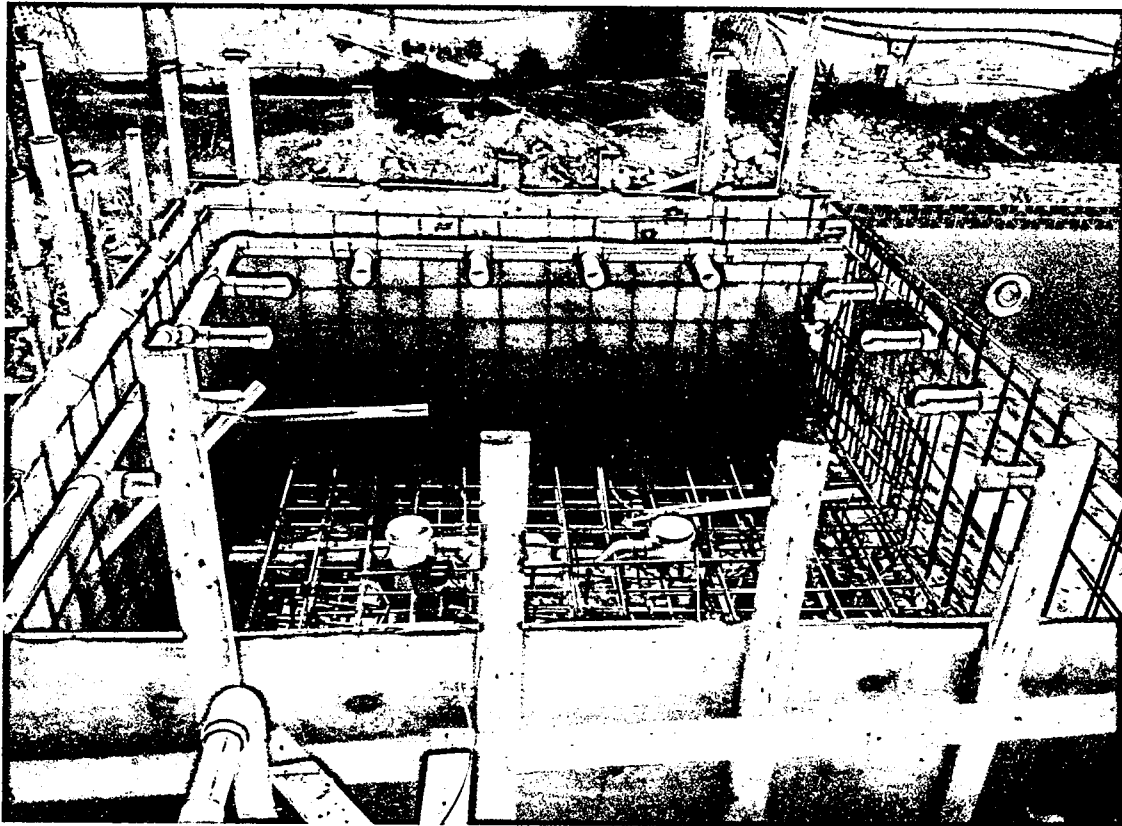
RE: Snider Residence's missing foundation inspection
Permit B1505842
190 S. Hibiscus Dr
Miami Beach, FL .

I Fernando Morales, P.E. (EOR) engineer of record of the Spa construction plans at the above mentioned address, permit No. B1505842, after field reviewed the project including the attached photographs (Exhibit # 1) of the spa reinforced steel, and without performing any intrusive or destructive testing, I can attest to the best of my knowledge that the reinforced steel in the spa was installed according to the permitted plans. If you have any question please do not hesitate in call me at (786) 380-9739.



Fernando Morales, P.E.
5201 SW 162 PL
Miami, FL 33186
(786) 380-9739

EXHIBIT # 1



CITY OF MIAMI BEACH
Building Department
1700 Convention Ctr Drive, 2nd Floor
Miami Beach, Florida 33139
Inspections: (305) 673-7370
Office: (305) 673-7610

B1400193 BCO15332 CO

Certificate of Occupancy
Certificate Number: BCO15332

Status: APPROVED

Issued By: BUILROBM

Site Address: 190 S HIBISCUS DR MBCH
Parcel #: 32320060870

Applied: 08/10/2015
Issued: 08/24/2015
Extended:
To Expire:

Tenant:

Property Owner: TODD DAVID SNYDER & W JESSICA
190 S HIBISCUS DR
MIAMI BEACH FL 331395130

Class Code: R3

Issued For: CO/ New 2 story addition , existing kitchen renovation and
new corridor at existing 2nd floor.

Temporary Expiration Date:


Current Use: SFR
Previous Use:

OCCUPANCY INFORMATION

Building Permit #: B1400193	Zoning Use District: RS-3
Occupancy Group: R3	Construction Type:
Maximum Occupant Content	Minimum Number of Exits:
Zoning Ordinance Number: 89-2665	SS # or Taxpayer ID#:

This is to certify that the above tenant, whose address is noted above, has filed for premission to use the property located at the address noted above, and said proposed use or uses being in comformity with the provisions of the zoning ordinance 89-2665 and the Building Code of the City of Miami Beach, a Certificate of Occupancy is hereby granted to use said building for the purpose described below, subject to any special condition(s) detailed in this document.

NOTE: Any unauthorized additions, alterations or change in use of this property will void this Certificate of Occupancy.

 8/24/15

Building Official Signature and Date
MARIANO V. FERNANDEZ, PE

This Certificate of Occupancy is valid only if there is an APPROVED Status and a Building Official Signature.
[CERTOCCP14]

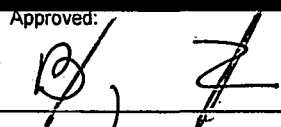
MIAMI BEACH

Building Department
1700 Convention Center Drive, 2nd FL
Miami Beach, Florida 33139
Tel: 305.673.7610 ext. 6868

Request for Certificate of Occupancy or Completion

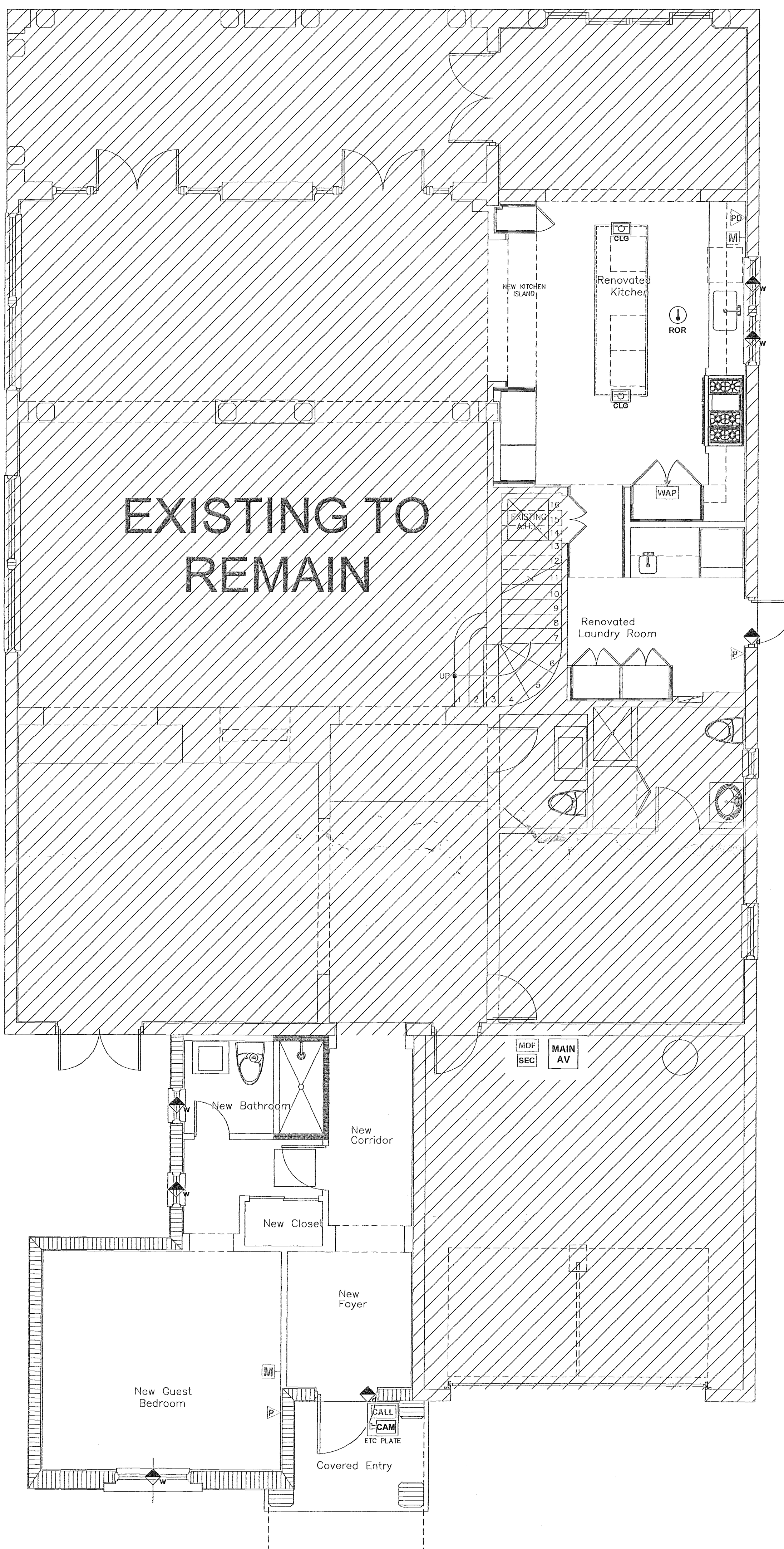
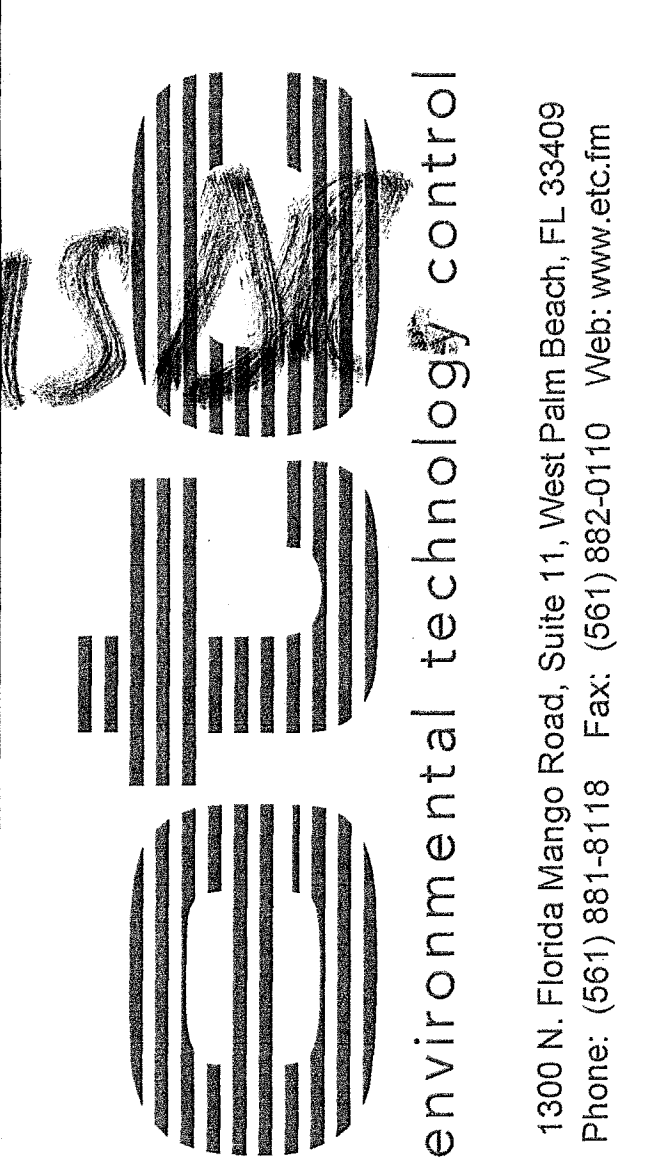
<input checked="" type="checkbox"/> Certificate of Occupancy	<input checked="" type="checkbox"/> Certificate of Completion
Date 8.5.15	Permit Number B1400193 / BCO15332
Job Address 190 S HIBISCUS DR.	Tax Folio # 3232000080 / RS-3
Unit/Suite #	Occupancy/Use RESIDENTIAL / R3/N
Total square feet for this CO/CC request 829 sf.	Number of Residential Units for this CO/CC request 1
Contractor JAROSZ DEVELOPMENT CORP	Mailing address 3320 MARY ST #900
Telephone Number	E-Mail Address ARLYN@JAROSZARCH.COM
Owner/Tenant TODD & JESSICA M SNYDER	Mailing address 190 S HIBISCUS DR. MIAMI BEACH, FL 33139
Telephone Number	E-Mail Address
Contact Name ARLYN VAZQUEZ	Contact Telephone Number 786 942 7448

- New 2 story addition

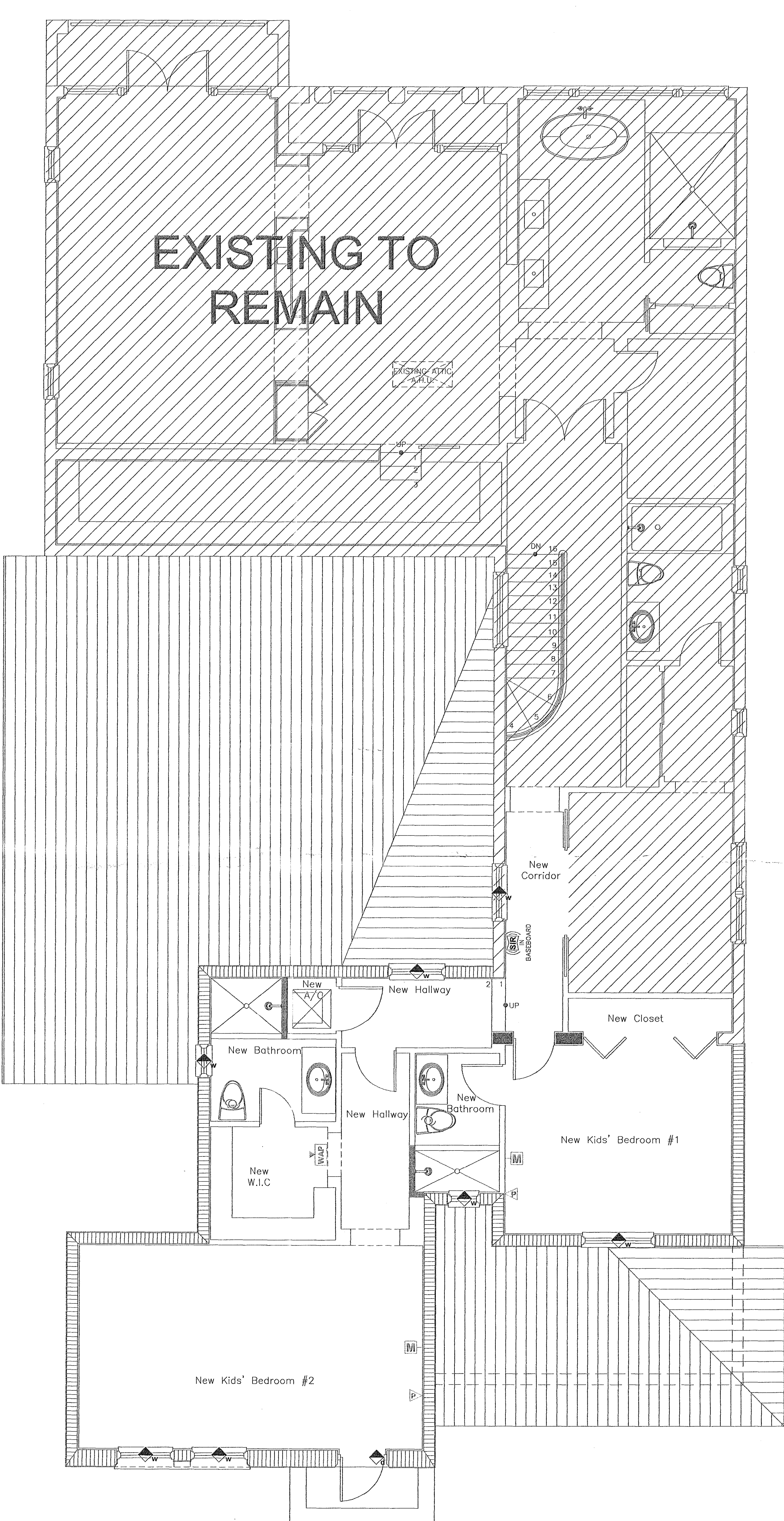
Office Use Only	
Approved: 	Mariano Fernandez, P.E. Building Official/Director

- Net cost - 8/10/15
- pend. zoning -
- pend payout -

BE141492
B1400193
190 S HIBISCUS



**FIRST FLOOR
LOW VOLTAGE PLAN**
REFER TO "NOTE" PAGE FOR LEGENDS AND NOTES



**SECOND FLOOR
LOW VOLTAGE PLAN**
REFER TO "NOTE" PAGE FOR LEGENDS AND NOTES

AUDIO/VIDEO/CONTROL PREWIRE SCHEDULE				
SYMBOL	DESCRIPTION	CABLE(S)	WIRE TO	MOUNTING/HEIGHT
MAIN AV	MAIN AV EQUIPMENT RACK	4-CAT6e (Blue) 3-RG6	MDF MDF	
WALL/CEILING	RECT. RECESSED SPEAKER	1-16/2 (White)	Main AV	Field locate per drawing.

STRUCTURED WIRING PREWIRE SCHEDULE				
SYMBOL	DESCRIPTION	CABLE(S)	WIRE TO	MOUNTING/HEIGHT
CALL	ENTRY CALL STATION	2-CAT6e (White)	MDF	64" on center
	MAIN DISTRIBUTION FRAME	1-CAT6e (White) 1-CAT6e (Blue) 2-RG6 1-Conduit (1") 1-#10 Ground Wire (Green - solid)	TELCO TELCO CABLESCO Main AV TELCO	
M	MULTIMEDIA OUTLET	1-CAT6e (White) 1-CAT6e (Blue) 2-RG6	MDF MDF MDF	Match Electrical
P	PHONE OUTLET	1-CAT6e (White)	MDF	Match Electrical
PD	PHONE/DATA OUTLET	1-CAT6e (White) 1-CAT6e (Blue)	MDF MDF	Match Electrical Match Electrical
WAP	WIRELESS ACCESS POINT	1-CAT6e (Blue) 1-16/2 (Blue)	MDF MDF	Locate high on wall in discrete location.

SECURITY-CAMERA PREWIRE SCHEDULE				
SYMBOL	DESCRIPTION	CABLE(S)	WIRE TO	MOUNTING/HEIGHT
CAM	CCTV FIXED CAMERA	1-RG59 Copper (WR) 1-16/2 (WR) 1-CAT6 (WR)	MDF MDF MDF	
ROR	HEAT DETECTOR (Rate of Rise)	1-22/6 FIRE/PLENUM	SECURITY PANEL	
(SIR)	SECURITY SIREN	1-22/4	SECURITY PANEL	Coordinate on site
D	DOOR CONTACT SENSOR	1-22/4	SECURITY PANEL	Door Jamb
W	WINDOW CONTACT SENSOR	1-22/4	SECURITY PANEL	Window Jamb

Numbered Notes:
1 All security contacts for doors and operable windows to be coordinated with GC on site.

Ventilation Notes:
EQUIPMENT CLOSET - When equipment is mounted in single or multiple racks.
Requirements:
1. An exhaust fan (similar to bathroom) using either a remote fan or a quiet one (Panasonic).
2. There needs to be adequate "pass-through" ventilation using louvered doors or as a minimum, a 12" x 30" grill on the bottom of the door(s).
EQUIPMENT CABINET - When equipment is mounted in a rack or sitting/stacked on shelves.
Requirements:
1.A. An exhaust fan (similar to bathroom) using either a remote fan or a quiet one (Panasonic).
- or -
1.B. Adequate ventilation behind the cabinet (leading into the ceiling or opening to the room) must be provided with two 3" x 3" holes.
2. If either method is used there must also be adequate "pass-through" ventilation using grille cloth or lattice at least 8" x 8".

NOTICE: In addition to the requirement of this permit there may be additional requirements applicable to this project that may be found in the Public Records of this County and there may be additional permits required from other agencies, or federal agencies, and the City of Miami Beach assumes no responsibility for accuracy of or results from these plans when are approved subject to compliance with all Federal, State, and Local Laws, Rules, and Regulations.

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:

BUILDING: 03/20/14
ZONING: 03/20/14
PLUMBING: 03/20/14
ELECTRICAL: 03/20/14
MECHANICAL: 03/20/14
FIRE PREVENTION: 03/20/14
FLOOD: 03/20/14
PUBLIC WORKS: 03/20/14
STRUCTURAL: 03/20/14
ELEVATOR: 03/20/14

* PERMIT SET *

**Snyder Residence
190 S. Hibiscus Drive
Miami Beach, FL 33139**

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REV	DESCRIPTION	DATE	REV	CHK	BY
1	Generated.	03/03/14	SC		

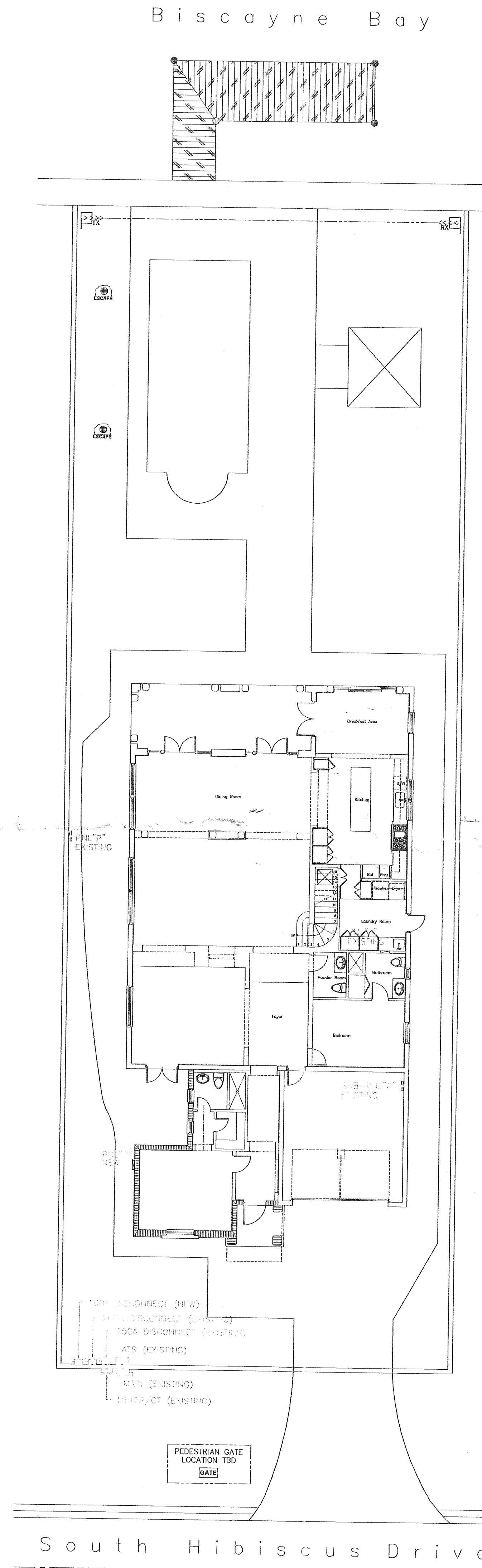
DATE: **03/03/14**

SCALE: **1/4" = 1'-0"**


DRAWN BY: **SC** CHECKED BY: **LBO** CLIENT REP:




DRAWING NO: **LV**

3-18-14



**SITE LAYOUT
LOW VOLTAGE PLAN**
REFER TO "NOTE" PAGE FOR LEGENDS AND NOTES

AUDIO/VIDEO/CONTROL PREWIRE SCHEDULE				
SYMBOL	DESCRIPTION	CABLE(S)	WIRE TO	MOUNTING/HEIGHT
	LANDSCAPE SPEAKER	1-16/2 (White)	Main AV	Field Locate per Drawing

SECURITY-CAMERA PREWIRE SCHEDULE				
SYMBOL	DESCRIPTION	CABLE(S)	WIRE TO	MOUNTING/HEIGHT
	PERIMETER BEAM RCVR	1-18/6	SECURITY PANEL	
	PERIMETER BEAM XMTR	1-18/6	SECURITY PANEL	
	GATE CONTROLS	2-CAT6e (White) 1-16/4 1-RG99	MDF	

*** PERMIT SET**

**Snyder Residence
190 S. Hibiscus Drive
Miami Beach, FL 33139**

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REV	DESCRIPTION	DATE	REV BY	CHK BY
1	Generated.	03/03/14	SC	

DATE: **03/03/14**

SCALE: **3/32" = 1'-0"**

DRAWN BY: **SC** CHECKED BY: **LBO** CLIENT REP:

DRAWING NO: **SITE**

Signature 3-18-14

BE141492

B1400193

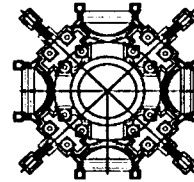
190 S Hibiscus

Dn

Office
Copy

Z.W. Jarosz Architect, P.A.

3326 Mary Street, 5th Floor
Coconut Grove, Florida 33133
(305) 446 0888, fax (305) 447 1177



Date: 5/30/2014

Ref: Snyder Residence

Process Number: B1400193

Dear Building Inspector:

Please see below for sheets affected by revision 2.

The scope of revision 2 entails bringing down the 2nd floor elevation of the eastern part of the proposed addition to match the western part of the proposed addition as well as the 2nd floor elevation of the existing residence. The layout of New Bathroom #204 has been slightly altered in order to avoid a conflict with the toilet pipe in the ceiling of New Corridor #106. The layout of New Bathroom #209 and New W.I.C. #208 has also been revised so that each room may have its own entrance.

Sheet A2.01 – Note that steps have been removed from the proposed addition at the 2nd floor, and the proposed elevation throughout the addition matches the elevation of existing residence. Note revised layout of New Bathroom #204, New W.I.C. #208, and New Bathroom #209.

Sheet A3.01 – Note revised elevation of 2nd floor.

Sheet A3.02 – Note revised elevation of 2nd floor.

Sheet A3.03 – Note revised elevation of 2nd floor.

Sheet A3.20 – Note revised elevation of 2nd floor.

Sheet A5.01 – Door #14 added.

NOTICE: In addition to the requirements of the City of Miami Beach, there may be additional restrictions applicable to this project. The City of Miami Beach assumes no responsibility for accuracy of or results from these plans which are approved subject to compliance with all Federal, State, and Local Laws, Rules, and Regulations.

STRUCTURAL
ANSWER TO COMMENTS
FOR
BRODSKY RESIDENCE

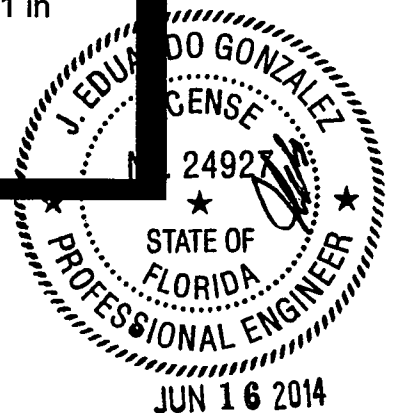
AT

5900 SW 113TH STREET
MIAMI, FLORIDA, 33156

BY

J. EDUARDO GONZALEZ, P.E., INC.
STRUCTURAL ENGINEERS
717 PONCE DE LEON BLVD., SUITE 209
CORAL GABLES, FLORIDA 33134
Tel. (305) 445-5100
Registration No. PE 24927
EB No. 0006188

This cover sheet is provided as per Florida Statute 61G15-31 in
lieu of signing and sealing each individual sheet.



ANSWER TO COMMENTS

PINE TREE DRIVE RESIDENCE
5900 SW 113th STREET
PINECREST, FLORIDA

Process No. BL 2014-0737

1. Architect / Engineer Team :Provide special inspector letters for piling (FBC R4404.6.1.20),soil compaction (FBC 1820.3.1),reinforced masonry (FBC R4407.5.4),trusses over 35 ft long or 6 ft high (FBC 2319.17.2.4.2),structural steel connection(FBC R4408.5.2),precast units and attachments (FBC R4405.9.12.2),curtain wall systems (FBC R4410.5.7),to comply with FBC 2010. PLEASE BE AWARE THAT PERMIT CANNOT BE CLOSED UNLESS WE RECEIVE ALL REPORTS,SIGNED AND SEALED.

Please find the special inspector form attached.

2. This is a Level 3 alteration as per FBC 2010 Existing Building Code New structural members ,including connection and anchorage ,shall comply with FBC building . The minimum design loads on existing elements the structure that do not support additional loads as a result of the alteration shall be the loads applicable at the time the building was constructed .An engineering evaluation and analysis that establishes the structural adequacy of the altered structure(both existing and new elements) must be prepared by S.E.R (or Architect) and submitted as a signed and sealed report to the building official. Since more than 30 percent of the total sum of.... floor and roof areas affected ,the evaluation and analysis shall demonstrate that the altered building or structure complies the 2010 FBC Building for wind loading . this applies to bolt horizontal pressures and uplift.

Acknowledge, on our design we calculate all new and existing structural member according with the actual Code (2010 FBC).

3. The existing walls are being raised in height. Provide details that provides continuity of wall reinforcing from foundation ,thru existing tie beam ,and hooking into new higher tie beam. Design walls for increased height.

Acknowledge, see revised section 3/S-5, and also the legend at S-1 where all new and existing masonry wall will reinforcing according with the calculation.

4. S-1 : Comply with FBC 2010 Appendix B, Section B303.4.5 and place diagonal corner bars at all reentrant corner .Provide size ,length, distance from top of slab, spacing and location in plan. Please read referenced code section carefully and comply with specified working.

Acknowledge, see revised drawing S-1 with corner bar detail according with Appendix B section B303.4.5.

5. **Detail F/S-4 : truss top and bottom chords will need to be connected by a gang nail plate and your connection would look different. Consult with truss manufactured and revise section to fit actual conditions.**

Acknowledge, please find revised detail F where the top cord and bottom cord are connect.

6. **Masonry calculations: Since $K_d=0.85$ is being used, do not overstress masonry or reinforcing .ACI 530 does not tell how to calculate wind. Increases in allowable stresses (as explained in ASCE 7-98,-02,-05 and -10) can only be made on materials where it can be demonstrated that such an increase is justified by structural behavior caused by rate or duration of load. Only wood and glass qualify.**

Acknowledge, see calculation attached and also revised structural drawings with #5@16" OC at zone 5 and #5@32" OC at zone 4.

7. **Calculation Page 66 : Where are these pilaster? Is this supposed to be calculation for reinforcing at sides of opening ? well height is 12'-4" ,not 10'-4" and tributary with for gravity and wind loads extends from half the opening width to half the distance between the innermost boundary reinforcing bar to the first adjacent wall reinforcing (or tie column) bar .Compression block extends from the same point to the edge of the opening (maximum 48 inches) .**

Acknowledge, see calculation attached and also revised drawings show the concrete column at each side of all opening.

8. **S-2 : Dimension setback from outer wall corner for all girder trusses in hip roofs ,to comply with 61G15-31.003(2)(d) in Florida State Rules and ANSI/TPI 1-2002 and also FBCR4409.6.10.**

Acknowledge, see revised structural roof framing plan with all girder truss location from the edge of the corner wall.





J. EDUARDO GONZALEZ P.E., INC.
STRUCTURAL ENGINEERS
717 Ponce de Leon Blvd. Suite 210
Coral Gables, Florida 33134
Tel. (305) 445-5100 Fax (305) 445-6644

SPECIAL INSPECTOR

Date: 6/12/14

To: Building Official
Village of Pinecrest
12645 Pinecrest Parkway
Pinecrest, Florida 33156

NOTICE TO BUILDING & ZONING DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR UNDER THE FLORIDA BUILDING CODE

I, (We) have been retained by the (homeowner), to perform special inspection services under the Florida Building Code, at the project located at (address) on the below listed items as of the date listed above. I am a registered engineer/architect licensed in the State of Florida.

Permit Numbers _____

- ☐ Special Inspector for PILING (FBC R4404.6.1.20)
☒ Special Inspector for TRUSSES OVER 35 FT LONG OR 6 FT HIGH (FBC R4409.6.17.2.4.2)
☒ Special Inspector for REINFORCED MASONRY (FBC R4407.5.4)
☐ Special Inspector for STEEL CONNECTIONS (FBC R4408.5.2)
☐ Special Inspector for SOIL COMPACTION (FBC R4404.4.3.2)
☐ Special Inspector for PRECAST UNITS & ATTACHMENTS (FBC R4405.9.12.2)
☐ Special Inspector for _____

Note: Only the marked boxes apply.

The following individual(s) employed by me or this firm are authorized to perform inspections.*

1. YOLY LOPEZ
2. OSCAR WIS RODRIGUEZ

*Special Inspectors utilizing authorized representatives shall insure the authorized representative is qualified by education or licensure to perform the duties assigned by the Special Inspector. The qualifications shall include licensure as a professional engineer or architect; graduation from an engineering education program in civil or structural engineering; graduation from an architectural education program; successful completion of the NCEES Fundamentals Examination; or registration as building inspector or general contractor.

I (We) will notify the Building Department of any changes regarding authorizing personnel performing inspection services.

I (We) understand that a Special Inspector's inspection log for each building must be displayed in a convenient location on the site for reference by the Building inspectors. All mandatory inspections, as required by the Florida Building Code, must be performed by the county. The inspections from the Building Department must be called for on all mandatory inspections. Inspections performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Department. Further, upon completion of the work under each building permit, I will submit the completed inspection log form and sealed Statement of Compliance to the building inspector at the time of final inspection and before making application for Certificate of Occupancy. The Statement of Compliance shall state that to the best of my knowledge, belief and professional judgment that portions of the project outlined above meets with the intent of the Florida Building Code and in substantial accordance with the approved Plans.

YOLY LOPEZ 06/12/2014
Engineer or Architect's signature
State Registration Number
Sign & Seal

BASED ON FBC 2010 RESIDENTIAL CODE

MecaWind Pro v2.2.4.7 per ASCE 7-10

Developed by MECA Enterprises, Inc. Copyright www.mecaenterprises.com

Date : 6/13/2014 Project No. : 1419
 Company Name : EDUARDO GONZALEZ PE Designed By : MM
 Address : 717 PONCE DE LEON BLVD Description :
 City : CORAL GABLES Customer Name :
 State : FLORIDA Proj Location :
 File Location: C:\NUGYear-2014\1419\CALCULATION\WIND LOADS WALL wnd.wnd

Input Parameters: Directional Procedure All Heights Building (Ch 27 Part 1)

Basic Wind Speed(V)	=	175.00 mph	Exposure Category	=	C
Structural Category	=	II	Flexible Structure	=	No
Natural Frequency	=	N/A	Kd Directional Factor	=	1.00
Importance Factor	=	1.00			
Damping Ratio (beta)	=	0.01			
Alpha	=	9.50	Zg	=	900.00 ft
At	=	0.11	Bt	=	1.00
Am	=	0.15	Bm	=	0.65
Cc	=	0.20	l	=	500.00 ft
Epsilon	=	0.20	Zmin	=	15.00 ft
Slope of Roof	=	3 : 12	Slope of Roof(Theta)	=	14.04 Deg
Ht: Mean Roof Ht	=	15.71 ft	Type of Roof	=	HIPPED
RHt: Ridge Ht	=	20.08 ft	Eht: Eave Height	=	11.33 ft
OH: Roof Overhang at Eave	=	3.00 ft	Overhead Type	=	OH w/ soffit
Bldg Length Along Ridge	=	129.17 ft	Bldg Width Across Ridge	=	64.00 ft
Length of Hipped Ridge	=	89.00 ft	Roof Slope on Hip End	=	20.76 Deg

Gust Factor Calculations

Gust Factor Category I Rigid Structures - Simplified Method
 Gust1: For Rigid Structures (Nat. Freq.>1 Hz) use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis
 Zm: $0.6 \cdot Ht$ = 15.00 ft
 lzm: $Cc \cdot (33/Zm)^{0.167}$ = 0.23
 Lzm: $1 \cdot (Zm/33)^{Epsilon}$ = 427.06 ft
 Q: $(1/(1+0.63 \cdot ((B+Ht)/Lzm)^{0.63}))^{0.5}$ = 0.91
 Gust2: $0.925 \cdot ((1+1.7 \cdot lzm \cdot 3.4 \cdot Q)/(1+1.7 \cdot 3.4 \cdot lzm))$ = 0.88

Gust Factor Summary
 Not a Flexible Structure use the Lessor of Gust1 or Gust2 = 0.85

Table 26.11-1 Internal Pressure Coefficients for Buildings, GCpi

GCpi : Internal Pressure Coefficient = +/-0.18

Wind Pressurs Main Wind Force Resisting System (MWFRS) - Ref Figure 27.4-1

Kh: $2.01 \cdot (Ht/Zg)^{(2/Alpha)}$ = 0.86
 Kht: Topographic Factor (Figure 6-4) = 1.00
 Qh: $.00256 \cdot (V)^2 \cdot I \cdot Kh \cdot Kht \cdot Kd$ = 40.32 psf
 Cpww: Windward Wall Cp(Ref Fig 6-6) = 0.80
 Roof Area = 9876.75 ft^2
 Reduction Factor based on Roof Area = 0.80

MWFRS-Wall Pressures for Wind Normal to 129.17 ft Wall (Normal to Ridge)

All pressures shown are based upon ASD Design, with a Load Factor of 1.6

Wall	Cp	Pressure +GCpi (psf)	Pressure -GCpi (psf)
Leeward Wall	-0.50	-24.39	-9.88
Side Walls	-0.70	-31.25	-16.73

Wall	Elev ft	Kz	Kzt	Cp	qz psf	Press +GCpi	Press -GCpi	Total +/-GCpi
Windward	11.33	0.85	1.00	0.80	39.93	19.90	34.41	44.29
Windward	1.33	0.85	1.00	0.80	39.93	19.90	34.41	44.29

Roof Location	Cp	Pressure +GCpi (psf)	Pressure -GCpi (psf)
Windward - Min Cp	-0.54	-25.76	-11.25
Windward - Max Cp	-0.03	-8.29	6.23

Leeward Norm to Ridge	-0.46	-23.02	-8.51
Hipped End (.00 to 7.85 ft)	-0.90	-38.10	-23.59
Hipped End (7.85 to 15.71 ft)	-0.90	-38.10	-23.59
Hipped End (15.71 to 31.41 ft)	-0.50	-24.39	-9.88
Hipped End (31.41 to 70.00 ft)	-0.30	-17.54	-3.02
Overhang Bottom Side (Windward only)	0.80	27.15	27.15

Notes - Normal to Ridge

- Note (1) Per Fig 27.4-1 Note 7, Since $\Theta > 10$ Deg base calcs on Mean Ht
Note (2) Wall & Roof Pressures = $Qh(G \cdot C_p - GC_{pi})$
Note (3) $+GC_{pi}$ = Positive Internal Bldg Press, $-GC_{pi}$ = Negative Internal Bldg Press
Note (4) Total Pressure = Leeward Press + Windward Press (For + or - GC_{pi})
Note (5) Hipped ends considered as parallel to ridge for all Θ .
Note (6) Ref Fig 27.4-1, Normal to Ridge ($\Theta > 10$), $\Theta = 14.0$ Deg, $h/l = 0.12$
Note (7) Overhang bottom based upon windward wall C_p and $GC_{pi} = 0$.
Note (8) X = Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (9) MIN = Minimum pressures on Walls = 9.6 psf and Roof = 4.8 psf
Note (10) Area* = Area of the surface projected onto a vertical plane normal to wind.

MWFRS-Wall Pressures for Wind Normal to 64 ft wall (Along Ridge)

All pressures shown are based upon ASD Design, with a Load Factor of 1.6

Wall	Cp	Pressure +GCpi (psf)	Pressure -GCpi (psf)
Leeward Wall	-0.30	-17.51	-2.99
Side Walls	-0.70	-31.25	-16.73

Wall	Elev ft	Kz	Kzt	Cp	qz psf	Press +GCpi	Press -GCpi	Total +/-GCpi
Windward	20.08	0.90	1.00	0.80	42.46	21.62	36.13	39.12
Windward	11.33	0.85	1.00	0.80	39.93	19.90	34.41	37.40
Windward	1.33	0.85	1.00	0.80	39.93	19.90	34.41	37.40

Roof - Dist from Windward Edge	Cp	Pressure +GCpi (psf)	Pressure -GCpi (psf)
Roof: 0.0 ft to 7.9 ft	-0.90	-38.10	-23.59
Roof: 7.9 ft to 15.7 ft	-0.90	-38.10	-23.59
Roof: 15.7 ft to 31.4 ft	-0.50	-24.39	-9.88
Roof: 31.4 ft to 135.2 ft	-0.30	-17.54	-3.02

Notes - Along Ridge

- Note (1) Ref Fig 27.4-1, Parallel to Ridge (All), $h/l = 0.12$
Note (2) X = Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (3) MIN = Minimum pressures on Walls = 9.6 psf and Roof = 4.8 psf
Note (4) Area* = Area of the surface projected onto a vertical plane normal to wind.

Total Base Reaction Summary

Description	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Normal to Ridge Walls+Roof +GCpi	-0.0	62.2	238.4	1020.2	-0.0	0.0
Normal to Ridge Walls Only +GCpi	.0	64.8	.0	367.2	.0	.0
Normal to Ridge Walls+Roof -GCpi	-0.0	79.1	32.3	68.6	-0.0	0.0
Normal to Ridge Walls Only -GCpi	.0	64.8	.0	367.2	.0	.0
Normal to Ridge Walls+Roof MIN	.0	23.4	.0	225.4	.0	.0
Along Ridge Walls+Roof +GCpi	21.0	0.0	204.7	-0.0	-1811.6	-0.0
Along Ridge Walls Only +GCpi	27.1	.0	.0	.0	-153.6	.0
Along Ridge Walls+Roof -GCpi	21.0	0.0	62.0	-0.0	-1856.4	-0.0
Along Ridge Walls Only -GCpi	27.1	.0	.0	.0	-153.6	.0
Along Ridge Walls+Roof MIN	10.0	.0	.0	.0	-83.3	.0

Notes Applying to MWFRS Reactions:

- Note (1) Per Fig 27.4-1, Note 9, Use greater of Shear calculated with or without roof.
Note (2) X = Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (3) MIN = Minimum pressures on Walls = 9.6 psf and Roof = 4.8 psf
Note (4) MIN area is the area of the surface onto a vertical plane normal to wind.
Note (5) Total Roof Area (incl OH Top) = 9876.75 sq. ft

Wind Pressure on Components and Cladding (Ch 30 Part 1)

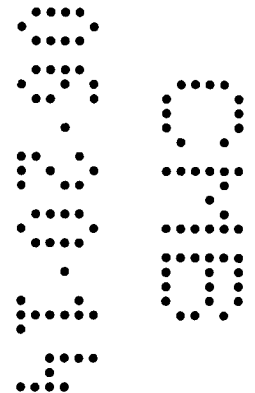
All pressures shown are based upon ASD Design, with a Load Factor of 1.6

Width of Pressure Coefficient Zone "a" = 6.28 ft

Description	Width ft	Span ft	Area ft^2	Zone	Max GCp	Min GCp	Max P psf	Min P psf
WALL	1.00	12.33	50.7	4	0.88	-0.98	42.56	-46.59
WALL	1.00	12.33	50.7	5	0.88	-1.15	42.56	-53.67

Khcc:Comp. & Clad. Table 6-3 Case 1
 Qhcc:.00256*V^2*Khcc*Kht*Kd

= 0.86
 = 40.32 psf



J. EDUARDO GONZALEZ, P.E., INC.
Structural Engineers
 717 Ponce De Leon Blvd. Ste. 210
 Coral Gables, Florida 33134
 (305) 445-5100 Fax (305) 445-6644
 P.E. #24927 STATE OF FL EB-0006188

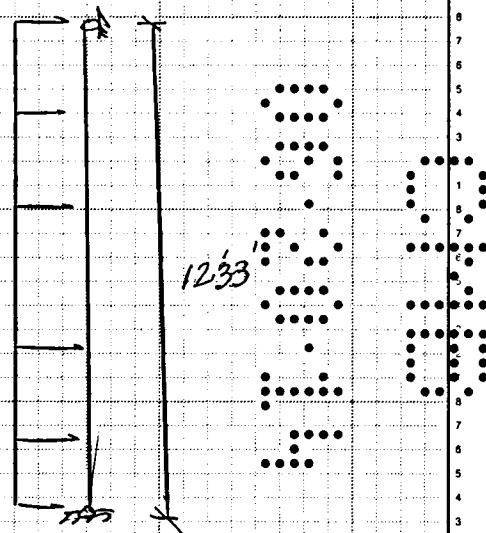
JOB Brodsky Residence
 SHEET NO. _____ OF 14/19
 CALCULATED BY M.M. DATE 5/7/2014.
 CHECKED BY _____ DATE _____
 SCALE _____

Masonry wall design.

Roof { $DC = 25 \text{ psf} \times (45/2 + 3) = 638 \text{ lb/ft}$
 $CL = 30 \text{ psf} \times (45/2 + 3) = 765 \text{ lb/ft}$

wind loads { Zone 4 - 40.59 psf.
 Zone 5 - 53.67 psf.

Use 8" masonry wall w/ #5 @ 32" o.c. Zone 4
 w/ #5 @ 16" o.c. Zone 5.



EDUARDO GONZALEZ PE
717 PONCE DE LEON BLVD
CORAL GABLES

Title :
Engineer:
Project Desc.:

Job #

MM

Masonry Slender Wall

File: S:\DWG\Year-2014\1419\CALCULATION\beam design.ec6
ENERCALC, INC. 1983-2011, Build:6.12.6.7, Ver:6.13.5.31

License #: KW-06006869

Licensee: J. EDUARDO GONZALEZ PE, INC.

Description : MASONRY WALL DESIGN ZONE 4

Code References

Calculations per ACI 530-08/MSJC 2009 Sec. 3.3.5, IBC 2009, CBC 2010, ASCE 7-05

Load Combinations Used : 2006 IBC & ASCE 7-05

General Information

Calculations per ACI 530-08/MSJC 2009 Sec. 3.3.5, IBC 2009, CBC 2010, ASCE 7-05

Construction Type : Grouted Hollow Concrete Masonry

F'm	=	1.50 ksi	Nom. Wall Thickness	8 in	Temp Diff across thickness	=	deg F
Fy - Yield	=	60.0 ksi	Actual Thickness	7.625 in	Min Allow Out-of-plane Defl Ratio	=	360.0
Fr - Rupture	=	61.0 psi	Rebar "d" distance	3.750 in	Minimum Vertical Steel %	=	0.0020
Em = fm *	=	900.0	Lower Level Rebar . . .				
Max % of ρ bal.	=	0.1182	Bar Size	# 5			
Grout Density	=	140 pcf	Bar Spacing	32.0 in			
Block Weight		Normal Weight					
Wall Weight	=	58.0 psf					

Wall is grouted at rebar cells only

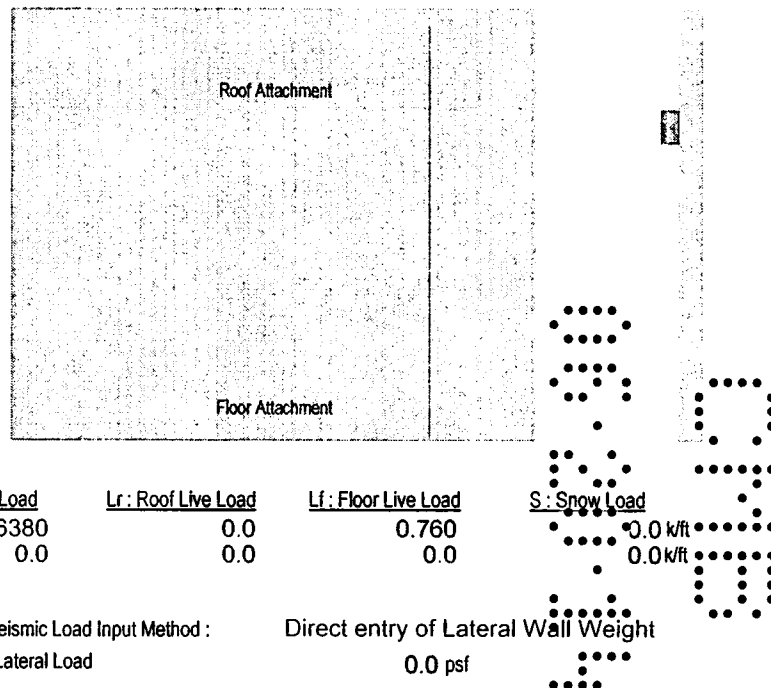
One-Story Wall Dimensions

A Clear Height	=	12.330 ft
B Parapet height	=	0.0 ft

Wall Support Condition Top & Bottom Pinned

B

A



Vertical Loads

Vertical Uniform Loads . . . (Applied per foot of Strip Width)

Ledger Load	Eccentricity	0.0 in
Concentric Load		

DL : Dead Load
0.6380
0.0

Lr : Roof Live Load
0.0
0.0

Lf : Floor Live Load
0.760
0.0

S : Snow Load
0.0 k/ft
0.0 k/ft

Lateral Loads

Full area WIND load		47.0 psf
Fp	1.0	= 0.0 psf

Wall Weight Seismic Load Input Method :
Seismic Wall Lateral Load

Direct entry of Lateral Wall Weight
0.0 psf

MM

Masonry Slender Wall

File: S:\DWG\Year-2014\1419\CALCULATION\beam design.ec6
ENERCALC, INC. 1983-2011, Build:6.12.6.7, Ver:6.13.5.31

Lic. #: KW-06006869

Licensee: J. EDUARDO GONZALEZ PE, INC.

Description: MASONRY WALL DESIGN ZONE 4

DESIGN SUMMARY

Results reported for "Strip Width" of 12.0 in

Governing Load Combination ...		Actual Values ...		Allowable Values ...	
PASS	Moment Capacity Check +0.90D+1.60W	Maximum Bending Stress Ratio = 0.6961			
		Max Mu	1.484 k-ft	Phi * Mn	2.131 k-ft
PASS	Service Deflection Check D + L + W	Min. Defl. Ratio	373.078	Max Allow Ratio	360.0
		Max. Deflection	0.3966 in	Max. Allow. Defl.	0.4110 in
PASS	Axial Load Check +1.20D+0.50Lr+1.60L	Max Pu / Ag	47.809 psi	0.02 * fc	300.0 psi
		Location	0.05138 ft		
PASS	Reinforcing Limit Check	Controlling As/bd	0.00250	As/bd @ 0.1182 rho bal	0.1182
PASS	Minimum Moment Check +1.40D	Mcracking	0.4565 k-ft	Minimum Phi Mn	2.480 k-ft
Maximum Reactions ... for Load Combination....					
		Top Horizontal	D + L + W + S/2		0.2909 k
		Base Horizontal	W Only		0.2898 k
		Vertical Reaction	D + L + Lr		2.113 k

Design Maximum Combinations - Moments

Load Combination	Axial Load			Moment Values					0.6 * rho bal
	Pu k	0.06*fc*b*t k	Mcr k-ft	Mu k-ft	Phi	Phi Mn k-ft	As in ²	As Ratio	
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
+1.20D+1.60Lr+0.80W at 5.75 to 6.17	1.223	17.640	0.46	0.73	0.90	2.21	0.116	0.0025	0.1182
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
+1.20D+1.60S+0.80W at 5.75 to 6.17	1.223	17.640	0.46	0.73	0.90	2.21	0.116	0.0025	0.1182
+1.20D+0.50Lr+0.50L+1.60W at 5.75 to 6.17	1.603	17.640	0.46	1.53	0.90	2.31	0.116	0.0025	0.1182
+1.20D+0.50L+0.50S+1.60W at 5.75 to 6.17	1.603	17.640	0.46	1.53	0.90	2.31	0.116	0.0025	0.1182
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000
+0.90D+1.60W at 5.75 to 6.17	0.917	17.640	0.46	1.48	0.90	2.14	0.116	0.0025	0.1182
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000	0.0000

Design Maximum Combinations - Deflections

Load Combination	Axial Load		Moment Values		Stiffness			Deflections	
	Pu k		Mcr k-ft	Mactual k-ft	I gross in ⁴	I cracked in ⁴	I effective in ⁴	Deflection in	Defl. Ratio
	0.000		0.00	0.00	0.00	0.00	0.000	0.000	0.0
D + L + W at 6.17 to 6.58	1.756		0.46	0.95	342.40	29.27	30.138	0.397	373.1
D + L + W + S/2 at 6.17 to 6.58	1.756		0.46	0.95	342.40	29.27	30.138	0.397	373.1
D + L + S + W/2 at 5.75 to 6.17	1.779		0.46	0.45	342.40	29.34	342.400	0.027	5,561.6
	0.000		0.00	0.00	0.00	0.00	0.000	0.000	0.0
D + 0.5(L+Lr) + 0.7W at 6.17 to 6.58	1.376		0.46	0.64	342.40	28.28	32.154	0.155	957.6
	0.000		0.00	0.00	0.00	0.00	0.000	0.000	0.0

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal		Top Horizontal		Vertical @ Wall Base	
D Only	0.0	k	0.00	k	1.353	k
S Only	0.0	k	0.00	k	0.000	k
W Only	0.3	k	0.29	k	0.000	k
E Only	0.0	k	0.00	k	0.000	k
D + L + Lr	0.0	k	0.00	k	2.113	k
D + L + S	0.0	k	0.00	k	2.113	k

6

EDUARDO GONZALEZ PE
717 PONCE DE LEON BLVD
CORAL GABLES

Title :
Engineer:
Project Desc.:

Job #

MM

Masonry Slender Wall

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ENERCALC, INC. 1983-2011, Build:6.12.6.7, Ver:6.13.5.31

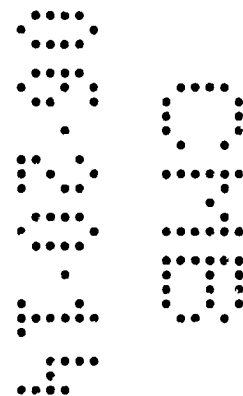
Lic.# : KW-06006869

Licensee : J. EDUARDO GONZALEZ PE, INC.

Description : MASONRY WALL DESIGN ZONE 4

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal	Top Horizontal	Vertical @ Wall Base
D + L + W + S/2	0.3 k	0.29 k	2.113 k
D + L + S + W/2	0.1 k	0.14 k	2.113 k
D + L + S + E/1.4	0.0 k	0.00 k	2.113 k



EDUARDO GONZALEZ PE
717 PONCE DE LEON BLVD
CORAL GABLES

Title :
Engineer:
Project Desc.:

Job #

MM

Masonry Slender Wall

File: S:\DWG\Year-2014\1419\CALCULATION\beam design.ec6
ENERCALC, INC. 1983-2011, Build:6.12.6.7, Ver:6.13.5.31
Licensee: J. EDUARDO GONZALEZ PE, INC

Lic: # KW-06006869
Description: MASONRY WALL DESIGN ZONE 5

Code References

Calculations per ACI 530-08/MSJC 2009 Sec. 3.3.5, IBC 2009, CBC 2010, ASCE 7-05

Load Combinations Used : 2006 IBC & ASCE 7-05

General Information

Calculations per ACI 530-08/MSJC 2009 Sec. 3.3.5, IBC 2009, CBC 2010, ASCE 7-05

Construction Type : Grouted Hollow Concrete Masonry

F'm	=	1.50 ksi	Nom. Wall Thickness	8 in	Temp Diff across thickness	=	deg F
Fy - Yield	=	60.0 ksi	Actual Thickness	7.625 in	Min Allow Out-of-plane Defl Ratio	=	360.0
Fr - Rupture	=	61.0 psi	Rebar "d" distance	3.750 in	Minimum Vertical Steel %	=	0.0020
Em = f'm *	=	900.0	Lower Level Rebar . . .				
Max % of ρ bal.	=	0.1182	Bar Size	# 5			
Grout Density	=	140 pcf	Bar Spacing	16.0 in			
Block Weight		Normal Weight					
Wall Weight	=	66.0 psf					

Wall is grouted at rebar cells only

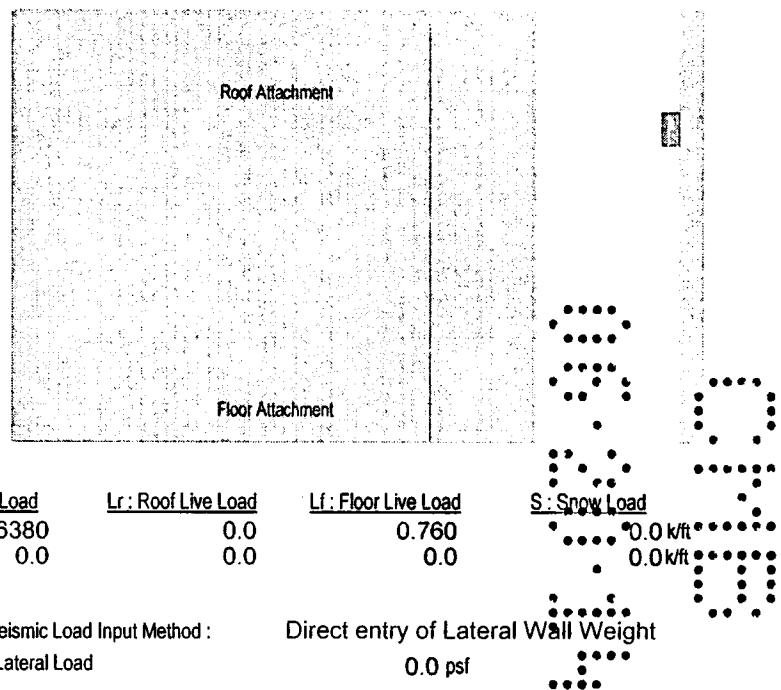
One-Story Wall Dimensions

A Clear Height = 12.330 ft
B Parapet height = 0.0 ft

Wall Support Condition Top & Bottom Pinned

B

A



Vertical Loads

Vertical Uniform Loads . . . (Applied per foot of Strip Width)

Ledger Load Eccentricity 0.0 in
Concentric Load

DL : Dead Load
0.6380
0.0

Lr : Roof Live Load
0.0
0.0

Lf : Floor Live Load
0.760
0.0

S : Snow Load
0.0 k/ft
0.0 k/ft

Lateral Loads

Full area WIND load = 54.0 psf
Fp 1.0 = 0.0 psf

Wall Weight Seismic Load Input Method :
Seismic Wall Lateral Load

Direct entry of Lateral Wall Weight
0.0 psf

MM

Masonry Slender Wall

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ENERCALC, INC. 1983-2011, Build:6.12.6.7, Ver:6.13.5.31

Lic. #: KW-06006869

Licensee: J. EDUARDO GONZALEZ PE, INC.

Description : MASONRY WALL DESIGN ZONE 5

DESIGN SUMMARY

Results reported for "Strip Width" of 12.0 in

Governing Load Combination ...		Actual Values ...		Allowable Values ...	
PASS	Moment Capacity Check +0.90D+1.60W	Maximum Bending Stress Ratio =	0.4461		
		Max Mu	1.686 k-ft	Phi * Mn	3.780 k-ft
PASS	Service Deflection Check D + L + W	Min. Defl. Ratio	488.575	Max Allow Ratio	360.0
		Max. Deflection	0.3028 in	Max. Allow. Defl.	0.4110 in
PASS	Axial Load Check +1.20D+0.50Lr+1.60L	Max Pu / Ag	42.034 psi	0.02 * f'c	300.0 psi
		Location	0.05138 ft		
PASS	Reinforcing Limit Check	Controlling As/bd	0.0050	As/bd @ 1182 rho bal	0.1182
PASS	Minimum Moment Check +1.40D	Mcracking	0.5013 k-ft	Minimum Phi Mn	4.650 k-ft
Maximum Reactions ... for Load Combination...					
		Top Horizontal	D + L + W + S/2		0.3339 k
		Base Horizontal	W Only		0.3329 k
		Vertical Reaction	D + L + S + W/2		2.212 k

Design Maximum Combinations - Moments

Load Combination	Axial Load			Moment Values				0.6 * rho bal
	Pu k	0.06*f'c*b*t k	Mcr k-ft	Mu k-ft	Phi	Phi Mn k-ft	As in ²	As Ratio
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
+1.20D+1.60Lr+0.80W at 5.75 to 6.17	1.286	20.880	0.50	0.84	0.90	3.85	0.233	0.0050
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
+1.20D+1.60S+0.80W at 5.75 to 6.17	1.286	20.880	0.50	0.84	0.90	3.85	0.233	0.0050
+1.20D+0.50Lr+0.50L+1.60W at 5.75 to 6.17	1.666	20.880	0.50	1.72	0.90	3.93	0.233	0.0050
+1.20D+0.50L+0.50S+1.60W at 5.75 to 6.17	1.666	20.880	0.50	1.72	0.90	3.93	0.233	0.0050
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000
+0.90D+1.60W at 5.75 to 6.17	0.965	20.880	0.50	1.69	0.90	3.78	0.233	0.0050
	0.000	0.000	0.00	0.00	0.00	0.00	0.000	0.0000

Design Maximum Combinations - Deflections

Load Combination	Axial Load	Moment Values		Stiffness			Deflections	
	Pu	Mcr	Mactual	I gross	I cracked	I effective	Deflection	Defl. Ratio
	k	k-ft	k-ft	in ⁴	in ⁴	in ⁴	in	
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
D + L + W at 6.17 to 6.58	1.805	0.50	1.07	376.00	45.32	46.489	0.303	488.6
D + L + W + S/2 at 6.17 to 6.58	1.805	0.50	1.07	376.00	45.32	46.489	0.303	488.6
D + L + S + W/2 at 5.75 to 6.17	1.832	0.50	0.52	376.00	45.37	101.183	0.030	4,902.2
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0
D + 0.5(L+Lr) + 0.7W at 6.17 to 6.58	1.425	0.50	0.73	376.00	44.54	49.457	0.130	1,133.9
	0.000	0.00	0.00	0.00	0.00	0.000	0.000	0.0

Reactions - Vertical & Horizontal

Load Combination	Base Horizontal		Top Horizontal		Vertical @ Wall Base
D Only	0.0	k	0.00	k	1.452 k
S Only	0.0	k	0.00	k	0.000 k
W Only	0.3	k	0.33	k	0.000 k
E Only	0.0	k	0.00	k	0.000 k
D+L+Lr	0.0	k	0.00	k	2.212 k
D+L+S	0.0	k	0.00	k	2.212 k