



MIAMI BEACH

Plan Routing Sheet

General Information		
Date <i>8-5-13</i>	Process/Permit Number Re-Submittals Only <i>B130 4674</i>	Historic (Y/N)
Job Address <i>3003 Sheridan Ave.</i>		
Contact Name <i>Rose Perez</i>	E-mail	Telephone

Re-submittals - New Sheets:

☒ **Yes**

☐ **No**

List all new sheets:

Calc's.

OFFICE USE ONLY

Required Approvals - As Indicated

<input type="checkbox"/> Planning & Zoning	<input type="checkbox"/> Fire	<input type="checkbox"/> Public Works
<input type="checkbox"/> Flood Plain Management	<input type="checkbox"/> Building	<input checked="" type="checkbox"/> Structural
<input type="checkbox"/> Electrical	<input type="checkbox"/> Mechanical	<input type="checkbox"/> Plumbing
<input type="checkbox"/> Elevator		

Comments:

Reviewer: *[Signature]*

Date: *8/5/13*

☒ **Walk Thru**

☐ **Drop Off**



MIAMI BEACH

BUILDING DEPARTMENT

1700 Convention Center Drive, 2nd Floor
Miami Beach, FL 33139
Phone: (305) 673-7610 Fax: (305) 673-7857

016
07.01.13

Owner/ Qualifier / Contractor Estimate Construction Cost Affidavit
(To be submitted for the main/master permits or the stand alone permits)

Permit Number: B 1304674

Date: 6/14/2013

Job Address: 3003 SHERIDAN AVE

Folio No.: 02-3227-001-0040

The construction cost should include the work under the main Permit and all associated permits.

Part I: FEMA 50% Related Construction Cost		
Items to be excluded from Estimate Construction Cost for Part I (FEMA 50% Related Construction Cost): <i>Plan and Specification, Survey Cost, Permit Fees, Swimming Pools, detached structures (garages, storages, cabanes), Landscaping, Fences, Yard light, Not Built-Ins Appliances and Furniture.</i>		
Estimated Construction Cost	General Contractor Cost	Owner Cost
Demolition & Removal	4500.00	
Building & Structural Elements	14500.00	
Roofing	3000.00	
Doors & Windows	-	
Railing	-	
Interior Finish, Floor Covering, Painting		
Cabinets and Furniture-Built-Ins	-	
Appliances-Built-Ins	-	
Other Building related Items		
Electrical including Fixtures	2500.00	
Elevator		
*Mechanical-HVAC-equipments		
Plumbing including Fixtures		
Overhead and Profit	6500.00	
Sub Total Construction Cost	\$ 32000.00	\$
Sub Total Construction Cost Estimate for FEMA 50% Rule Purposes	\$	



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Part II: Non-Related FEMA 50% Construction Cost		
Estimated Construction Cost	General Contractor Cost	Owner Cost
Swimming Pools	—	
Fences, Pavers, Sidewalks, Site Improvements	3000.00	
Yard Light	—	
Other and detached: garages, storage and cabanas	—	
Sub Total Cost	\$ 3000.00	\$
Sub Total Construction Cost Estimate for non FEMA 50% Rule Purposes	\$	

Part III: Total Construction Cost (Note: The construction cost will be validated by Plan Examiners)	
Estimated Construction Cost	
Sub Total Construction Cost Estimate for FEMA 50% Rule Purposes-Part I	\$ 32,000.00
Sub Total Construction Cost Estimate for Non FEMA 50% Rule Purposes- Part II	\$ 3,000.00
Total Construction Cost Estimate. (Add Part I and Part II of Construction Cost)	\$ 35,000.00

Part IV: Signature Required	
<p>If the improvements cost will increase at any point during the proposed construction, it is Owner and the Contractor of Record responsibility to submit the revised Improvements cost to the Building Department for review and approval.</p>	
<p><u>Marta Nizam</u> Signature of Owner</p>	
<p>STATE OF FLORIDA COUNTY OF <u>MIAMI-DADE</u></p>	
<p>Sworn to and Subscribed before me this <u>20th</u> day of <u>JUNE</u> 20<u>13</u>, by:</p>	
<p><u>MARTA NIZAM</u></p>	
<p><input checked="" type="checkbox"/> Personally known <input type="checkbox"/> Produced Identification - Type of Identification _____</p>	
<p><u>[Signature]</u></p>	
<p></p>	



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Phone: (305) 673-7610 Fax: (305) 673-7857

0718

07.01.13

[Signature]
Signature of Qualifier / Contractor

STATE OF FLORIDA
COUNTY OF MIAMI Dade

Sworn to and Subscribed before me this 1 day of June 20 13, by:
GUILLEN MONROY

☐ Personally known ☒ Produced Identification - Type of
Identification FL ID H 560 28564 3260 -

[Signature]
Signature of Notary Public

Part V. Building Department Use Only		
A	Sub Total Construction Cost Estimate for FEMA 50% Rule Purposes.	\$ <u>32,000.-</u> NOT TO BE COUNTED - EXT. WORK EXCLUDED SEE PERMITS
B	Over Five Year Improvements	\$ <u>98,000.-</u>
C	Total Improvements	\$ <u>98,000.-</u>
D	Building Tax Assessed Value	\$ <u>242,150.-</u>
E	Building Appraised Market Value	\$
F	Improvements Cost Ratio (C/E or C/D)	% <u>39.49.-</u>

If improvements cost exceed 40% of the Building Tax Value or Building Appraised Market Value is required for evaluation of Improvement Cost Ratio.

Check one box: *

☐ New Construction and Substantial Improvement ☒ Existing Building and Non Substantial Improvement

C. Gued
Engineering Inspector Name

Engineering Inspector Signature and Date

Notes: Over \$1,000,000.00 Improvements Cost requires Chief Governmental Compliance Division Approval, over \$50,000.00 Improvements Cost requires Building Director Approval.

Name

Signature and Date

7450 Griffin Road, Suite 140
Davie, FL 33314
Tel: 954-584-6115
Fax: 954-581-2415
E-mail: ppeana@soilprobe.net

Soilprobe Engineering & Testing, Inc.

June 27, 2013

Gemco USA
320 N Congress Avenue #107
Delray Beach, FL 33445

RE: Subsurface Investigation
Proposed Addition
3003 Sheridan Avenue
Miami Beach, FL 33140

In accordance with your request and authorization, **Soilprobe Engineering & Testing, Inc.** has completed subsurface exploration and geotechnical studies at the above referenced project site. We explored the general subsurface conditions in order to evaluate their suitability for supporting the anticipated construction, and to provide recommendations for site preparation and foundation design. Our work included standard penetration test (SPT) borings and engineering analyses. This report describes our explorations and tests, reports their findings, and presents our recommendations developed from the investigation.

Our report has been prepared specifically for this project. It is intended for the exclusive use of Gemco USA, their representatives and/or assigns. Our work has used methods and procedures consistent with local foundation engineering practices. No other warranty, expressed or implied, is made. We do not guarantee project performance in any respect, only that our work meets normal standards of professional care.

It is in our understanding that the proposed construction will consist of a roofed, open patio addition at the rear side of existing house. The existing and proposed construction and location of our soil tests are presented in the attached plot plan. The construction will consist of wood framed roof structure supported by reinforced concrete columns founded on shallow, spreadfootings designed for an allowable soil bearing capacity of 2,500 PSF. The geotechnical recommendations presented in this report are based on the available project information, proposed building location and the assumed data described in this report. If any of the noted information is incorrect, please inform this office, so that we may amend the recommendations presented in this report as appropriate.

The subsurface conditions at the site were explored with two (2), engineering borings advanced to a depth of 14 feet below existing ground surface. Our representatives selected the tests locations in field within the area of proposed addition as shown in the attached plot plan.

Samples of the in place materials were recovered at frequent intervals with a standard 24 inches split barrel sampler driven into the ground with a 140-pound hammer falling 30 inches. This work was performed on June 27, 2013, following closely the procedures recommended in ASTM Method D-1586. Detailed subsurface conditions encountered at the locations and depth explored is presented in the attached logs. Our drillers examined the soil recovered from the SPT sampler and maintained a log for each boring. Soil samples were inspected and classified using nomenclature consistent with the Unified Soil Classification System (USCS). Soil stratification, shown on the boring logs, is based on examination of recovered soil samples and drillers interpretation in the field. It indicates only the approximate boundaries between soil types. The actual transitions between the adjacent soil strata may be gradual and indistinct.

A review of boring logs revealed a surfacing layer of dark sand with roots approximately 24 inches thick underlain, to the depth of tests, by layered sand. Standard penetration resistance, N-value, recorded during the tests indicated that the sand was in a loose to medium dense condition.

The water table measured at the time of test was found at an approximately depth of 3.00 feet below prevailing grade. Fluctuations in ground water level should be anticipated throughout the year due to seasonal variations in rainfall, drainage and other factors. The groundwater level measured at the time of test is not intended to define a limit or ensure that future seasonal fluctuations in groundwater levels will not vary from this level. We recommend that the Contractor determine the actual water table at the time of the construction to determine groundwater impact on his construction procedures.

Based on our observations, results of boring, and evaluation of the existing soil conditions, it is our opinion that the soil at the site is generally suitable to support the proposed structure on conventional shallow foundations. However, densification of the existing loose sub-soils is required; if they are to support the shallow foundations, in order to avoid excessive settlements that may be detrimental to the structure. The following site preparation procedures are recommended prior to the installation of the foundations:

1. Clear and grub the proposed addition area plus a perimeter of minimum three feet outside the foundation limits removing topsoil, vegetation, roots and any existing improvements. The thickness of the surfacing topsoil with roots to be removed is expected to be approximately 24 inches. Tree roots and stumps shall be entirely removed.
2. Level and **compact** the construction area with a light, 2.5 ton (Ingersoll Rand DD-23) self propelled vibratory roller or Wacker BPU 3345A vibratory plate or equivalent in order to densify the existing loose sand to a minimum 95% of maximum dry density as determined by ASTM Method D-1557. During compacting operation the sandy soil shall have moisture content close to the optimum of approximately 12%. **Field density tests shall verify compacting effort** prior placing any additional fill.
3. Bring the addition pad area to the design grades using clean sand or sand rock mixture fill, free of organic or other deleterious materials, by placing the soil in layers not exceeding 8 inches in loose thickness. Compact each layer to a minimum 95% of maximum dry density prior placing the next lift of fill or concrete.

Gemco USA
3003 Sheridan Avenue
June 27, 2013
Page 3 of 3

It is our opinion that the addition pad installed as specified above will provide an allowable bearing capacity of 2,500 PSF. The foundation base shall be placed at minimum depth of 18 inches below lowest adjacent final grades.

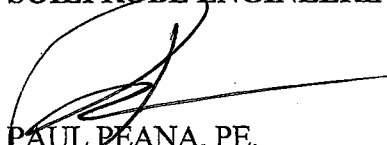
In all cases where vibrato-compaction is utilized, care must be taken to prevent damages to the nearby structures. Monitor the adjacent structures for induced vibrations and adjust the compacting operation as needed to prevent any damages to this building.

Any modification to the above-recommended procedures should be approved by Soilprobe Engineering & Testing, Inc. Further, in order to verify compliance with these specifications and document construction procedures used and actual conditions encountered, we recommend that Soilprobe Engineering & Testing, Inc. be retained to test the compacting effort on this project. This office does not accept any responsibility for any conditions that deviate from those described in this report, nor for the performance of the foundation if not engaged to provide construction observation, testing and certification for this project.

Due to the fact that soils are generally, naturally deposited material under variable conditions, it must be understood that major subsurface discontinuity may occur within short distances. It is unlikely that the tests used for this investigation revealed all subsurface conditions. Our office does not warrant or imply that the data collected on our log of borings are indicative of the subsurface features; except the locations where borings were taken. If variant or unusual soil conditions are found during construction, please notify this office for further evaluation.

It was a pleasure to have had the opportunity to perform this investigation for you, and we hope that you will call on us if we may be of further service.

Sincerely,
SOILPROBE ENGINEERING & TESTING, INC.



PAUL PEANA, PE.
#37334

Enc. Soil Logs
Plot plan

LOG OF BORING

JOB #:		Sheet:	1 of 1
Boring:	1	Date:	6/27/13

CLIENT:	GEMCO USA	Date started:	6/27/13
PROJECT:	3003 SHERIDAN STREET, MIAMI BEACH, FL	Date completed:	6/27/13
LOCATION:	SEE ATTACHED LOCATION SKETCH	Driller (s):	PP

Sample number	Depth In Feet	Blows / 6 inches	N Value	Sample Recovery (inches)	Water	Layer thickness	Symbol	DESCRIPTION: Soil type, color, texture, and consistency. Notes on drilling conditions.
	0.00					0'-0"		
		3	8	17		2'-0"		Dark sandy muck, roots
	1.00	4						
		4						
	2.00	5	9	22	≡	3'-0"		Tan sand with some shell fragments and rocks
		4						
	3.00	5						
		4	18	24		4'-0"		
	4.00	5						
		7						
	5.00	9	30	24				Tan grayish marine sand, medium grained
		9						
	6.00	10						
		14	42	24				
	7.00	15						
		15						
	8.00	16	40	24				
		19						
	9.00	20						
		22	43	24				Grey fine grained sand
	10.00	24						
		19						
	11.00	20	40	24		11'-0"		
		20						
	12.00	22						
		24	43	24				
	13.00	23						
		20						
	14.00	20				14'-0"		End of test 14'-0"
	15.00							
	16.00							
	17.00							
	18.00							
	19.00							
	20.00							

TYPE of SAMPLE:

D - disturbed
U.L. - Undist. Liner
S.T. - Shelby tube
S.S. - Sample Spoon
R.C. - Rock Core

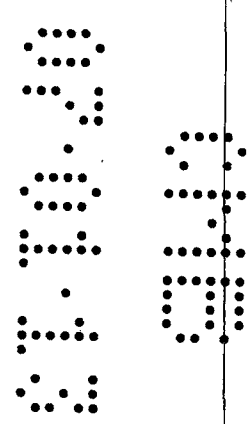
Cohesionless Density:

(2) - Penetrometer
0-10 Loose
10-30 Medium
30-50 Dense
50+ Very dense

Cohesive Consistency:

0-4 Soft
4-8 Medium Stiff
8-15 Stiff
15-30 Very Stiff
30+ Hard

140 Lb. WT. Hammer x 30" fall on 2" sampler



LOG OF BORING

JOB #:		Sheet:	1 of 1
Boring:	2	Date:	6/27/13
		Date started:	6/27/13
FL		Date completed:	6/27/13
		Driller (s):	PP

[illegible]

TYPE of SAMPLE:
D – disturbed (2) – Penetrometer
U.L. - Undist. Liner
S.T. – Shelby tube
S.S. – Sample Spoon
R.C. – Rock Core

0-10 Loose
10-30 Medium
30-50 Dense
50+ Very dense

0-4 Soft
4-8 Medium Stiff
8-15 Stiff
15-30 Very Stiff
30+ Hard

Soilprobe Engineering & Testing, Inc.

APPENDIX SUBSURFACE EXPLORATION INFORMATION

Our borings describe subsurface conditions only at the locations drilled and at the time drilled. They provide no information about subsurface conditions below the bottom of the boreholes. At locations not explored, subsurface conditions that differ from those observed in the borings may exist and should be anticipated.

The groundwater depth shown on our boring logs is the water level the driller(s) observed in the borehole when it was drilled. These water levels may have been influenced by the drilling procedures. An accurate determination of groundwater level requires long-term observation of suitable monitoring wells. The absence of a groundwater level on certain logs indicates that no groundwater data is available. It does not mean that no groundwater will be encountered at that boring location.

Standard Penetration Test Borings

The Standard Penetration Test (SPT) is a widely accepted method of testing foundation in place. The N-value obtained from the test has been correlated empirically with various soil properties. These empirical correlations allow satisfactory estimates to be made of how the soil is likely to behave when subjected to foundation loads. Tests are usually performed in the boreholes at intervals of five feet. In addition, our Firm performs tests continuously in the interval directly below the expected foundation-bearing grade where the soil will be most highly stressed.

Boreholes where SPT will be performed are drilled with a truck mounted SIMCO 2800 drill-rig. The boreholes are advanced by rotary drilling with a winged bit that makes a hole about seven inches in diameter. After the borehole has been advanced to the depth where a SPT will be performed, the soil sampler used to run the test is attached to the end of drill rods and lowered to the bottom of the borehole. The testing procedure used conforms closely to the methods recommended in ASTM D-1586. The sampler has a split-barrel 24 inches long and an outside diameter of 2.0 inches. It is driven into the ground below the bottom of the borehole using a hammer that weighs 140 pounds and falls freely 30 inches. The driller records the number of hammer blows needed to advance the sampler the first, second and third six-inches increments. The total number of blows required to advance the sampler the second and the third six-inches increments constitutes the test results; that is N-Value at the depth. The test is completed after the sampler has been driven not more than 24 inches or when refusal is encountered, whichever occurs first. Refusal occurs when 100 hammer blows advance the sampler six inches or less.

After the test is completed, the sampler is removed from the borehole and opened. The driller examines and classifies the soil recovered by the sampler. He places representative soil specimen from each test in closed jars or plastic bags and takes them to our laboratory. In the laboratory, additional evaluations and tests are performed, if needed. Jar samples are retained in our laboratory for thirty days, then discarded unless our clients request otherwise.

Hand Auger Borings

Hand auger borings are used if soil conditions are favorable, when the soil strata are to be determined within a shallow (approximately 6 feet) depth, or when access is not available for our truck-mounted or portable mounted drill-rigs. A three inches diameter hand bucket auger with a cutting head is simultaneously turned and pressed into the ground. The bucket auger is retrieved at approximately six inches increments and its content emptied for inspection. Sometimes posthole diggers are used, especially in the upper three feet or so. The soil samples obtained are described and representative samples put in jars or plastic bags and transported to the laboratory for further classification and testing, if necessary.



MIAMI BEACH

Building Department
1700 Convention Center Drive, 2nd Flr
Miami Beach, FL 33139

**NOTICE TO THE CITY OF MIAMI BEACH BUILDING
DEPARTMENT OF EMPLOYMENT AS SPECIAL INSPECTOR
UNDER THE FLORIDA BUILDING CODE**

I have been retained by: Gemco USA Corp to perform special inspector services under the
Florida Building Code at the 3003 Sheridan Ave. project on the below listed structures as of
7/1/13 (date). I am a professional engineer licensed in the State of Florida.

Process Number: B 1304674 Master Permit (IF APPLICABLE): _____

- ☐ Special Inspector for Pilings, FBC 1822.1.20
- ☐ Special Inspector for Lightweight Insulating Concrete, FBC 1917.2
- ☒ Special Inspector for Soil Compaction, FBC 1820.3.1
- ☐ Special Inspector for Precast Units and Attachments, FBC 1927.12.2 (By P.E. or R.A..)
- ☒ Special Inspector for Reinforced Masonry, FBC 2122.4 (By P.E. or R.A.)
- ☐ Special inspection for Steel Bolted & Welded Connections, FBC 2218.2 (By P.E. or R.A..)
- ☐ Special Inspector for Trusses over 35 feet long or 6 feet high, FBC 2319.17.2.4.2 (By P.E. or R. A..)
- ☐ Special Inspector for _____

NOTE: Only the marked boxes apply.

The following individual's employed by this firm or me are authorized representatives to perform inspections

1. Paul Perna, PE
2. Raniero Coletto
3. _____
4. _____

* Special inspectors utilizing authorized representatives shall insure the authorized representative is qualified by education or licensure to perform the duties assigned by the Special Inspector. The qualifications shall include: licensure as a professional engineer or architect; graduation from an engineering education program in civil or structural engineering; graduation from an architectural education program; successful completion of the NCEES Fundamentals Examination; or registration as a building inspector or general contractor.

I will notify the City of Miami Beach Building Department of any changes regarding authorized personnel performing inspection services.

I understand that all mandatory inspections, as required by the Florida Building Code, shall be requested by the permit holder and approved by the Building Department Inspectors. Inspections performed by the Special inspector hired by the Owner are in addition to the mandatory inspections performed by the Building Department. A Special Inspection Log for each building must be displayed in a convenient location on the site for inspection by the Building Department Inspectors. Further, upon completion of the work under each building permit, I will submit to the Building Department at the time of final inspection the completed inspection Log form and sealed statement that, to the best of my knowledge, belief and professional judgment those portions outlined above meet the intent of the Florida Building Code and are in subsequent accordance with the approved plans.

Architect/Engineer Signature: _____

Architect/Engineer

Name Printed: _____

Address: _____

Phone Number: _____

Owner/Agent Signature: _____

Owner/Agent Name Printed: _____

Building Department

Accepted By: _____

Signed and Sealed

37334

License Number

Date: 7/2/13

Paul Perna, PE

7450 Griffin Rd #140, Davie, FL.

954-384-6115

Guillermo A. Monroy

Guillermo A. Monroy

AV 8/5/13

07/02/13

BISHOP ENGINEERING COMPANY

Consulting Structural Engineering

STRUCTURAL CALCULATIONS

For

NIZAM PATIO ADDITION

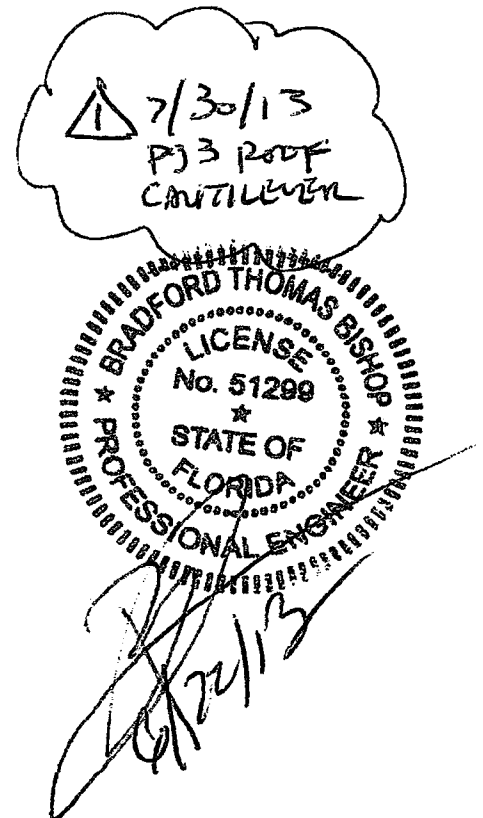
**3003 Sheridan Avenue
Miami Beach, Florida**

**Architect
Troy Moses-Panton
AR94097**

**By: Bradford T. Bishop, P.E.
Fl Lic # 51299**

Date: June 22, 2013

bec



BISHOP ENGINEERING COMPANY
WINTER PARK, FLORIDA
(407) 622-2477 FAX (407) 622-2479

JOB NIZAM PATIO
SHEET NO. 1 OF 3
CALCULATED BY BEB DATE 6/19/13
CHECKED BY _____ DATE _____
SCALE _____

PATIO ADDITION
3003 SHERIDAN AVE, MIAMI BEACH, FL
MIAMI-DADE COUNTY.

CODE: FLORIDA BLDG CODE 2010

ULTIMATE WIND = 175 MPH
RISK CAT = II
EXPOSURE = C

COMPOUND'S / CLADDING +45, 3PSF / -52.7 (STEEL BEAMS / JOISTS)

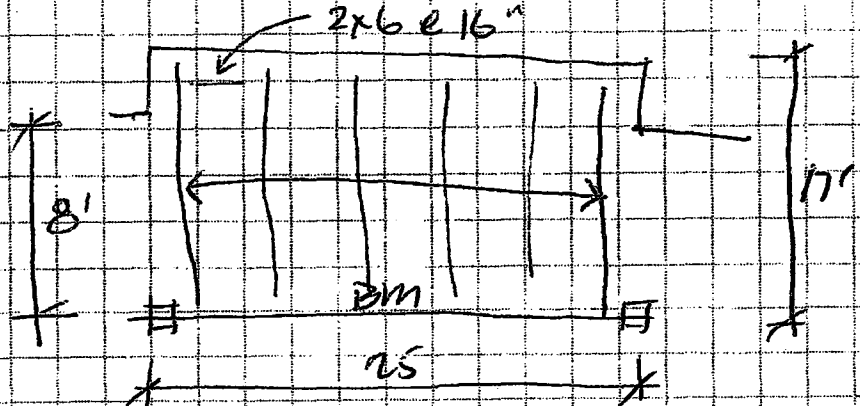
ROOF LL = 30 PSF
DL = 25 PSF

PORCH ADDITION - (2) COLUMN - COLLECT RAFTERS
TO EXISTING CMN WALL (EXIST HOUSE)

BM = 5 1/2" x 20" GLB
PART = 3 1/2" x 14" GLB
@ 5'-0" O.C.

NET UPLIFT (COVERED)

52.7 PSF - 5 PSF
= 47.7 PSF



2x6's $W = 47.7 \times 1.33' = 63.6 \text{ PLF}$ $L = 5'$ $M = 2385 \text{ lb}$
 $S_x \text{ REQ'D} = \frac{2385}{1400 \text{ PSI}} = 1.7143$

$S_x 2x6 = 1.5(5.5)^2/6 = 7.5 > 1.7 \text{ OK}$

REACTING = $47.7 \text{ PSF} \times 1.33' \times 5' \times (0.75) = 2385 \text{ lb}$
 SIMPSON LWS26 - CAP = $1165/1.6 = 728 \text{ lb OK}$ (2x6 OK).

CHECK PROFILES.

$$L = 17' \quad W = 47.7 \text{ PSF} \times 5' = 239 \text{ PLF}$$

$$M = 239 (17)^2 / 8$$

$$= 103,607 \text{ lb}$$

$$S_{XREQ} = 103,607 / 2400 \text{ PSI (COR.)}$$

$$= 43.16 \text{ in}^3$$

$$S_{X 3\frac{1}{2} \times 14 \text{ CLB}} = 3.5 (14)^2 / 6 = 114.33 \text{ in}^3$$

So, $3\frac{1}{2} \times 14$ CLB OK

STEEL PLATE w/ (3) $\frac{3}{4}$ " & (DBL SHEAR)

OK BY INSPECTOR.

CHECK CAPACILB BEAM

$$L = 25' \quad W = 47.7 \text{ PSF} \times (8.5' + 2.0' \text{ OVER HANG})$$

$$= 500 \text{ PLF}$$

$$M = 500 (25)^2 / 8 = 468,750 \text{ lb}$$

$$= 352,000 \text{ lb} \quad 1.33 \text{ (DURATION)}$$

$$S_{XREQ} = \frac{352,000}{2400 \text{ PSI}} = 146 \text{ in}^3$$

$$S_{X 5.5 \times 20 \text{ CLB}} = 5.5 (20)^2 / 6 = 366 \text{ in}^3 > 146 \text{ in}^3 \text{ OK}$$

ALL WOOD BEAMS OK.

CONX TO T.O. COL = (4) $\frac{3}{4}$ " (W/SHIM) ANCHOR BOLT.

$$\text{EA BOLT CAP} = 3,500 \# \times 4 = 14,000 \#$$

$$\text{UPLIFT} = 47.7 \times 25 \times (10.3) = 6,230 \# < 14,000 \# \text{ OK}$$

16" x 16" CMU COL OK BY INSPECTOR.

CHECK FOOTINGS

$$\text{TOTAL UPLIFT} = 6,300 \text{#} \times (1.5 \text{ STABILIZ}) = 9,450 \text{#}$$

$$\text{DL COL} = 150 \text{ PCF} \times 1.33 \times 6.33 \times 7.5' = 1,195 \text{#}$$

$$\text{DL. PIER} = 150 \text{ PCF} \times 1.33 \times 1.33 \times 3.5' = 940 \text{#}$$

$$\text{DL FCG} = 5.66 \times 5.66 \times 1.0 \times 150 \text{ PCF} = 1,805 \text{#}$$

$$7,740 \text{#}$$

$$\text{DL. (CONTRIBUTOR) SLABS} = 150 \times 0.33 \times 6.5^2$$

$$(\text{SLABS}) = 2,011 \text{#}$$

$$9,831 \text{# D.L.}$$

$$9,400 \text{# w/F.S.}$$

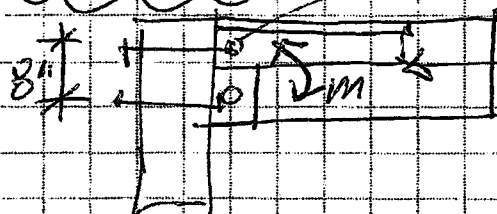
STABILIZ

STRUCTURAL IMPACTS / EXCEEDS.

ALL DEAD, LIVE, AND WIND LOADS
DESIGN REQUIREMENTS.

ROOF CANTILEVER

BOLTS



STEEL ANGLE
L4x4x1/4 (x2)

$$W = 600 \text{ PSF (UPLIFT)} \times 5'$$

$$= 3000 \text{ PLF}$$

$$M = .300 (5)^2 / 2 (x 12)$$

$$= 45 \text{#-K}$$

$$S_{\text{REQD}} = 45 / 1.66 \times 36$$

$$= 1.89$$

$$S_x \text{ L4x4x1/4} = 1.05 \times 2 \text{ ANGLES}$$

$$= 2.10 > 1.89 \text{ OK}$$

FOR BENDING

$$\text{TENSION ON BOLT. } 45 / 12 = 3.75 \text{#-K} = 11$$

$$T = 3.75 / 1.66 = 5.68 \text{#-K} = 5,680 \text{#} / 2 \text{ BOLTS}$$

$$= 2,840 \text{# BOLT.}$$

$$3/4 \text{\" DIA A307 BOLT (TENSION ALLOWABLE)} = 8.8 \text{#} > 2.8 > \text{OK}$$

B1304674

A Rear Covered Patio for MR. RIAD & MRS. MARTA NIZAM

3003 Sheridan Avenue - Miami Beach, FL 33140
Miami-Dade County



PROJECT LOCATION

CONSULTANTS

Architect

Name: Troy-Moses Panton
Lic. #: AR94097
Addr: PO Box 614323
Miami, FL 33261
Phone: 786-985-9156
e-mail: troymoses@gmail.com

Landscape Architect

Name: rafael e. batalla
Urban Design South
Phone: 561-749-0342
e-mail: rbatalla@udsia.com

General Contractor

Name: Bill Monroy
Gemco USA Corp.
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Client

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Phone: 310-903-6886
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REVISIONS

No.	Date	Action

48 HOURS PRIOR TO EXCAVATING
CONTRACTOR SHALL CALL FOR LOCATION
OF UNDERGROUND UTILITIES
305-673-7080
CITY OF MIAMI BEACH 305-673-7080

OFFICE COPY
CITY OF MIAMI BEACH
APPROVED FOR PERMIT BY
THE FOLLOWING:

BUILDING: YAS 2/26/13
ZONING: DRB/HPB
CONCURRENCY: R-1 6/11/13
PLUMBING: 2/26/13
ELECTRICAL: 2/26/13
MECHANICAL: 2/26/13
FIRE PREVENTION: 2/26/13
ENGINEERING: 2/26/13
PUBLIC WORKS: 2/26/13
STRUCTURAL: 2/26/13
ELEVATOR: 2/26/13

PUBLIC WORKS
PLAN REVIEW NOTICE
Phone 305-673-7080 Fax 305-673-7028

THIS PLAN REVIEW CONSTITUTES APPROVAL FOR
OBTAINING BUILDING PERMITS ONLY

All construction and/or use of equipment in the right-of-way and/or
easements, requires a separate Public Works Department permit prior
to start of construction.

Permit Requirements: Proof of existing sidewalk/curb area conditions
(pictures) and/or posting of sidewalk/curbway bonds
(Public Works Inspection of the right-of-way will be required prior to
final sign-off on the C.C. / C.O., or the release of bonds.)

Approved/Reviewed By: [Signature] Date: 2/26/13

PROJECT NAME

A Residential Alteration for
MR. RIAD & MRS. MARTA NIZAM
3003 Sheridan Avenue - Miami Beach FL 33140
Miami-Dade County

DRAWING NAME

Cover Sheet,
Project Location

Drawn By : TMKP
Checked By : TMKP
Issue Date : 2013-06-10
Scale :
Job No. : 0065

A0

COMPANY SEAL

6/10/2013 3:45:22 AM

Sheet Number	Sheet Name
A0	Cover Sheet, Project Location
A100	Site Plan, Existing Photo, Proposed Perspective, Structural Notes
A101	Floor & Roof Plans, Exterior Elevations, Cross Section
A102	Foundation & Framing Plans, Wall Section, Details

STRUCTURAL NOTES:

A. GENERAL

1. ALL NEW WORK SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE FLORIDA BUILDING CODE, 2010 EDITION REVISIONS AND ANY MORE STRICT STANDARDS AS REQUIRED BY THE CITY OF MIAMI BEACH, FLORIDA (MIAMI-DADE COUNTY).
2. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS PRIOR TO BEGINNING WORK. ANY DEVIATIONS FOUND SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING.
3. THE CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION AND SHALL PERFORM HIS WORK IN A SAFE MANNER.

4. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL TEMPORARY SHORING OR BRACING, AS REQUIRED, DURING CONSTRUCTION.

B. FOUNDATIONS

1. PERFORM FOUNDATION WORK AS INDICATED ON THE PLANS.
2. THE FOUNDATIONS HAVE BEEN DESIGNED FOR AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 1,500 PSF. THE CONTRACTOR SHALL PROVIDE SUITABLE MEANS FOR COMPACTING THE SOILS SO AS TO ACHIEVE THE ASSUMED ALLOWABLE SOIL BEARING CAPACITY USED IN THE DESIGN. ALL SOILS BENEATH SLABS AND FOUNDATIONS SHALL BE COMPACTED AND SHALL BE TESTED BY AN INDEPENDENT GEOTECHNICAL TESTING LABORATORY PRIOR TO PLACING ANY CONCRETE AS FOLLOWS: CONTINUOUS FOOTING - ONE TEST EVERY 100 LINEAL FEET, INDIVIDUAL COLUMN FOOTINGS - ONE TEST FOR EACH FOOTING, AND EVERY 50 YARDS OF EACH CLASS OF CONCRETE.
3. THE CONTRACTOR IS RESPONSIBLE FOR ANY DEWATERING AT THE SITE DURING CONSTRUCTION.

C. CONCRETE

1. ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 2010 EDITION, AND THE REQUIREMENTS OF ACI 301-08 BY THE "AMERICAN CONCRETE INSTITUTE FOR CAST-IN-PLACE, REINFORCED CONCRETE AND REINFORCING, AND ACI 318-08 "BUILDING CODE REQUIREMENTS FOR CONCRETE STRUCTURES".
2. CEMENT SHALL BE PORTLAND CEMENT, TYPE I.
3. REINFORCING SHALL BE ASTM A-615, GRADE 60 (DEFORMED) AND FREE OF RUST, MILL, SCALE AND OIL.
4. ALL CONCRETE SHALL BE PLACED AND COMPACTED TO A 28-DAY DESIGN COMPRESSIVE STRENGTH OF 3000 PSI, EXCEPT CONCRETE BEAMS (4000 PSI) SLUMPS SHALL BE AS FOLLOWS: CONCRETE FOR MASONRY GROUT 8" TO 11" ALL OTHER CONCRETE 4" TO 11"

SLUMPS SHALL BE MEASURED AT THE DISCHARGE END OF THE HOSE, IF A PUMP IS USED.

5. ALL CONCRETE SLABS AT SIDEWALKS, STAIR TREADS, AND LANDINGS SHALL HAVE A NON-SLIP, BROOM FINISH.
6. PROVIDE CONCRETE COVER OVER ALL REINFORCING AS FOLLOWS:

FOOTINGS (SIDES AND BOT) 3"
FORMED, EXPOSED TO EARTH 2"
FORMED, EXPOSED TO WEATHER 1 1/2"
FORMED, NOT EXPOSED TO EARTH OR WEATHER 3/4"

7. ALL REINFORCING IN SLABS-ON-GRADE SHALL BE 6x6 W/1.4 W/1.4 WELDED WIRE FABRIC, TYP UNO ASTM A-65.
8. REINFORCING BARS IN CONCRETE SHALL BE LAPPED A MIN OF 36 BAR DIA, TYP (EXCEPT 48 BAR DIA AT VERTICAL MASONRY WALL REINFORCING), 3/4" CHAMFER AT ALL EXPOSED CONCRETE EDGES (BINS).
9. THE CONTRACTOR SHALL PREPARE SHOP DRAWINGS SHOWING ALL REINFORCING PLACEMENT AND CONCRETE DESIGN MIXES TO BE AVAILABLE FOR THE A/E TO REVIEW FOR CONFORMANCE TO PROJECT REQUIREMENTS PRIOR TO PLACING ANY CONCRETE. ALL CONCRETE SHALL BE FROM A SINGLE SOURCE SUPPLIER.

D. MASONRY

1. ALL MASONRY WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 530-08 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
2. ALL MASONRY SHALL BE NORMAL WEIGHT, ASTM C-90, TYPE II, NON-MOISTURE CONTROLLED UNITS AND SHALL HAVE AN ASSUMED STRENGTH OF MASONRY ASSEMBLAGE, $f_m = 1500$ PSI (1900 PSI ON THE NET AREA).
3. ALL MASONRY SHALL BE NOMINAL SIZE UNITS 8"x8"x16" (7 5/8"x7 5/8"x15 5/8") PROVIDE HALF-SIZE, KNOCK-OUT, OR U-BLOCKS WHERE SPECIFIED.
4. ALL MASONRY SHALL BE LAID IN RUNNING BOND WITH 3/8" STRUCK JOINTS. MORTAR FOR MASONRY SHALL BE TYPE S THROUGH-OUT, (TYPE M FOR BELOW GRADE PLACEMENT).
5. DO NOT WET MASONRY PRIOR TO APPLYING MORTAR.
6. PROVIDE PRECAST 1"x6" LINTELS OVER ALL MASONRY OPENINGS. PRECAST LINTELS SHALL BE REINFORCED WITH (1) #6 CONT AND SHALL BE FILLED SOLID WITH GROUT. LINTELS SHALL BEAR A MINIMUM OF 8" EACH END. KNOCK OUT BLINTEL AT BEARING TO ADMIT VERTICAL REINFORCING.
7. PROVIDE STANDARD DURO-WALL LADDER TYPE, 3/16" SIDE AND CROSS RODS, GALVANIZED HORIZONTAL JOINT REINFORCING PLACED AT 16" OC, TYP.

E. WOOD FRAMING

1. ALL WOOD FRAMING SHALL BE IN ACCORDANCE WITH THE NDS "NATIONAL DESIGN SPECIFICATIONS" FOR STRUCTURAL WOOD FRAMING, 2012 EDITION.
2. ALL LUMBER SHALL BE NO. 2 OR BETTER SO, YELLOW PINE, WITH $F_b = 1200$ PSI, $E = 1,600,000$ PSI, MC = 18%, KILN DRIED IN STANDARD NOMINAL SIZES AS SHOWN.
3. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY AND/OR EXPOSED TO EARTH OR WEATHER SHALL BE PRESERVATIVE TREATED (WOLMANIZED OR PRESSURE TREATED), TYP.
4. ALL CONNECTIONS SHALL BE AS INDICATED AND USE GALV CONNECTORS BY SIMPSON FOR DIFFERENT TYPES OF CONNS SHOWN.
5. ANCHOR BOLTS FOR SILL PLATES SHALL BE A307 AND GALVANIZED (G-90), TYP WITH WASHERS.
6. ROOF SHEATHING SHALL BE 5/8" CDX APA PLYWOOD, NAILED WITH (SHANK) 10d NAILS AT 4" O.C. AT SUPPORTED EDGES, AND 10d NAILS AT 8" O.C. AT INTERMEDIATE SUPPORTS. PROVIDE (2) PLYWOOD CLIPS PER SPAN BETWEEN SHEET EDGES. PROVIDE SOLID 2x BLOCKING BETWEEN SUPPORTS AT ALL HIP, RIDGES, VALLEYS, AND CHANGES IN ROOF SLOPE.
7. WALL (OR SIM) SHEATHING SHALL BE 1/2" CDX APA PLYWOOD (1/2" NOMINAL), NAILED WITH 10d NAILS AT 4" O.C. AT SUPPORTED EDGES, AND 10d NAILS AT 8" O.C. AT INTERMEDIATE SUPPORTS.
8. ALL TRUSS CONNECTIONS SHALL BE MADE WITH SIMPSON HARDWARE, GALVANIZED G-60 (MIN), UNLESS SPECIFICALLY NOTED OTHERWISE, TYP.

G. DESIGN LOADS

ROOF (SLOPED)	LIVE LOAD	DEAD LOAD
TOP CHORD	30 PSF	20 PSF
BOTTOM CHORD	0	5 PSF
SLAB-ON-GRADE	100 PSF	75 PSF

WIND LOADS:

THE STRUCTURE HAS BEEN DESIGNED FOR WIND LOADS IN ACCORDANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE, 2010 EDITION AS FOLLOWS:
BASIC WIND SPEED (3-SECOND GUST) = 175 MPH

BUILDING RISK CATEGORY = II
WIND EXPOSURE CATEGORY = C

INTERNAL PRESSURE COEFFICIENT (WINDWARD) = +0.43/-0.73
INTERNAL PRESSURE COEFFICIENT (LEEWARD) = -0.51/-0.21
INTERNAL PRESSURE COEFFICIENT (SIDE WALL) = -0.66/-0.35
BUILDING HEIGHT = abt 15'

BUILDING TYPE = OPEN
HEIGHT AND EXPOSURE ADJUSTMENT COEFFICIENT = 1.00
ROOF SLOPE = 5:12 (SEE ARCH'L)

MAXIMUM DESIGN WIND PRESSURES

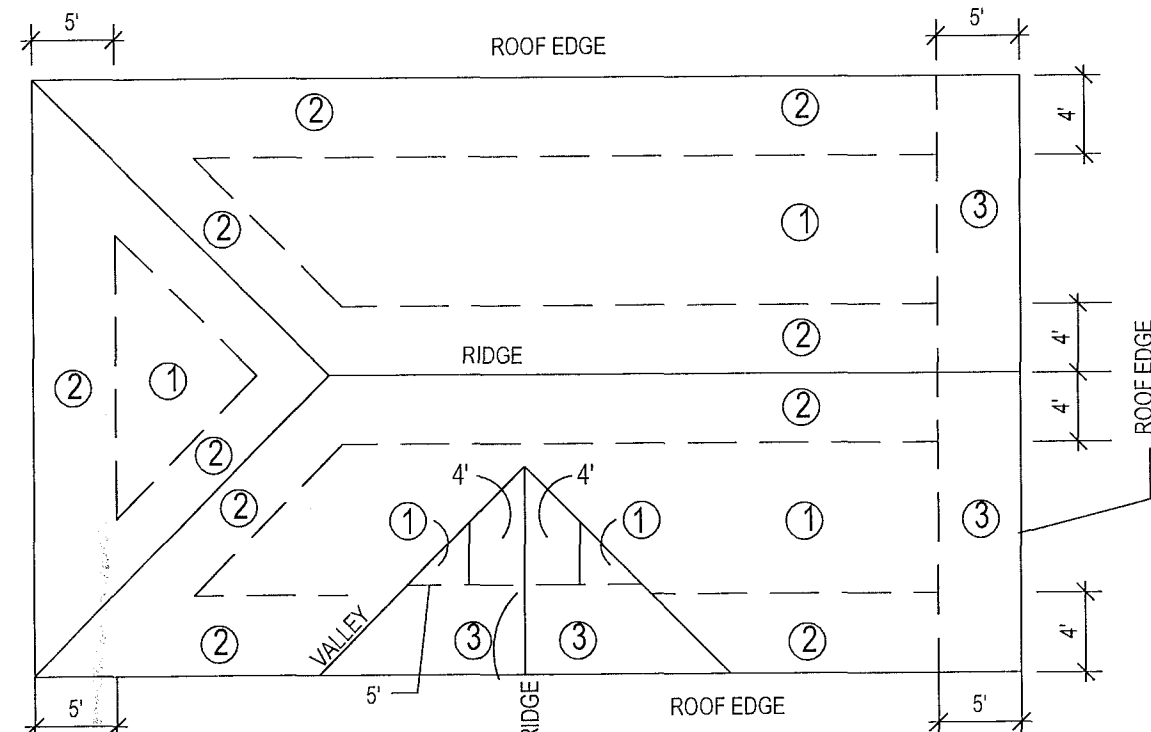
- MAIN WIND FORCE RESISTING SYSTEM (MWFRS)

WINDWARD WALLS +42.5 PSF / -59.9 PSF
LEEWARD WALLS -48.4 PSF / -25.9 PSF
SIDE WALLS -59.8 PSF / -33.9 PSF

WINDWARD ROOFS -32.9 PSF / -19.9 PSF
LEEWARD ROOFS -50.1 PSF / -31.6 PSF

- COMPONENTS AND CLADDING (ASCE 7-10)

GROSS ROOF UPLIFT = -65.3 PSF



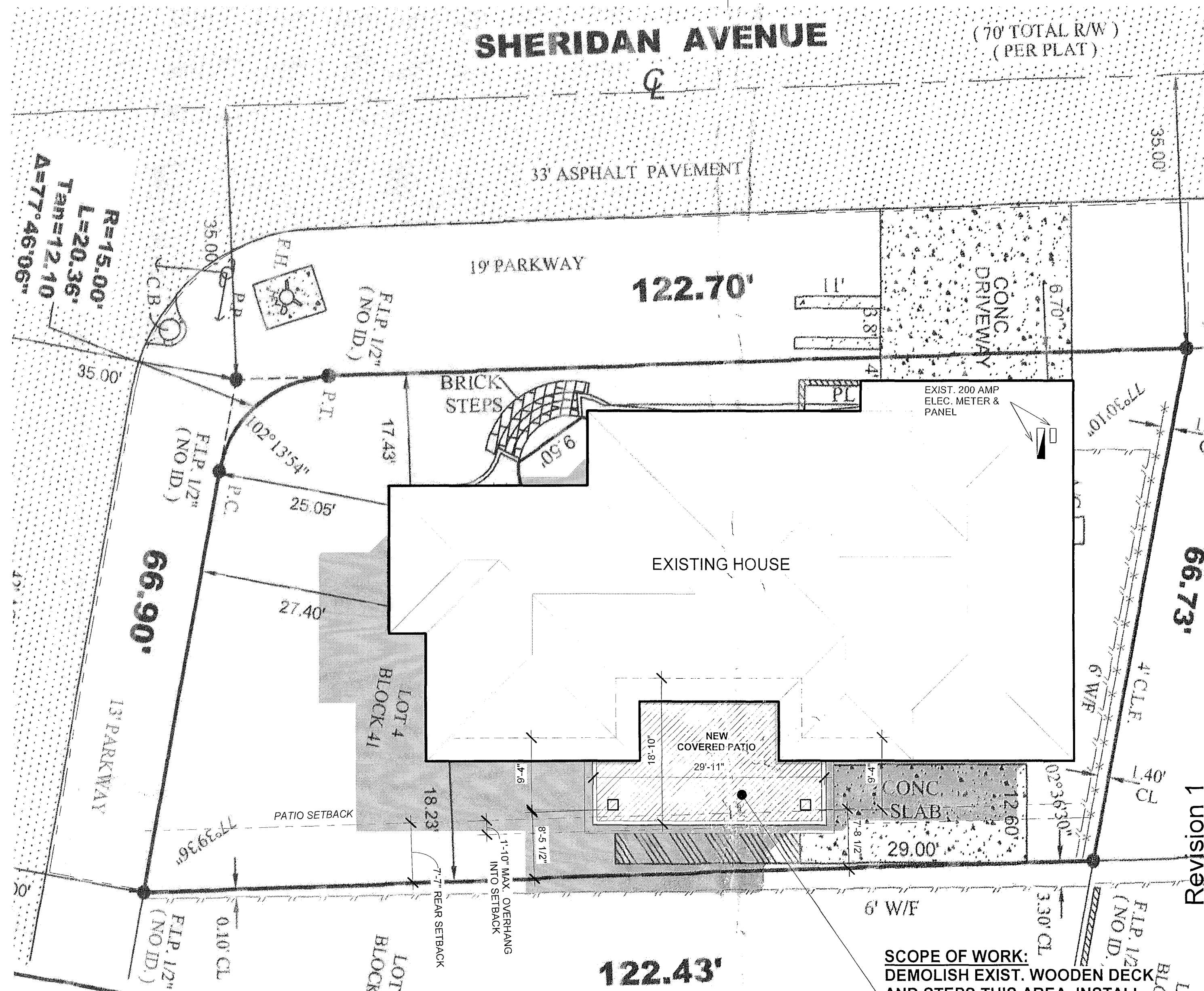
ALL SHEATHING NAILS TO BE SHANK NAILS, TYP.

ZONE 1: 10d SHANK NAIL @ 8" O.C. AT EDGES AND @ SUPPORTS

ZONE 2: 10d SHANK NAIL @ 5" O.C. AT EDGES AND 12" O.C. @ SUPPORTS

ZONE 3: 10d SHANK NAIL @ 3" O.C. AT EDGES AND 6" O.C. @ SUPPORTS

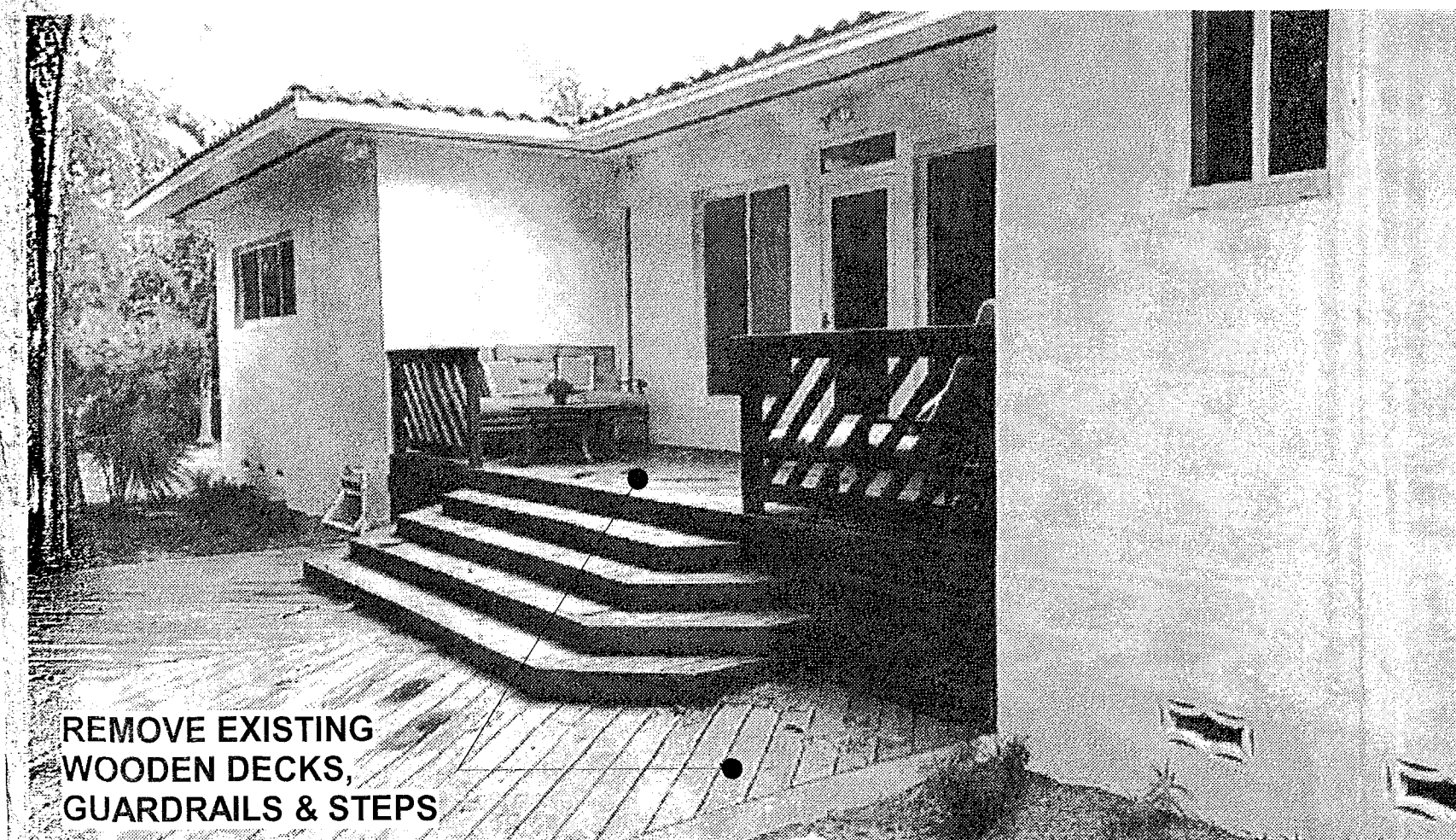
ROOF SHEATHING NAIL ZONE



LEGAL DESCRIPTION:
LOT 4 BLOCK 41 OF ORCHARD SUBDIVISION NO. 1.
ACCORDING TO THE PLAT THEREOF AS RECORDED IN
PLAT BOOK 6 PAGE 111 OF THE PUBLIC RECORD OF
MIAMI DADE COUNTY, FLORIDA

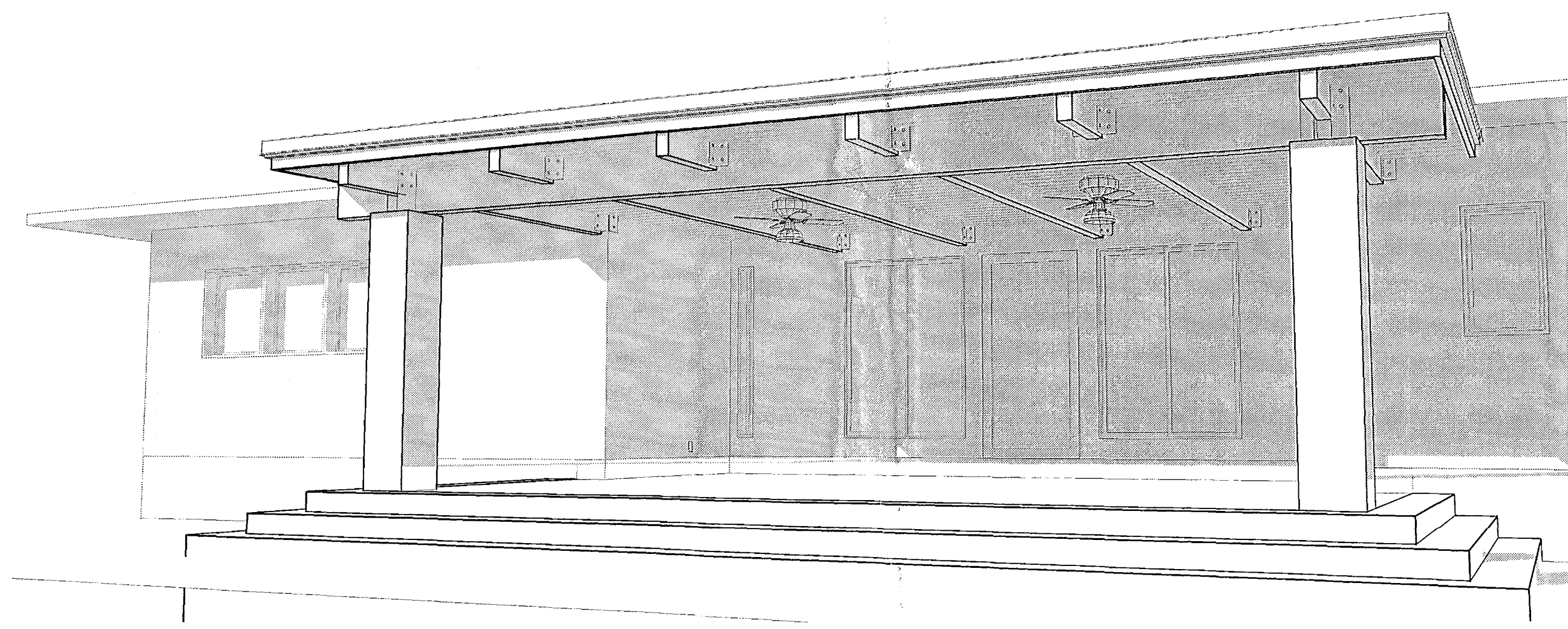
① AERIAL SITE & DEMOLITION PLAN
1" = 10'-0"

SCOPE OF WORK:
DEMOLISH EXIST. WOODEN DECK
AND STEPS THIS AREA. INSTALL
NEW WOOD FRAME COVERED
PATIO AND PAVED DECK



REMOVE EXISTING
WOODEN DECKS,
GUARDRAILS & STEPS

PHOTO OF EXISTING CONDITION



PERSPECTIVE OF NEW COVERED PATIO

CONSULTANTS

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General Contractor

Name: Bill Monroy
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Client

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REVISIONS

No.	Date	Action
1	2013/06/24	STRUCTURAL

PROJECT NAME

A Residential Alteration for

MR. RIAD & MRS. MARTA NIZAM

3003 Sheridan Avenue - Miami Beach FL 33140
Miami-Dade County

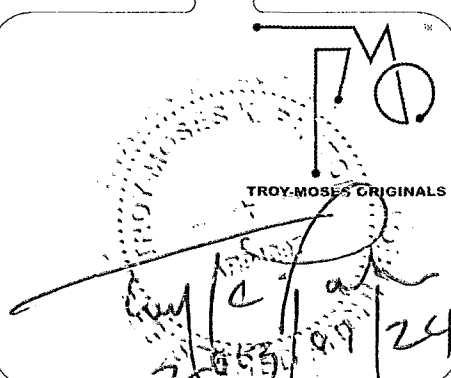
DRAWING NAME

Site Plan, Existing
Photo, Proposed
Perspective,
Structural Notes

Drawn By : TMKP
Checked By : TMKP
Issue Date : 2013-06-10
Scale : As indicated
Job No. : 0065

A100

COMPANY SEAL



EXIST.
200 AMP
PANEL

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A Residential Alteration for

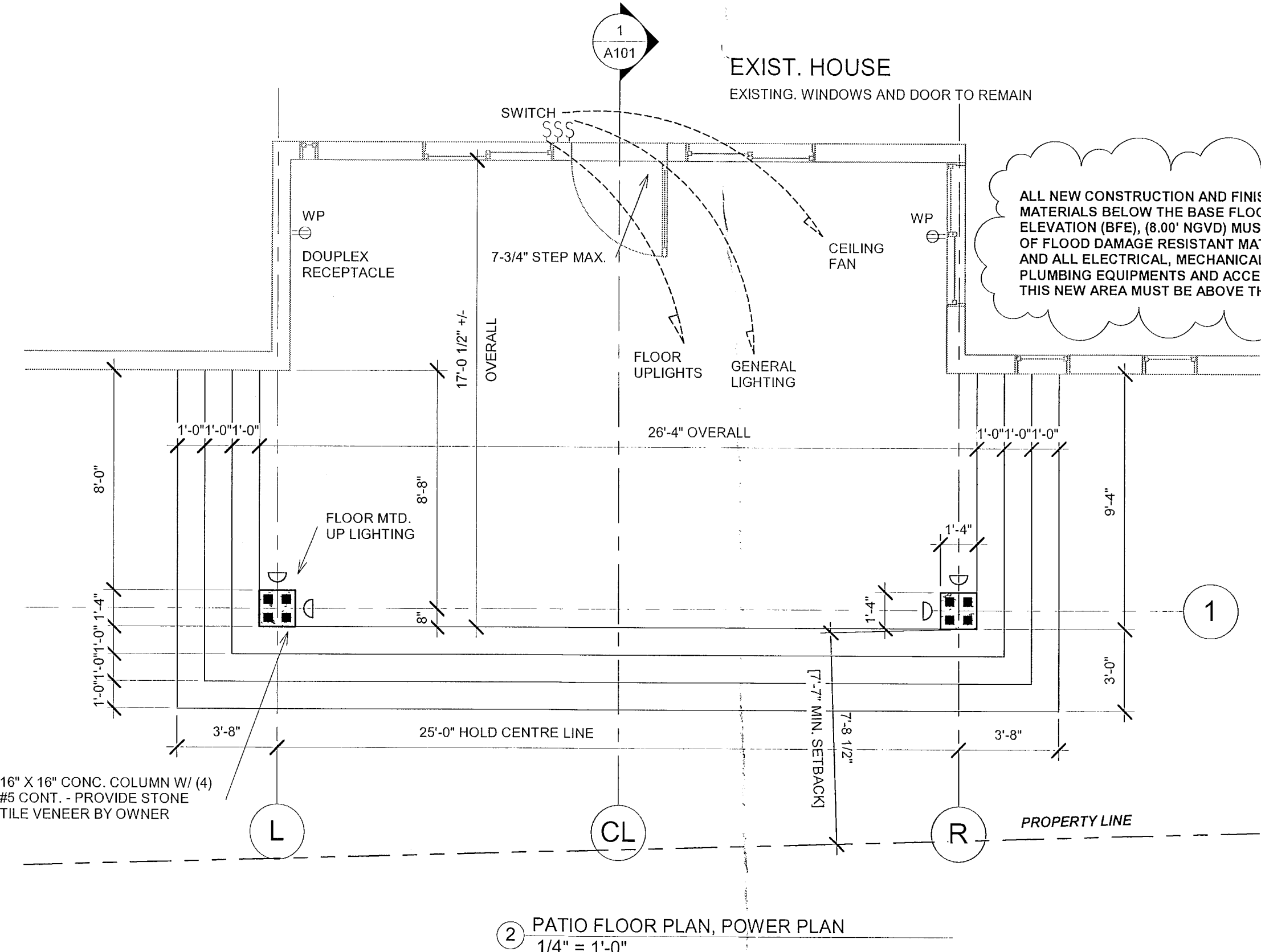
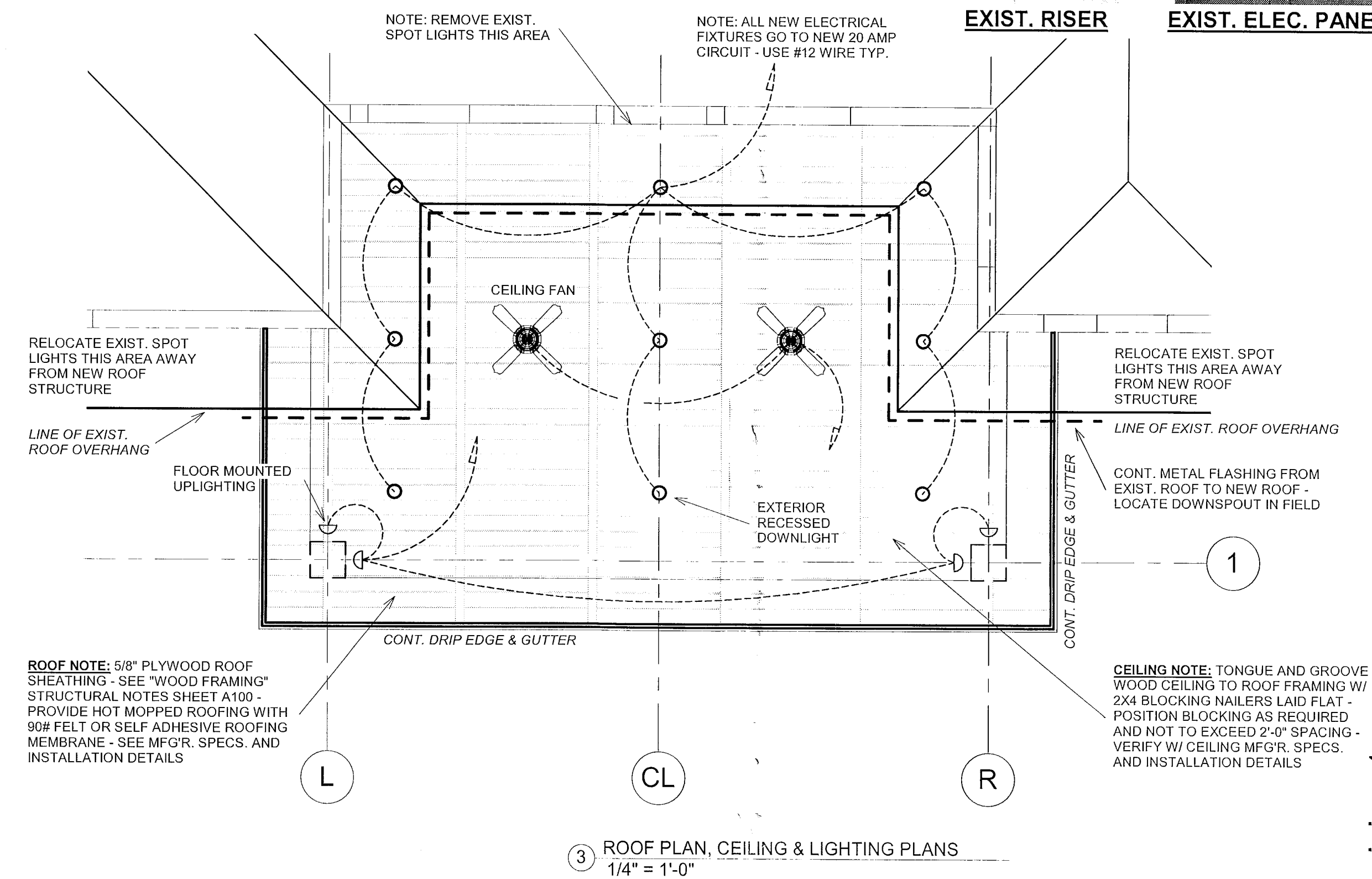
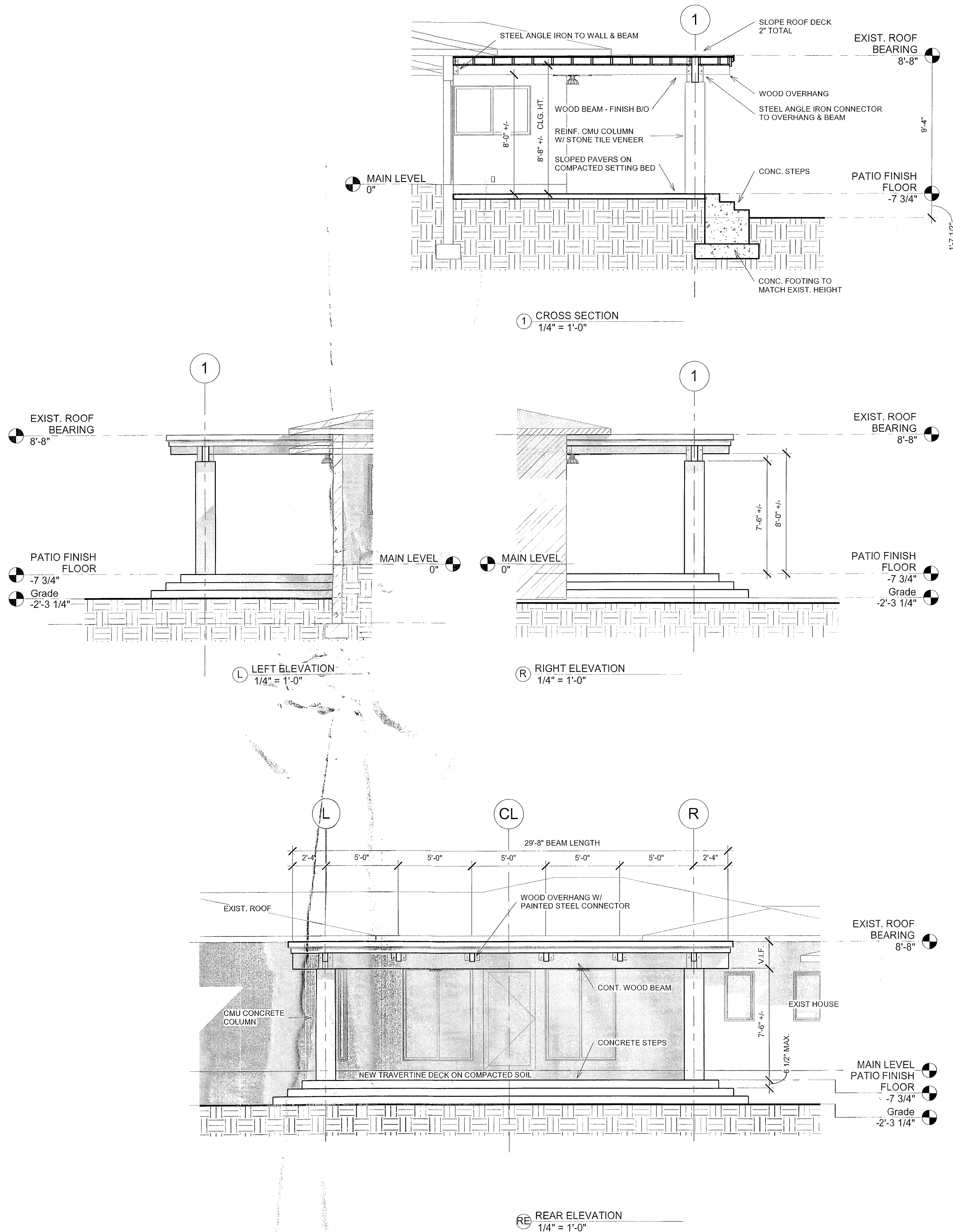
MR. RIAD & MRS. MARTA NIZAM
3003 Sheridan Avenue - Miami Beach FL 33140
Miami-Dade County

Floor & Roof
Plans, Exterior
Elevations, Cross
Section

Drawn By	:	TMKP
Checked By	:	TMKP
Issue Date	:	2013-06-10
Scale	:	1/4" = 1'-0"
Job No.	:	0065

A101

770
TROY MOORE ORIGINALS
Cup / C / am
3-13-07 / zcf



Revision 1

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REVISIONS

No.	Date	Action
1	2013/06/24	STRUCTURAL

Revision 1

PROJECT NAME
A Residential Alteration for
MR. RIAD & MRS. MARTA NIZAM
3003 Sheridan Avenue - Miami Beach FL 33140
Miami-Dade County

DRAWING NAME

Foundation & Framing Plans, Wall Section, Details

Drawn By: TMKP
Checked By: TMKP
Issue Date: 2013-06-10
Scale: As indicated
Job No.: 0065

A102

COMPANY SEAL

7/24/2013 10:41:10 AM

